

Enclosure 8 to  
NRC-96-0127  
Page 1

ENCLOSURE 8

DC-5766 Vol I Rev 0

System 2850-2 Duct String Analysis and Support Evaluation

9611250121 961118  
PDR ADOCK 05000305  
P PDR

DESIGN CALCULATION COVER SHEET

**PART 1: DESIGN CALCULATION IDENTIFICATION**

A) Design Calculation Number DC-5766		B) Volume Number I	
C) Revision 0	D) PIS Number T4102	E) QA Level <input type="checkbox"/> Non-Q <input checked="" type="checkbox"/> I <input type="checkbox"/> IM	
F) ASME Code Classification <input checked="" type="checkbox"/> NA		G) Certification Required <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No	
H) Lead Discipline MECHANICAL / CIVIL		I) Incorporation Code F	
J) Title CC HVAC CONCERN RESOLUTION TASK - DUCT STRING ANALYSIS AND SUPPORT EVALUATION OF 2850 SYSTEM SERIES			
K) Design Change Documents Incorporated (Number and Revision)  NONE			
L) Design Calculations Superseded (Number and Revision)  NONE			
M) Revision Summary ORIGINAL ISSUE STRING ANALYSIS AND STRUCTURAL ADEQUACY EVALUATION OF SUPPORTS AND THEIR ANCHORAGES OF DUCT/SUPPORT SYSTEMS IN STRINGS 2850-1, 2850-2 AND 2850-4 ARE PERFORMED. MODIFICATIONS TO 9 SUPPORTS ARE IDENTIFIED AND DESIGNED.  THIS CALCULATION IS PART OF THE BASIS FOR EDP-28147.			

**PART 2: PREPARATION, REVIEW, AND APPROVAL**

A) Prepared By Sign <i>E. ATAY</i>	Date <i>6/6/96</i>
B) Checked By Sign <i>R. Sam</i>	Date <i>6/7/96</i>
C) Verified By Sign <i>R. Sam</i>	Date <i>6/7/96</i>
D) Approved By (Includes PE approval) * Sign <i>E. Odar</i> <i>Abdul Alchaleb</i>	Date <i>6/7/96</i>

\* Approve attached PE, if applicable

Not Decommissioning Related

DTC: TPMMES DSN: MES15001 Rev: 0 P1/1 File: 1703.22 Date: 8-25-95 Recip: \_\_\_\_\_  
 DTC: TDPCAS  TDPELE  TDPINC  TDPMEC   
 DSN: *DC-5766* Rev: *0* File: 1801 IP: 1  
*vol I*

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ATTACHMENTS

ATTACHMENT A1: Computer Output for String Analysis  
System 2850-1

A.	D28501.O	61	Sheets
B.	FQ28501.O	161	Sheets
C.	O28501.O	38	Sheets
D.	S28501.O	38	Sheets
E.	PO28501.O	65	Sheets
F.	PS28501.O	65	Sheets

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 ATTACHMENT A2: Computer Output for String Analysis  
 System 2850-1 Initial Modification

A.	D28501.MO	61	Sheets
B.	FQ28501.MO	161	Sheets
C.	O28501.MO	38	Sheets
D.	S28501.MO	38	Sheets
E.	PO28501.MO	65	Sheets
F.	PS28501.MO	65	Sheets

 ATTACHMENT A3: Computer Output for String Analysis  
 System 2850-1 Final Modification

A.	D28501.FO	61	Sheets
B.	FQ28501.FO	198	Sheets
C.	O28501.FO	38	Sheets
D.	S28501.FO	38	Sheets
E.	PO28501.FO	65	Sheets
F.	PS28501.FO	65	Sheets

 ATTACHMENT A4: Computer Output for Stiffness Analysis  
 Supports 6M-2850-H-74 & 6M-2850-H-77

A.	H74.O	33	Sheets
B.	H77.O	33	Sheets

 ATTACHMENT A5: Computer Output for String Analysis  
 System 2850-2

A.	D28502.O	52	Sheets
B.	FQ28502.O	157	Sheets
C.	O28502.O	35	Sheets
D.	S28502.O	35	Sheets
E.	PO28502.O	53	Sheets
F.	PS28502.O	53	Sheets

 ATTACHMENT A6: Computer Output for String Analysis  
 System 2850-2 Modified Configuration

A.	D28502.O	51	Sheets
B.	FQ28502S.O	58	Sheets
C.	O28502.O	35	Sheets
D.	S28502.O	35	Sheets
E.	PO28502.O	30	Sheets
F.	PS28502.O	30	Sheets

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 ATTACHMENT A7: Computer Output for String Analysis  
 System 2850-4

A.	D28504.O	42	Sheets
B.	FQ28504.O	76	Sheets
C.	O28504.O	26	Sheets
D.	S28504.O	26	Sheets
E.	PO28504.O	33	Sheets
F.	PS28504.O	33	Sheets

Attachment B1 :	Computer Output for Support Analysis Support 6M-2850-H-76	93	Sheets
Attachment B2 :	Computer Output for Support Analysis Support 6M-2850-H-77, 4/22/96	148	Sheets
Attachment B3 :	Computer Output for Support Analysis Support 6M-2850-H-81	78	Sheets
Attachment B4 :	Computer Output for Support Analysis Support 6M-2850-H-87	83	Sheets
Attachment B5 :	Computer Output for Support Analysis Support 6M-2850-H-88	83	Sheets
Attachment B6 :	Computer Output for Support Analysis Support 6M-2850-1-H-11	78	Sheets
Attachment B7 :	Computer Output for Support Analysis Support 6M-2850-H-73	78	Sheets
Attachment B8 :	Computer Output for Support Analysis Support 6M-2850-H-77, 5/20/96	95	Sheets
Attachment B9 :	Computer Output for Support Analysis Support 6M-2850-H-49	121	Sheets
Attachment B10:	Computer Output for Support Analysis Support 6M-2850-H-54	120	Sheets
Attachment B11:	Computer Output for Support Analysis Support 6M-2850-H-59, 4/4/96	78	Sheets
Attachment B12:	Computer Output for <del>Support</del> Analysis Support 6M-2850-H59, 4/6/96	78	Sheets

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Attachment B13: Computer Output for Support Analysis Support 6M-2850-H-63	78	Sheets
Attachment B14: Computer Output for Support Analysis Support 6M-2850-H-50	120	Sheets
Attachment C1 : Computer Output for Anchor and Base Plate Analysis Support 6M-2850-H-72	25	Sheets
Attachment C2 : Computer Output for Anchor and Base Plate Analysis Support 6M-2850-H-50	39	Sheets
Attachment D1 : Pertinent Reference Sheets from RICO 2850-1, 2850-2 and 2850-4	24	Sheets
Attachment E1 : Duct properties System 2850-1	41	Sheets
Attachment E2 : Duct properties System 2850-2	50	Sheets
Attachment E3 : Duct properties System 2850-4	25	Sheets

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PURPOSE

Fermi 2 Control Center Heating, Ventilation and Air Conditioning, (CCHVAC), and Standby Gas Treatment System (SGTS) safety related seismic category I duct, duct supports and anchorages were revalidated to demonstrate their structural adequacy under the effects of applicable loads and their combinations in accordance with the requirements and acceptance criteria contained in Design Criteria No: Fermi-DC-76230-1, "CCHVAC Duct and Duct Support Qualification", (Ref. 1). This revalidation effort is a result of the commitments made by DECO to the USNRC staff, to provide resolution of all the issues on the CCHVAC structural aspects identified by the staff in their letter, dated December 7, 1995 (Ref. 2) and during subsequent meetings held on February 7, 1996 and March 1, 1996 in Bethesda, MD. The overall revalidation program, referred to as the "CCHVAC Concern Resolution Task" is an integrated effort which includes recalculation of system operating ( $P_0$ ) and maximum ( $P_A$ ) pressures for duct evaluations; determination of pertinent seismic response on duct, duct supports and anchorages; evaluation of duct and duct transition segment (Tees, Ys, elbows, etc.) structural adequacy; evaluation of support and anchorage structural adequacy; and documentation of the adequacy in calculations and/or identifying required hardware modifications which are issued for implementation in EDP-28147. Duct/support systems shown to require modification(s) are reevaluated with the modified configurations to demonstrate structural adequacy of the modified system.

Pressure calculations are contained in calculations DC-5744 and DC-5758, (Ref. 3 & 4). The results of structural evaluation of ducts, supports and anchorages are summarized in a series of calculations as listed below. Generally the results of ducts/support system evaluations are arranged based on the drawing series where they appear, to facilitate correlation.

NON RIGID DUCT/SUPPORT SYSTEMS:

STRING ANALYSES, SUPPORT AND ANCHORAGE EVALUATION

DC-5762      Generic Evaluation  
DC-5763      2268 Series  
DC-5764      2848 Series  
DC-5765      2849 and 4126 Series  
DC-5766      2850 Series  
DC-5767      4316 Series

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PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION

EFFECTIVE SECTION CALCULATIONS AND DUCT EVALUATIONS

DC-5768 Duct Skin Stress Evaluation Under Internal Pressure  
DC-5770 2268 and 2848 Series  
DC-5771 2849 and 4126 Series  
DC-5772 2850 Series  
DC-5773 4316 Series

RIGID DUCT/SUPPORT SYSTEM EVALUATIONS:

DC-5774 Rigid duct/supports (all)

EXTENT OF CONDITION EVALUATION OF OTHER SAFETY RELATED DUCT AND DUCT SUPPORTS:

DC-5769 2255, 4148 and 4355 Series



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PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION

SCOPE

This calculation contains the structural evaluation of duct/support systems 2850-1, 2850-2 and 2850-4 .

Included in this calculation is the duct string mathematical modeling, dynamic response analyses to develop seismic responses, static analyses to establish loads due to dead weight, and structural evaluation of supports and their anchorages. Due to the exceedance of allowable stresses identified in a few areas, this calculation also includes reevaluation of the above systems with the modified configurations to show structural adequacy of the modified systems, and the calculations to demonstrate conformance to the allowable stress limits of the modified components.

The string and support configuration data, as well as the data on the as installed anchorage configuration was obtained from the existing documents (Ref. 11 thru 17). Existing design change documents were also considered.

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PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION

METHODOLOGY

These duct/support systems are three of the twenty two systems classified as Non Rigid. The following is performed and documented in this calculation:

1. Review configuration and types of support.
2. Calculate rectangular duct section properties.
3. Evaluate horizontal and vertical stiffnesses of a selected support to validate acceptability of support springs contained in calculation DC-2789 (Ref. 11).
4. Validate acceptability of the duct/support string 2850-1, 2850-2 and 2850-4 models for analysis.
5. Model strings and perform response spectra modal analysis for three directional simultaneous application of OBE and SSE effects utilizing STARDYNE. Enveloped response spectra in three directions for OBE (4% damping) and SSE (7% damping) identified as El 677' are used as earthquake input for these rectangular duct systems. Digitized spectra are printed in the dynamic analysis printouts. The basis for the digitized spectra is provided in calculation DC-5762.
6. Select and evaluate bounding supports based on the calculated response loads and show structural adequacy.
7. Evaluate structural adequacy of critical welds for the supports.
8. Evaluate base plates and anchor bolts utilizing support member offsets on base plate as determined by field data contained in calculation DC-2833 ,DC-2834 ,DC-2836 .
9. For identified overstress conditions, determine constructible modifications and reevaluate the modified configurations to demonstrate structural adequacy of the modified systems and supports.
10. Summarize results.

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SUBJECT STRUCTURAL QUALIFICATION

**RESULTS AND CONCLUSIONS**

1. Member and weld stresses for the bounding case supports of systems 2850-1, 2850-2 and 2850-4 under the combined effects of dead load and seismic responses are within allowables with the exception of supports H-74 (2850-1), H-76 (2850-1), H-80 (2850-1), H-81 (2850-1) and H-60 (2850-2), H-61 (2850-2), H-63 (2850-2), H-64 (2850-2) which required modifications. Bracing members have been added to supports H-73, H-74, H-76, H-77, H-81, H-60, H-61, H-63, H-64 and the supports have been shown to be adequate.
2. Base plate and anchorage of systems 2850-1, 2850-2 and 2850-4 are structurally adequate with the exception of supports H-73 (2850-1), H-75 (2850-1), H-76 (2850-1), H-77 (2850-1) and H-60 (2850-2), H-61 (2850-2), H-62 (2850-2), H-63 (2850-2), H-64 (2850-2). After the above modifications (bracing members have been added) the base plates and anchorages have been shown to be adequate.
3. The bracing arrangements are included in EDP-28147.

It is concluded that, with modifications (see (D) MODIFICATIONS), the duct/support systems 2850-1, 2850-2 and 2850-4 are structurally adequate and meet the requirements and acceptance criteria of Design Criteria No. Fermi-DC-76230-1.

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PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION

REFERENCES

1. CCHVAC Duct and Duct Support Qualification, Fermi Design Criteria No. FERMI-DC-76230-1, Revision 0, dated 3/15/96.
2. NRC Letter Dated December 7, 1995, "Fermi 2 Control Center Heating, Ventilation and Air Conditioning (CCHVAC) System, including Safety Evaluation on the Same Subject".
3. DECo Calculation DC-5744, Rev. 0.
4. DECo Calculation DC-5758, Rev. 0.
5. AISC Manual of Steel Construction 7th Edition and 8th Edition.
6. AISC Specification for Allowable Stress Design of Single-Angle Member, included in the 9th Edition of AISC Manual.
7. DECo Calculation DC-2935, Revision 0, Design Methods - QAI Ductwork Supports.
8. DECo Specification 3071-226, Revision J, Purchase and Installation of Concrete Anchors.
9. DECo Calculation DC-5762, Rev. 0, Generic Evaluations for String Analysis & Duct Supports.
10. Design of Welded Structures, Blodgett-1972.
11. DECo Calculation DC-2833 for System 2850-1 .
12. DECo Calculation DC-2834 for System 2850-2 .
13. DECo Calculation DC-2836 for System 2850-4 .
14. Report No. RICO-2850-1 .
15. Report No. RICO-2850-2 .
16. Report No. RICO-2850-4 .
17. Drawing RICO 2850, REV. 15 .

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PROJECT DECO FERMI 2

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SUBJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

SUBJECT

COMPUTER DISCLOSURE SHEET (REF. FORM 5611)

Program used : STARDYNE - 4.0, MAY01/93

RE&C Prequalification : Yes

Analysis description : Static and dynamic analysis of duct/support  
system string no. 2850-2

Stiffness analysis of support H59

Run no.s : D28502.O Mar. 29, 1996 Static analysis  
FQ28502.O Mar. 29, 1996 Frequency analysis  
O28502.O Mar. 29, 1996 OBE dynamic analysis  
S28502.O Mar. 29, 1996 SSE dynamic analysis  
PO28502.O Mar. 29, 1996 Post processing for DL and OBE  
PS28502.O Mar. 29, 1996 Post processing for DL and SSE

H49.OUT Static analysis of support H49  
H54.OUT Static analysis of support H54  
H59.OUT Static analysis of support H59  
H59.OUT Stiffness analysis of support H59  
H63.OUT Static analysis of support H63

Program used : STARDYNE - 4.4, AUG03/95

RE&C Prequalification : Yes

Analysis description : Static and dynamic analysis of duct/support  
system string no. 2850-2

D28502.O Apr. 22, 1996 Modified Static analysis  
FQ28502S.O Apr. 22, 1996 Modified Frequency analysis  
O28502.O Apr. 22, 1996 Modified OBE dynamic analysis  
S28502.O Apr. 22, 1996 Modified SSE dynamic analysis  
PO28502.O Apr. 22, 1996 Modified Post processing for DL and OBE  
PS28502.O Apr. 22, 1996 Modified Post processing for DL and SSE

The computer outputs attached to this calculation have been reviewed, the input data checked, and the results approved for release. Input criteria for this analysis were established.

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COMPUTER DISCLOSURE SHEET (REF. FORM 5611)

Program used : STARDYNE - 4.4, AUG03/95  
 RE&C Prequalification : Yes  
 Analysis description : Static and dynamic analysis of duct/support system string no. 2850-1  
 Stiffness analysis of supports H74 & H77

Run no.s : D28501.O Static analysis  
 FQ28501.O Frequency analysis  
 O28501.O OBE dynamic analysis  
 S28501.O SSE dynamic analysis  
 PO28501.O Post processing for DL and OBE  
 PS28501.O Post processing for DL and SSE

D28501.MO Initial modification Static analysis  
 FQ28501.MO Initial modification Frequency analysis  
 O28501.MO Initial modification OBE dynamic analysis  
 S28501.MO Initial modification SSE dynamic analysis  
 PO28501.MO Initial modification Post processing for DL and OBE  
 PS28501.MO Initial modification Post processing for DL and SSE

D28501.FO Final modified Static analysis  
 FQ28501.FO Final modified Frequency analysis  
 O28501.FO Final modified OBE dynamic analysis  
 S28501.FO Final modified SSE dynamic analysis  
 PO28501.FO Final modified Post processing for DL and OBE  
 PS28501.FO Final modified Post processing for DL and SSE

H74.O Stiffness analysis of support H74  
 H77.O Stiffness analysis of support H77

H76.OUT Static analysis of support H76  
 H77.OUT Static analysis of support H77  
 H87.OUT Static analysis of support H87  
 H88.OUT Static analysis of support H88  
 H73.OUT Static analysis of support H73  
 H77.OUT Static analysis of support H77

The computer outputs attached to this calculation have been reviewed, the input data checked, and the results approved for release. Input criteria for this analysis were established.

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COMPUTER DISCLOSURE SHEET (REF. FORM 5611)

Program used : STARDYNE - 4.0, MAY01/93

RE&C Prequalification : Yes

Analysis description : Static and dynamic analysis of duct/support system string no. 2850-4

Run no.s : D28504.O Static analysis  
 FQ28504.O Frequency analysis  
 O28504.O OBE dynamic analysis  
 S28504.O SSE dynamic analysis  
 PO28504.O Post processing for DL and OBE  
 PS28504.O Post processing for DL and SSE

H50.OUT Static analysis of support H50

The computer outputs attached to this calculation have been reviewed, the input data checked, and the results approved for release. Input criteria for this analysis were established.

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COMPUTER DISCLOSURE SHEET (REF. FORM 5611)

Program used : STARDYNE - 4.0, MAY01/93

RE&C Prequalification : Yes

Analysis description : Static analysis of support  
System string no. 2850-1

H81.OUT Static analysis of support H81  
H11.OUT Static analysis of support H11

The computer outputs attached to this calculation have been reviewed, the input data checked, and the results approved for release. Input criteria for this analysis were established.



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COMPUTER DISCLOSURE SHEET (REF. FORM 5611)

Program used : PHI-DELTA STRUDL, version 0894

RE&C Prequalification : Yes

Analysis description : Base plate analysis

Run no.s : Job 00000081  
00000085

The computer outputs attached to this calculation have been reviewed, the input data checked, and the results approved for release. Input criteria for this analysis were established.

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 SUBJECT STRUCTURAL QUALIFICATION SYSTEM  
 2850-1, 2 & 4

(A) MODEL AND DATA FOR STRING ANALYSIS

<b>Raytheon</b> Engineers & Constructors	GENERAL COMPUTATION SHEET			CALCULATION SET NO DC-5766		REV 0	COMP. BY EA	CHK'D BY R. J.	
	PRELIM		FINAL		VOID		DATE 6/6/96	DATE 6/7/96	
PROJECT <u>DECO FERMI 2</u>				X		DATE		DATE	
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SUBJECT <del>STRUCTURAL QUALIFICATION</del> SYSTEM 2850-1, 2 & 4				JO 76230.502					

### STRING ANALYSIS

The string analysis STARDYNE computer runs consist of six individual runs as described below:

1. Frequency run is made using the STAR module. Frequencies up to 50 cps are extracted plus two missing mass correction modes, one for each of the two orthogonal horizontal directions. The mathematical model input data is echoed in the output.
2. Response spectra analysis runs are made using the DYNRE4 module. One run is performed using OBE response spectra and one run is performed using SSE spectra. Missing mass correction in two horizontal directions are included. Since the string is essentially rigid in the vertical direction, missing mass correction for the vertical direction is performed using a static procedure as described under dead load runs and post runs.

DYNRE4 utilizes the working files saved by the frequency run, therefore, only response spectra and control cards are echoed in the output.

3. Dead load run is made using the STAR module. Dead load, static vertical response at ZPA for OBE and static vertical response at ZPA for SSE are calculated. The mathematical model input data which is identical to the frequency run, with the exception of the loading cases, is echoed in the output.
4. Post runs are made using the POST module. One run is performed for OBE and one run is performed for SSE. Design values for member forces/moments and support loads are obtained from the post outputs.

Post utilizes the working files saved by the dead load run and the response spectra analysis runs, and output the following load cases:

- a) Dead load.
- b) Earthquake (OBE or SSE).
- c) Dead load + Earthquake, if requested.
- d) Dead load - Earthquake, if requested.

Missing mass correction in the vertical direction is accomplished by combining, using the SRSS method, the static vertical responses at ZPA with the respective response spectra analysis results. The output of the final member forces/moment and support loads are printed.

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The output contains the echo of the load cases selected and the combination method. The mathematical model input data which are echoed in the output of the "source" runs are not repeated .

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2850-1, 2 &amp; 4

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ANALYSIS MODEL

THE MODELS FOR STRING 2850-1, 2850-2 & 2850-4  
ARE BASED ON THE RICO STRESS REPORTS  
(EXCERPTS FROM THE REPORT ARE INCLUDED  
IN ATTACHMENT "D1")  
COMPUTER PLOT OF THE MODEL, PROPERTY,  
ADDITIONAL WEIGHT AND STIFFNESS CALCULATIONS  
ARE PRESENTED ON THE FOLLOWING SHEETS.

TH1 :

TH2 : 45.

EBASCO SERVICES, INC.

PREP. BY: R. Law 1/24/96

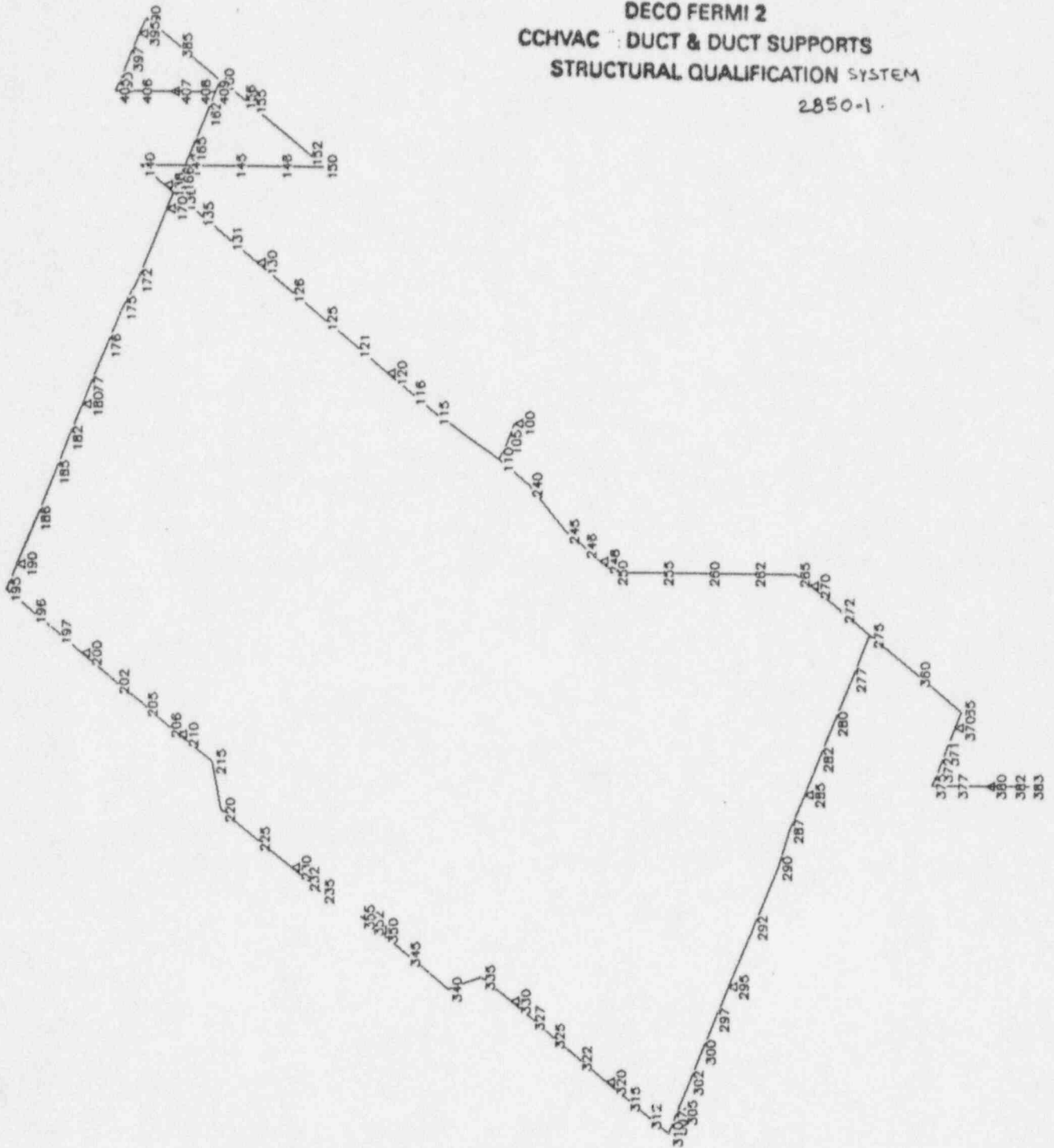
CHCK. BY: E. ATAY shg/96

STRING 2850-1

DC-5766

J.O. 76730.502

Fri May 24 15:04:02 1996



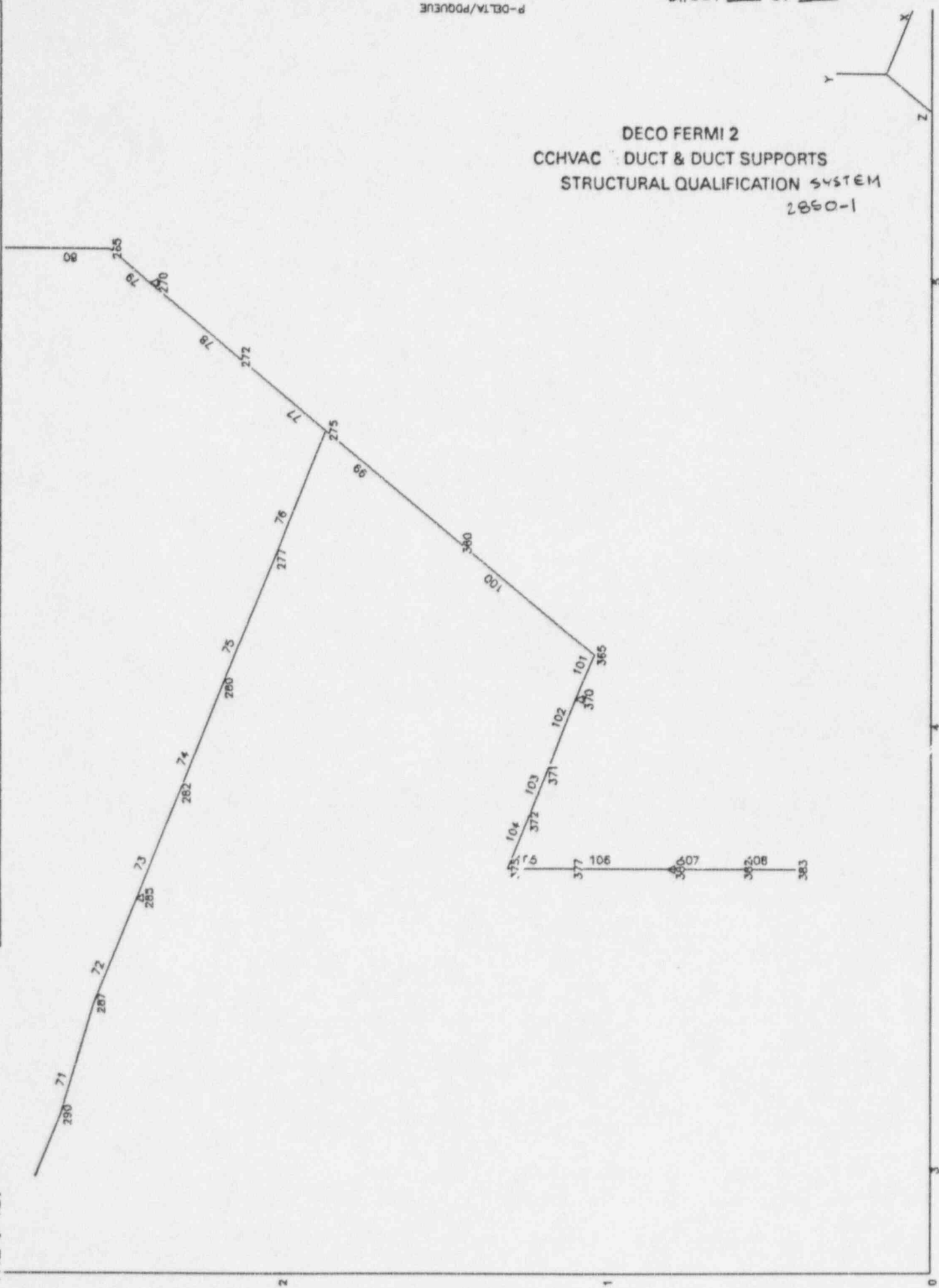
DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION SYSTEM  
 2850-1

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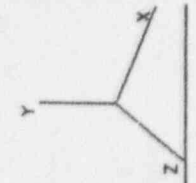
TH1 :  
 TH2 : 45.

EBASCO SERVICES, INC.  
 STRING 2850-1 DC-5766 J.O. 76230.502

PREP. BY: P. Cassin 5/24/96  
 CHCK. BY: S. Atkins/hvdg  
 Fri May 24 15:04:52 1996



DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION SYSTEM  
 2850-1



TH1 :

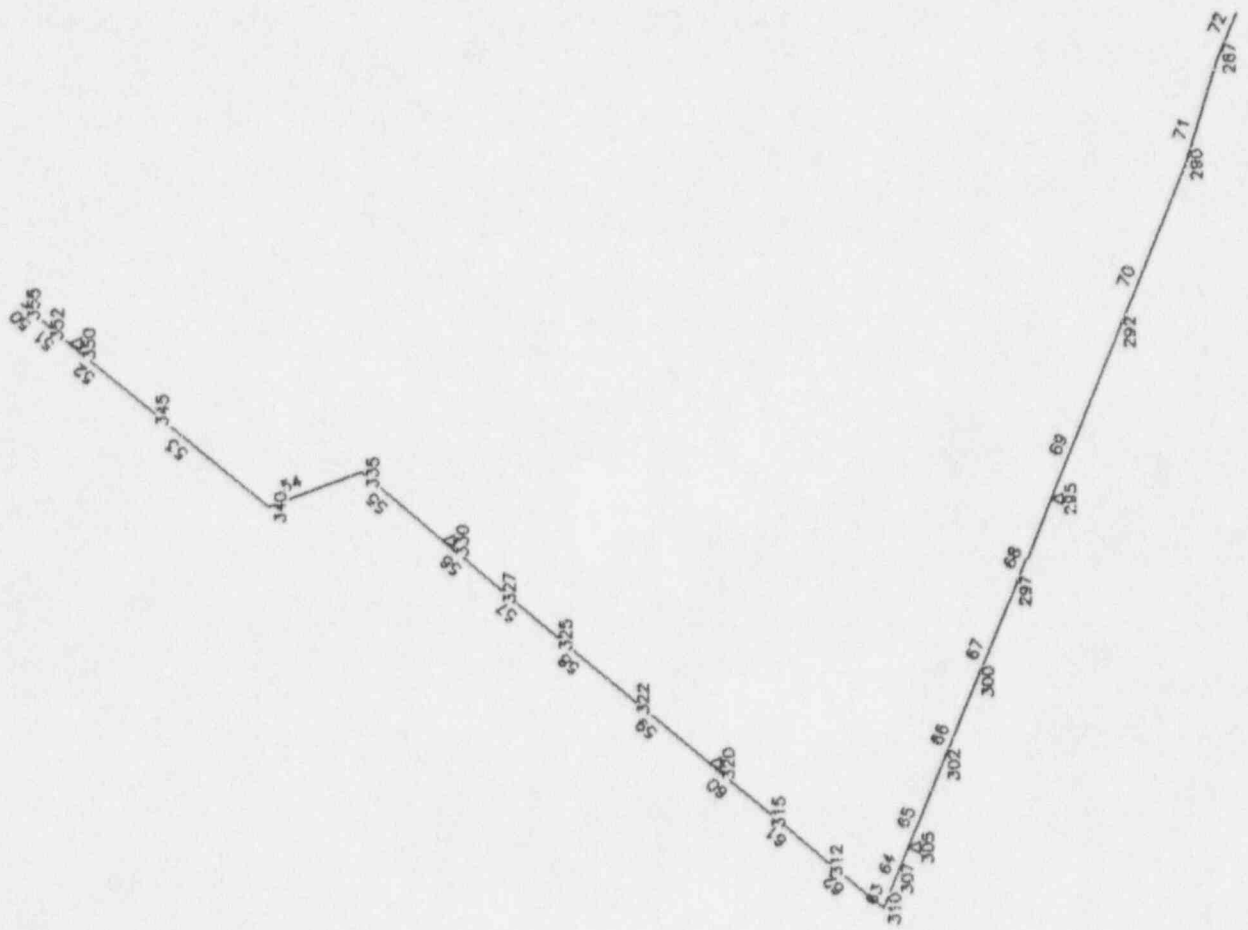
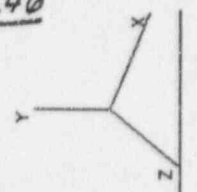
TH2 : 45.

EBASCO SERVICES, INC.	PREP. BY: <i>L. Bann</i>	CHCK. BY:	<i>E. ATAY</i>
STRING 2850-1 DC 5'166	J.O. 76230.502	Fri May 24 15:10:56 1996	

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 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION SYSTEM  
 2850-1



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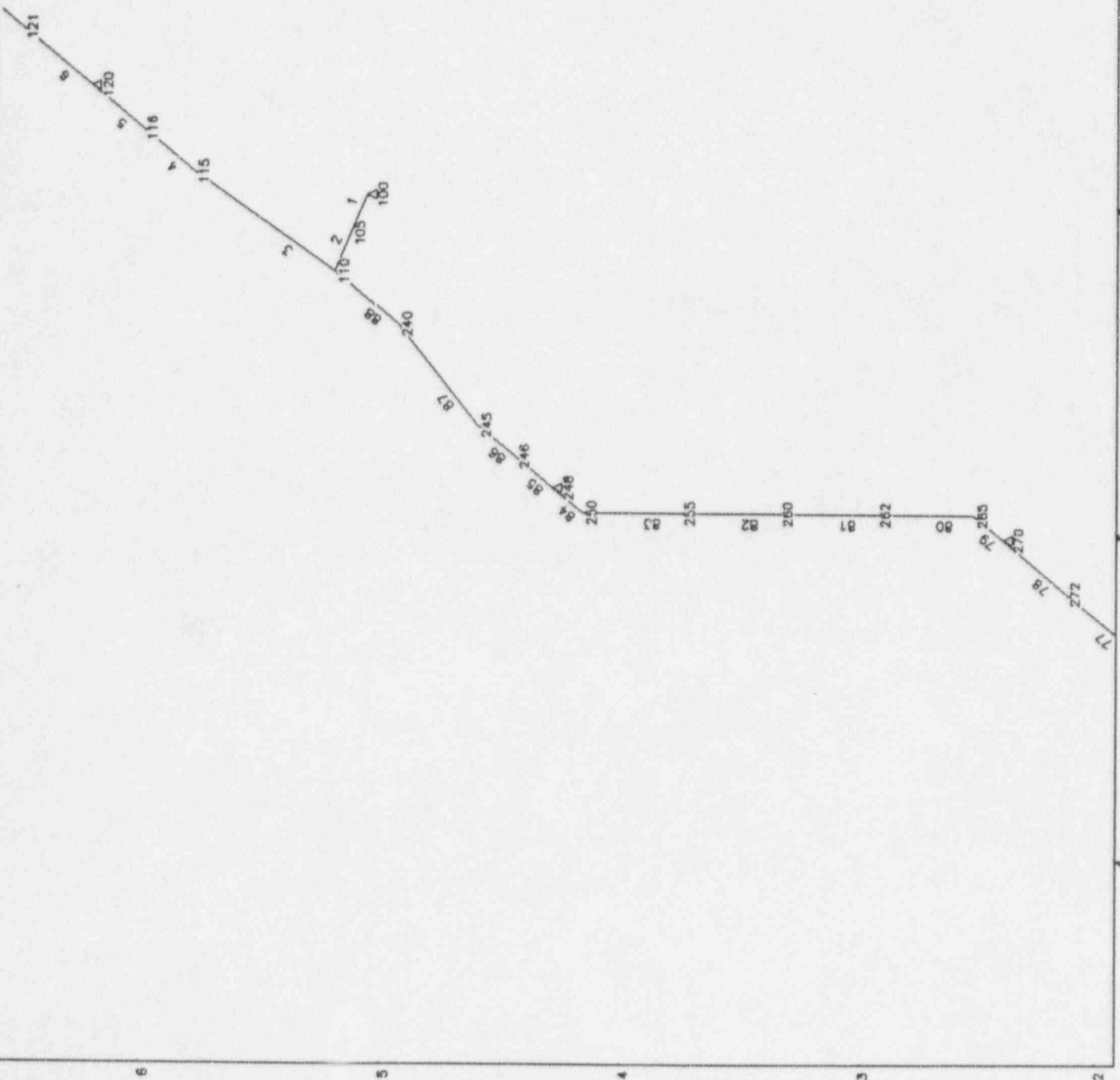
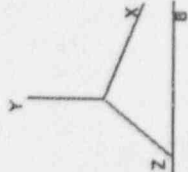
TH1 :  
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EBASCO SERVICES, INC.	PREP. BY: <i>J. O. 76230 .50Z</i>	CHCK. BY: <i>EATY</i>
STRING 2850-1 DC-5766	J.O. 76230 .50Z	5/24/96
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 STRUCTURAL QUALIFICATION SYSTEM  
 2850-1



TH1 :

TH2 : 45.

EBASCO SERVICES, INC.

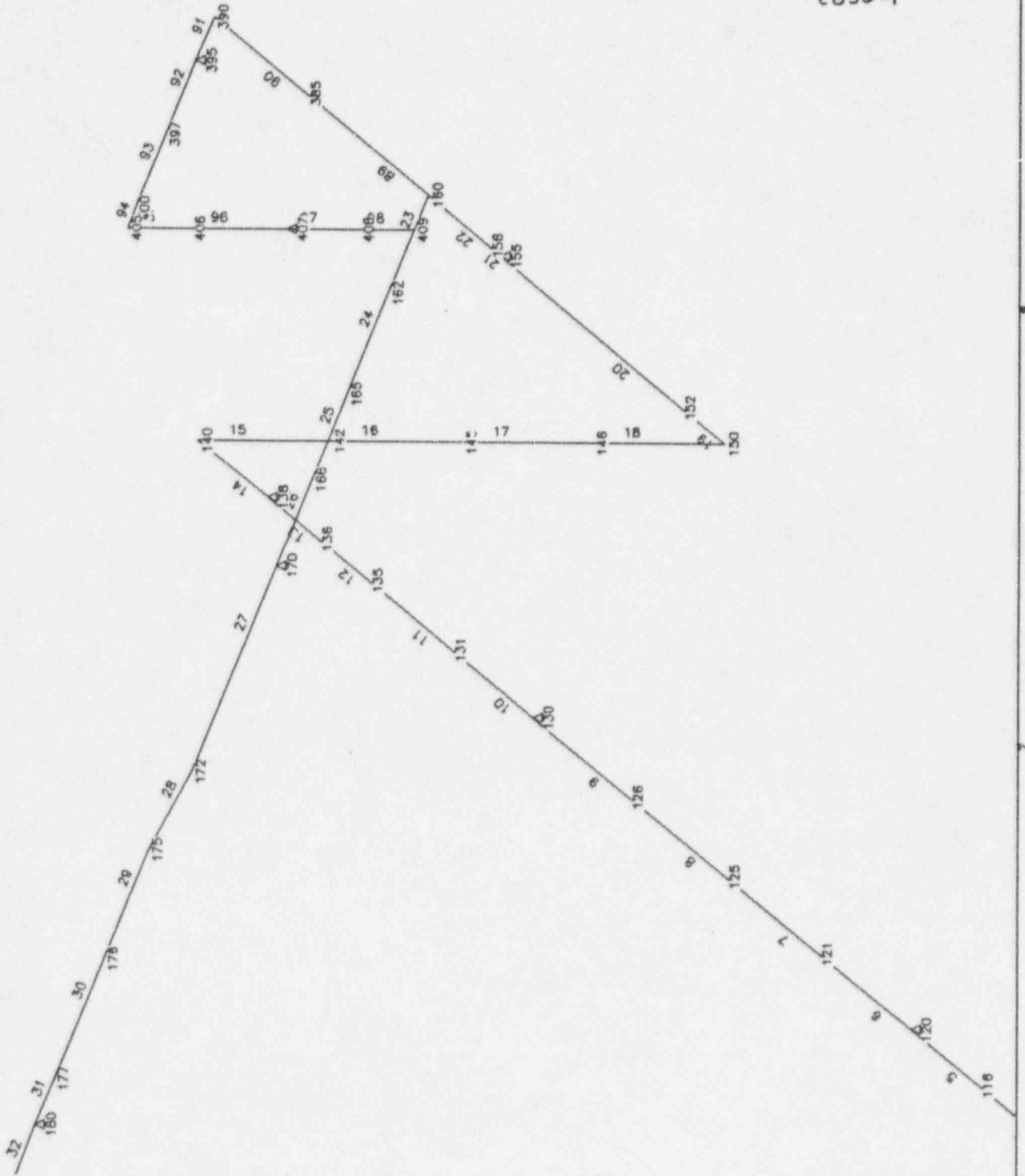
PREP. BY: *E. Carr* 1/24/96

CHCK. BY: *E. ATAY* 5/24/96

STRING 2850-1 DC-5766

J.O. 76230.502

Fri May 24 15:12:40 1996



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 STRUCTURAL QUALIFICATION SYSTEM  
 2850-1

TH1 :

TH2 : 45.

EBASCO SERVICES, INC.

STRING 2850-1 DC-5766

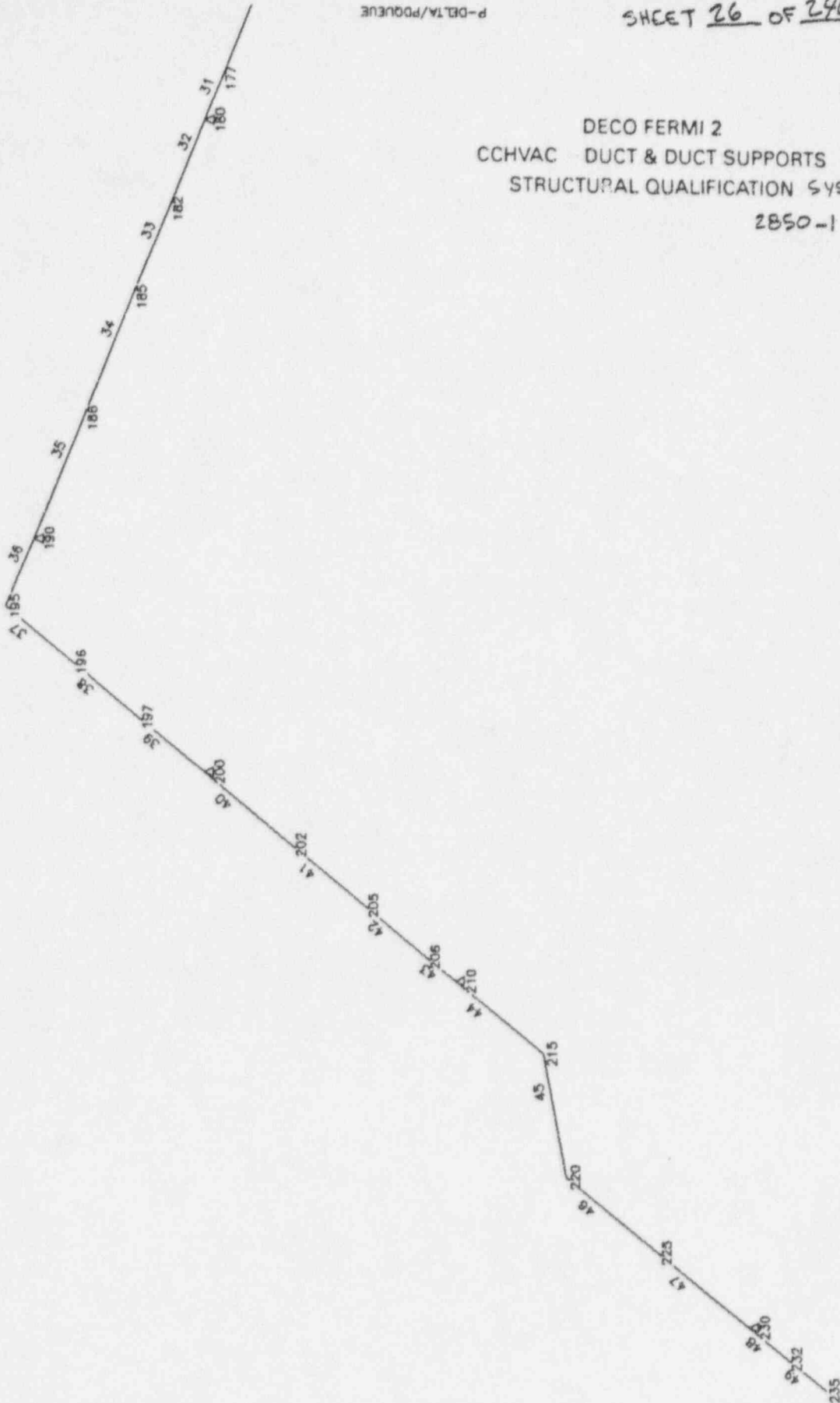
PREP. BY: R. Carr

J.O. 762230-50Z

CHCK. BY:

Fri May 24 15:13:52 1996

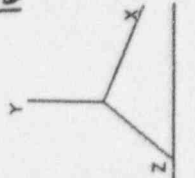
E. ATAY  
5/24/96



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DECO FERMI 2  
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2850-1



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PROJECT

DECO FERMI 2

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STRUCTURAL QUALIFICATION SYSTEM  
2850-1

MATLG

DUCT SIZE	#	(A) 1b/ft DUCT + INSUL =	(B) ft <sup>2</sup> EFFECTIVE AREA	(A)/(B) DENSITY
34 x 20	1	19.44 + 6.75 = 26.19	0.02005	1306.234
28 x 14	2	15.12 + 5.25 = 20.37	0.01611	1264.432
24 x 18	3	15.12 + 5.25 = 20.37	0.01607	1267.579
24 x 14	4	13.68 + 4.75 = 18.43	0.01479	1246.112
20 x 18	5	13.68 + 4.75 = 18.43	0.01474	1250.339
20 x 14	6	12.24 + 4.25 = 16.49	0.01340	1230.597
16 x 20	7	12.96 + 4.50 = 17.46	0.01408	1240.057
16 x 14	8	10.80 + 3.75 = 14.55	0.01206	1206.472
15 x 15	9	10.80 + 3.75 = 14.55	0.01205	1207.468
15 x 8	10	8.28 + 2.88 = 11.16	0.00976	1143.443
12 x 12	11	8.64 + 3.00 = 11.64	0.01005	1158.209
10 x 8	12	6.48 + 2.25 = 8.73	0.00804	1085.821
8 x 14	13	7.92 + 2.75 = 10.67	0.00958	1113.779

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	DUCT SIZE	A (ft <sup>2</sup> )	A3 (ft <sup>2</sup> )	A2 (ft <sup>2</sup> )	A3/A	A2/A
1	34 x 20	0.020050	0.01328	0.02257	0.662344	1.125686
2	28 x 14	0.016110	0.009294	0.01859	0.576909	1.153942
3	24 x 18	0.016070	0.01195	0.01593	0.743622	0.991288
4	24 x 14	0.014790	0.009294	0.01593	0.628398	1.077079
5	20 x 18	0.014740	0.01195	0.01328	0.810719	0.900950
6	20 x 14	0.013400	0.009294	0.01328	0.693582	0.991
7	16 x 20	0.014080	0.01328	0.01062	0.943182	0.754261
8	16 x 14	0.012060	0.009294	0.01062	0.770647	0.880597
9	15 x 15	0.012050	0.009958	0.009958	0.826390	0.826390
10	15 x 8	0.009766	0.005311	0.009958	0.543826	1.01966
11	12 x 12	0.010050	0.007967	0.007967	0.792736	0.792736
12	10 x 8	0.008036	0.005311	0.006639	0.660901	0.826157
13	8 x 14	0.009578	0.009294	0.005311	0.970349	0.554499

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PROJECT DECO FERMI 2  
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 SUBJECT STRUCTURAL QUALIFICATION SYSTEM  
2850-1

STIFFNESS (LB/FT)

HANGER	NODE	X (EW)	Y (VERT)	Z (NS)
6M-2849-H-57	130	91440	12 812 988	-
58	120	106 416	12 834 216	-
61	170	-	4 739 340	10 944
62	180	-	4 739 904	9 360
6M-2850-H-73	155	60 048	7 716 540	-
74	190	-	7 716 048	3 5472
75	200	35472	7 716 048	-
76	210	4 572	3 740 184	-
77	270	95 076	7 704 648	-
78	370	5 844	1 190 712	11 078 796
79	285	-	7 703 664	52 884
80	295	-	3 744 960	62 64
81	305	-	6 714 036	37 548
82	320	35 472	7 716 048	-
83	330	4 572	3 740 184	-
84	138	24 672	12 797 268	-
85	248	23 988	12 797 268	-
86	407	102 600	2 8 560	18 066 852
87	380	72 168	18 804	14 443 908
88	395	107 904	106 704	19 679 460
6M-2850-1-H-3	230	4 332	3 849 300	-
- 11	350	12 947 772	19 272	-
6M-2850-H-57	100	4 712 904	28 011 204	408 960
ROTATION	100	5.5558 x 10 <sup>7</sup>	1.0966 x 10 <sup>7</sup>	1.5042 x 10 <sup>6</sup>

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 SUBJECT CCHVAC DUCT & DUCT SUPPORTS  
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 2850-1

<u>NODE</u>	<u>WT</u>
110	72
115	35
135	92
145	75
150	30
160	86
175	50
185	59
195	30
205	42
215	38
240	61
250	65.5
260	50
265	32
275	92
280	27
290	85
300	53
310	30
325	42
335	37.4
345	37
360	36
365	25
385	36
390	25

<u>NODE</u>	<u>WT</u>
100	138
120	44
130	44
138	44
155	196
170	115
180	115
190	181
200	181
210	79
230	99
248	46
270	224
285	214
295	84
305	183
320	181
330	79
350	37
370	65
380	88
395	102
400	-
407	77

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				DATE	DATE

HANGER 6M-2849-61 (NODE 170)

$$L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4} : \left[ \frac{5.833}{2} + 2(14.77) + 2(2.11) + 2(2.69) \right] 4.1$$

$$\cong 173\#$$

$$@ \text{NODE 170} : \frac{2}{3} \times 173 = 115\#$$

HANGER 6M-2849-62 (NODE 180)

SAME AS H-61

$$@ \text{NODE 180} : 115\#$$

HANGER 6M-2850-73 (NODE 155)

$$L 4 \times 4 \times \frac{1}{4} : \left[ 2(14.88) + 3(3.33) + 2.69 + 2.02 \right] 6.6$$

$$= 294\#$$

$$@ \text{NODE 155} : \frac{2}{3} \times 294 = 196\#$$

HANGER 6M-2850-74 (NODE 190)

$$L 4 \times 4 \times \frac{1}{4} : \left[ 2(14.88) + 2(2.02) + 3(2.42) \right] 6.6$$

$$= 271\#$$

$$@ \text{NODE 190} : \frac{2}{3} \times 271 = 181\#$$

REF

DWG  
10-4221-000  
6M-2849-  
61-A

DWG  
2849-62-A

DWG  
2850-73-A

DWG  
2850-74-A



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REF

HANGER 6M-2850-75 (NODE 200)

Same as H-74

$$\text{@ NODE 200 : } \frac{2}{3} \times 271 = 181\#$$

HANGER 6M-2850-76 (NODE 210)

$$L 2 \times 2 \times 1/4 : [2(14.71) + 2(1.35) + 3(1.71)] 3.19$$

$$= 119\#$$

$$\text{@ NODE 210 : } \frac{2}{3} \times 119 = 79\#$$

HANGER 6M-2850-77 (NODE 270)

$$L 4 \times 4 \times 1/4 : [2(14.71) + 3(3.81) + 2(3.02) + 3+1] 6.6$$

$$= 336\#$$

$$\text{@ NODE 270 : } \frac{2}{3} \times 336 = 224\#$$

HANGER 6M-2850-78 (NODE 370)

$$L 2\frac{1}{2} \times 2\frac{1}{2} \times 1/4 : (2(6.46) + 2(1.10) + 7.83 + 1^{CUR}) 4.1$$

$$= 98\#$$

$$\text{@ NODE 370 : } \frac{2}{3} \times 98 = 65\#$$

HANGER 6M-2850-79 (NODE 285)

$$L 4 \times 4 \times 1/4 : [2(14.88) + 3(2.71) + 2(4.36) + 2] 6.6$$

$$= 321\#$$

$$\text{@ NODE 285 : } \frac{2}{3} \times 321 = 214\#$$

DWG  
6M-2850-75DWG  
6M-2850-76DWG  
6M-2850-77-BDWG  
6M-2850-78DWG  
6M-2850-79-B

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HANGER 6M-2850-81 (NODE 305)

$$L 3 \times 3 \times 1/4 : [2(14.79) + 2(1.85) + 8.33 + 3(2.46) + 7] 4.9$$

$$= 274\#$$

$$@ \text{NODE } 305 : \frac{2}{3} \times 274 = 183\#$$

HANGER 6M-2850-80 (NODE 295)

$$L 2 \times 2 \times 1/4 : [2(14.71) + 2(1.69) + 3(2.17)] 3.19$$

$$= 125\#$$

$$@ \text{NODE } 295 : \frac{2}{3} \times 125 = 84\#$$

HANGER 6M-2850-82 (NODE 320)

$$L 4 \times 4 \times 1/4 : [2(14.88) + 2(2.02) + 3(2.42)] 6.6$$

$$= 271\#$$

$$@ \text{NODE } 320 : \frac{2}{3} \times 271 = 181\#$$

HANGER 6M-2850-83 (NODE 330)

$$L 2 \times 2 \times 1/4 : [2(14.71) + 2(1.35) + 3(1.71)] 3.19$$

$$= 119\#$$

$$@ \text{NODE } 330 : \frac{2}{3} \times 119 = 79\#$$

HANGER 6M-2850-84 (NODE 138)

$$L 2\frac{1}{2} \times 2\frac{1}{2} \times 1/4 : [2(5.58 + 2.44)] 4.1$$

$$= 66\#$$

$$@ \text{NODE } 138 : \frac{2}{3} \times 66 = 44\#$$

DWG

6M-2850-81A

DWG

6M-2850-80

DWG

6M-2850-82

DWG

6M-2850-83

DWG

6M-2850-84

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HANGER 6M-2850-1-11 (NODE 350)

$$L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4} : [2(5.44 + 1.27)] \times 4.1$$

$$= 55\#$$

$$@ \text{NODE 350} : \frac{2}{3} \times 55 = 37\#$$

HANGER 6M-2850-1-3 (NODE 230)

$$L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4} : [4.08 + \frac{4.54}{2} + 1.5 + 14.75 + 10.94 + 2(1.29)] \times 4.1$$

$$= 148\#$$

$$@ \text{NODE 230} : \frac{2}{3} \times 148 = 99\#$$

HANGER 6M-2849-57-A (NODE 130)

$$L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4} : 2[5.58 + 2.44] \times 4.1 = 66\#$$

$$@ \text{NODE 130} : \frac{2}{3} \times 66 = 44\#$$

HANGER 6M-2850-57 (NODE 100)

$$L 3 \times 3 \times \frac{1}{4} : [2(5.63) + 3.35 + 2.85 + 2(1) + 2(6) + 5.42 + 5.38] \times 4.9$$

$$= 207\#$$

$$@ \text{NODE 100} : \frac{2}{3} \times 207 = 138\#$$

HANGER 6M-2849-58 (NODE 120)

$$L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4} : [2(5.58) + 2(2.44)] \times 4.1$$

$$= 66\#$$

$$@ \text{NODE 120} : \frac{2}{3} \times 66 = 44\#$$

 DWG  
 6M-2850-1-11-A

 DWG  
 6M-2850-1-3

 DWG  
 6M-2849-57-A

 DWG  
 6M-2850-57-B

 DWG  
 6M-2849-58

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PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION SYSTEM  
2850-1

REF

HANGER 6M-2850-85 (NODE 248)

$$L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4} : [2(5.58) + 2(2.77)] 4.1 = 69\#$$

$$@ \text{NODE 248} : \frac{2}{3} \times 69 = 46\#$$

DWG  
6M2850-85

HANGER 6M-2850-87 (NODE 380)

$$L 4 \times 4 \times \frac{1}{4} : [2(6.58) + 2(1.94) + 1.96] 6.6 = 125$$

$$L 2 \times 2 \times \frac{1}{4} : 2(1) \times 3.19 = \frac{7}{132}$$

$$@ \text{NODE } \underline{\hspace{1cm}} : \frac{2}{3} \times 132 = 88\#$$

DWG  
6M2850-87-B

HANGER 6M-2850-88 (NODE 395)

$$L 4 \times 4 \times \frac{1}{4} : [2(5.92) + 2(1.5) + 6.17 + 1] 6.6 = 145$$

$$L 2 \times 2 \times \frac{1}{4} : [4(0.58)] \times 3.19 = \frac{8}{153\#}$$

$$@ \text{NODE } \underline{\hspace{1cm}} : \frac{2}{3} \times 153 = 102\#$$

DWG  
6M2850-88

HANGER 6M-2850-86 (NODE 407)

$$L 4 \times 4 \times \frac{1}{4} : [2(5.98) + 2(1.69)] 6.6 = 102$$

$$L 3 \times 3 \times \frac{1}{4} : (1.96) 4.9 = 10$$

$$L 2 \times 2 \times \frac{1}{4} : (1) 3.19 = \frac{3}{115}$$

$$@ \text{NODE } \underline{\hspace{1cm}} : \frac{2}{3} \times 115 = 77\#$$

DWG  
6M-2850-86B

HANGER 6M-2850-47 (NODE 400)

N/A

**Raytheon**  
 Engineers & Constructors

 GENERAL  
 COMPUTATION  
 SHEET

CALCULATION SET NO			REV	COMP BY	CHK'D BY
DC-5766			0	R. Laverne	N. Yurgen
PRELIM	FINAL	VOID		DATE	DATE
	✓			5/17/96	6/1/96
SHEET 36 OF 246				DATE	DATE
JO 76230.502					

 PROJECT DECO FERMI 2  
 SUBJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-1
SYSTEM 2850-1 HANGER H-74 STIFFNESS

THE PURPOSE OF THIS EVALUATION IS TO DETERMINE THE PROPORTION OF THE LATERAL STIFFNESS TO THE VERTICAL STIFFNESS THAT CAN BE ASSUMED WERE A LATERAL BRACE IS ADDED GLOBALLY TO THE HANGER. IN THIS CASE A GLOBAL BRACE IN THE LATERAL DIRECTION IS PROPOSED FOR HANGER H74.

LATERAL STIFFNESS

 LATERAL LOAD @ NODE 7 & 8  $P_H = 500 \#$ 

 LATERAL DISPL. @ NODE 7  $\delta_{x7} = 0.01087"$ 

 LATERAL DISPL. @ NODE 8  $\delta_{x8} = 0.01123"$ 

$$\begin{aligned} \text{AVERAGE LATERAL DISPL.} \quad \delta_{xAVG} &= \frac{\delta_{x7} + \delta_{x8}}{2} \\ &= \frac{0.01087 + 0.01123}{2} \\ \delta_{xAVG} &= 0.01106" \end{aligned}$$

 LATERAL STIFFNESS  $K_x = \frac{2P_H}{\delta_{xAVG}}$ 

$$K_x = \frac{1000}{0.01106} = 90416 \#/\text{in} = 1084991.0 \#/\text{in}$$

VERTICAL STIFFNESS

 VERTICAL LOAD @ NODE 6 & 7  $P_V = 500 \#$ 

 VERTICAL DISPL. @ NODE 6  $\delta_{y6} = 0.001515"$ 

 VERTICAL DISPL. @ NODE 7  $\delta_{y7} = 0.00159"$ 

 AVG. VERTICAL DISPLACEMENT  $\delta_{yAVG} = \frac{\delta_{y6} + \delta_{y7}}{2}$ 

$$\delta_{yAVG} = \frac{0.001515 + 0.00159}{2} = 0.00155"$$

$$\begin{aligned} \text{VERTICAL STIFFNESS} \quad K_y &= \frac{2P_V}{\delta_{yAVG}} = \frac{1000}{0.00155} = 645161 \#/\text{in} \\ &= 7741935.5 \#/\text{in} \end{aligned}$$

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DATE  
5/17/96

DATE  
4/9/96

SHEET 37 OF 246

JO 76230.502

PROJECT

DECO FERMI 2

SUBJECT

CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION SYSTEM  
2850-1

STRUCTL MODEL

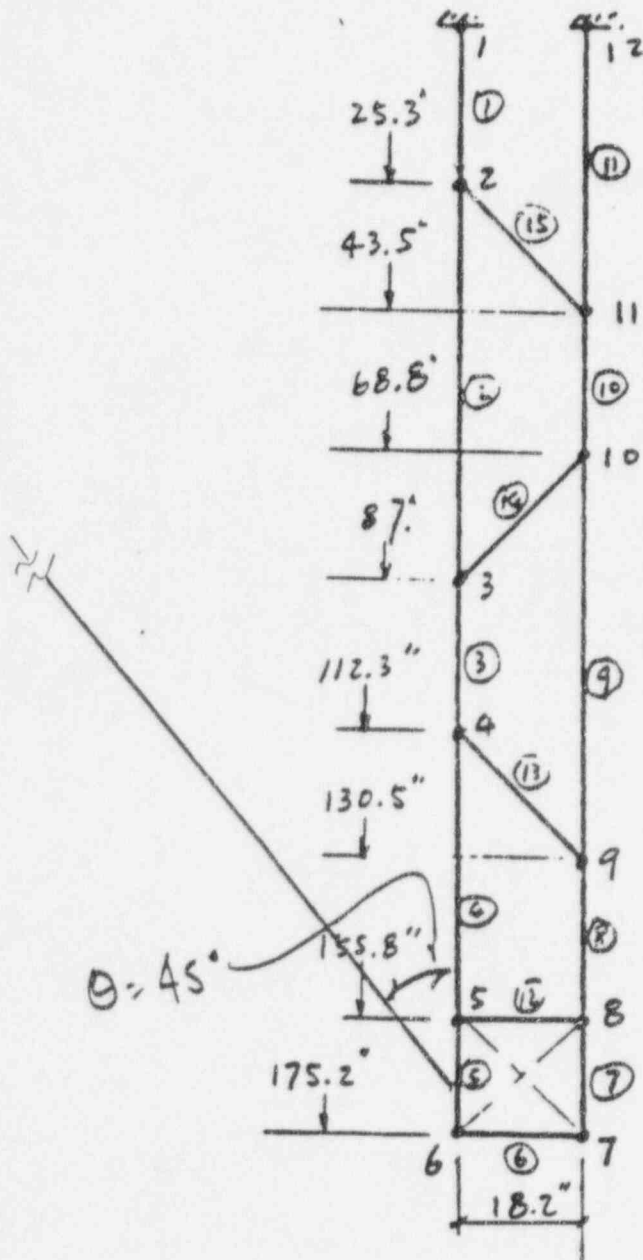
FOR GM-2850-H-74

REF. CALC.

DC-2833

VOL 1 REV B

PAGE 44 OF 100



ALL MEM: L4x4x4

**Raytheon**  
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 GENERAL  
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 SHEET

CALCULATION SET NO.

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 5/17/96

 NYURER  
 DATE  
 5/17/96

PROJECT

DECO FERMI 2

SUBJECT

CCHVAC DUCT &amp; DUCT SUPPORTS

 STRUCTURAL QUALIFICATION SYSTEM  
 2850-1

SHEET 39 OF 246

JO 76230.502

DATE

DATE

SYSTEM 2850-1 - HANGER H77 STIFFNESS

REF STADBYE COMPUTER OUTPUT "H77.0" 5/17/96

LATERAL STIFFNESS

 LATERAL LOAD APPLIED AT NODE 9  $P_1 = 500 \#$ 

 LATERAL LOAD APPLIED AT NODE 11  $P_2 = 500 \#$ 

 LATERAL DISPLACEMENT AT NODE 9  $\delta_{9x} = 0.0930858"$ 

 LATERAL DISPLACEMENT AT NODE 11  $\delta_{11x} = 0.07783263"$ 

 AVERAGE LATERAL DISPLACEMENT  $\delta_{AVGX} = \frac{\delta_{9x} + \delta_{11x}}{2}$ 

$$\delta_{AVGX} = \frac{0.0930858 + 0.07783263}{2}$$

$$\delta_{AVGX} = 0.0854592$$

$$\text{LATERAL STIFFNESS } K_x = \frac{P_1 + P_2}{\delta_{AVGX}} = \frac{1000}{0.0854592} = 11701.5 \#/\text{in}$$

$$= 140417.9 \#/\text{ft}$$

VERTICAL STIFFNESS

 VERTICAL LOAD APPLIED AT NODE 7  $P_1 = 500 \#$ 

 VERTICAL LOAD APPLIED AT NODE 9  $P_2 = 500 \#$ 

 VERTICAL DISPLACEMENT AT NODE 7  $\delta_{7y} = 0.0015553"$ 

 VERTICAL DISPLACEMENT AT NODE 9  $\delta_{9y} = 0.0015580"$ 

 AVERAGE VERTICAL DISPLACEMENT  $\delta_{AVGY} = \frac{\delta_{7y} + \delta_{9y}}{2}$ 

$$\delta_{AVGY} = \frac{0.0015553 + 0.0015580}{2}$$

$$\delta_{AVGY} = 0.0015567$$

$$\text{VERTICAL STIFFNESS } K_y = \frac{P_1 + P_2}{\delta_{AVGY}} = \frac{1000}{0.0015567} = 642405 \#/\text{in}$$

$$= 7708862 \#/\text{ft}$$

TH1 : 0

TH2 : 0.

EBASCO SERVICES, INC.

PREP. BY: *R. Quil 5/17/96*

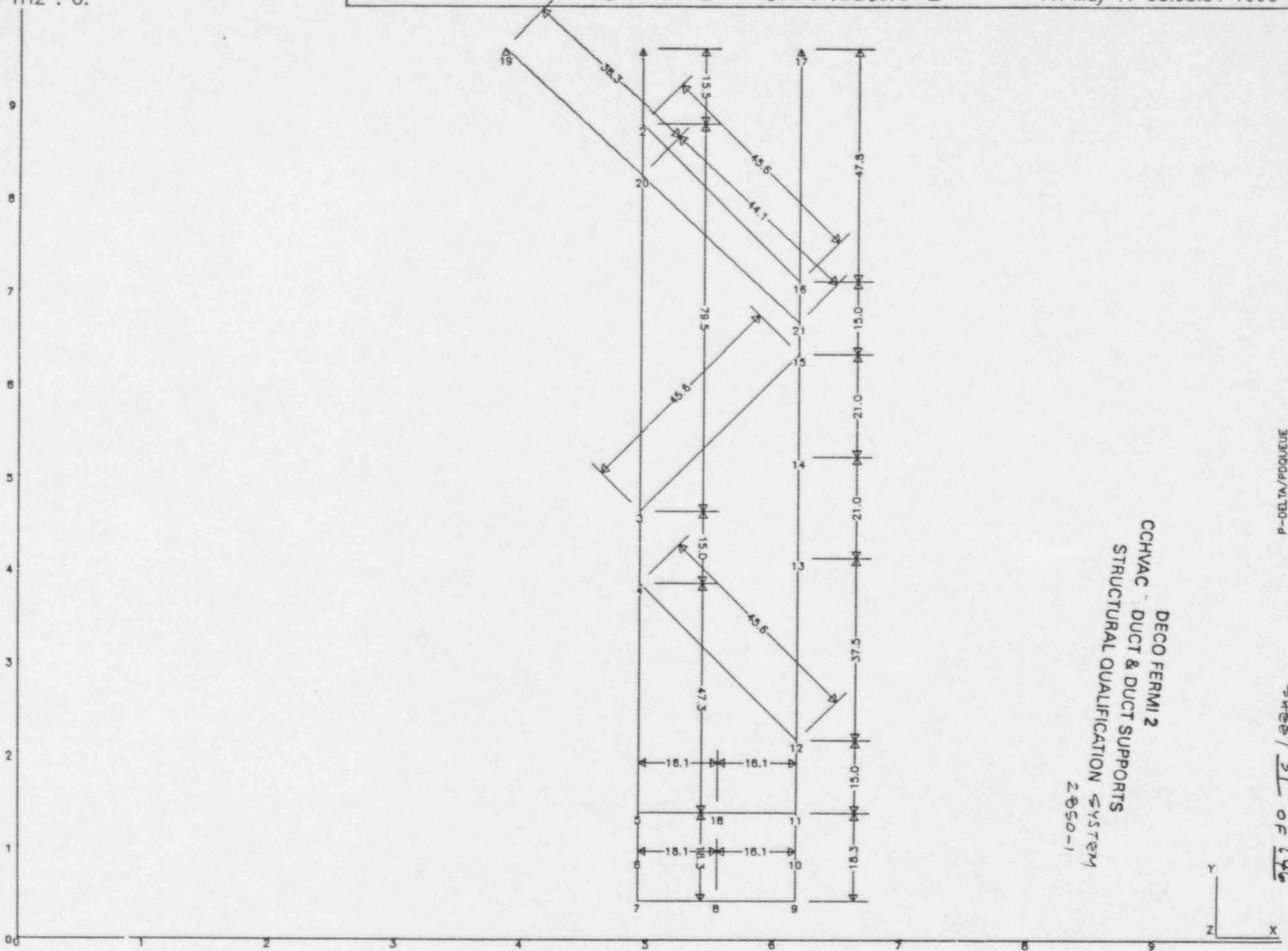
CHCK. BY: *N.Y. TER/C-17.96*

2850-1-H77

DL-5766

J.O. 76230.502

Fri May 17 08:05:34 1996



DECO FERM 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION SYSTEM  
 2850-1

P-DELTA/POQUEUE

Sheet 39 of 246





TH1 :  
TH2 : 0.

EBASCO SERVICES, INC.  
2850-1-H77 PC-5766

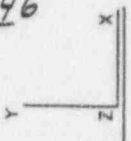
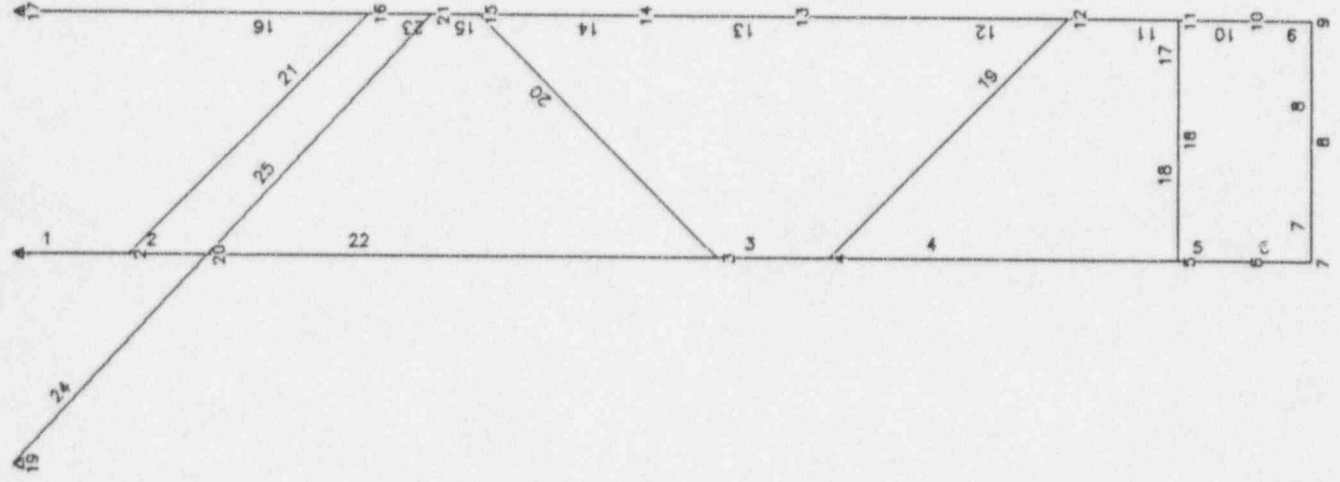
PREP. BY: R. Lewis 5/17/96  
S.O. 76230.502

CHCK. BY: M. J. EFER 5-17-96  
Fri May 17 08:04:26 1996

P-DELTA/PDQUEUE

SHEET 40 OF 246

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-1



00 1 2 3 4 5 6 7 8 9

TH1 : 0

TH2 : 0.

EBASCO SERVICES, INC.

PREP. BY: R. Cant 5/17/96

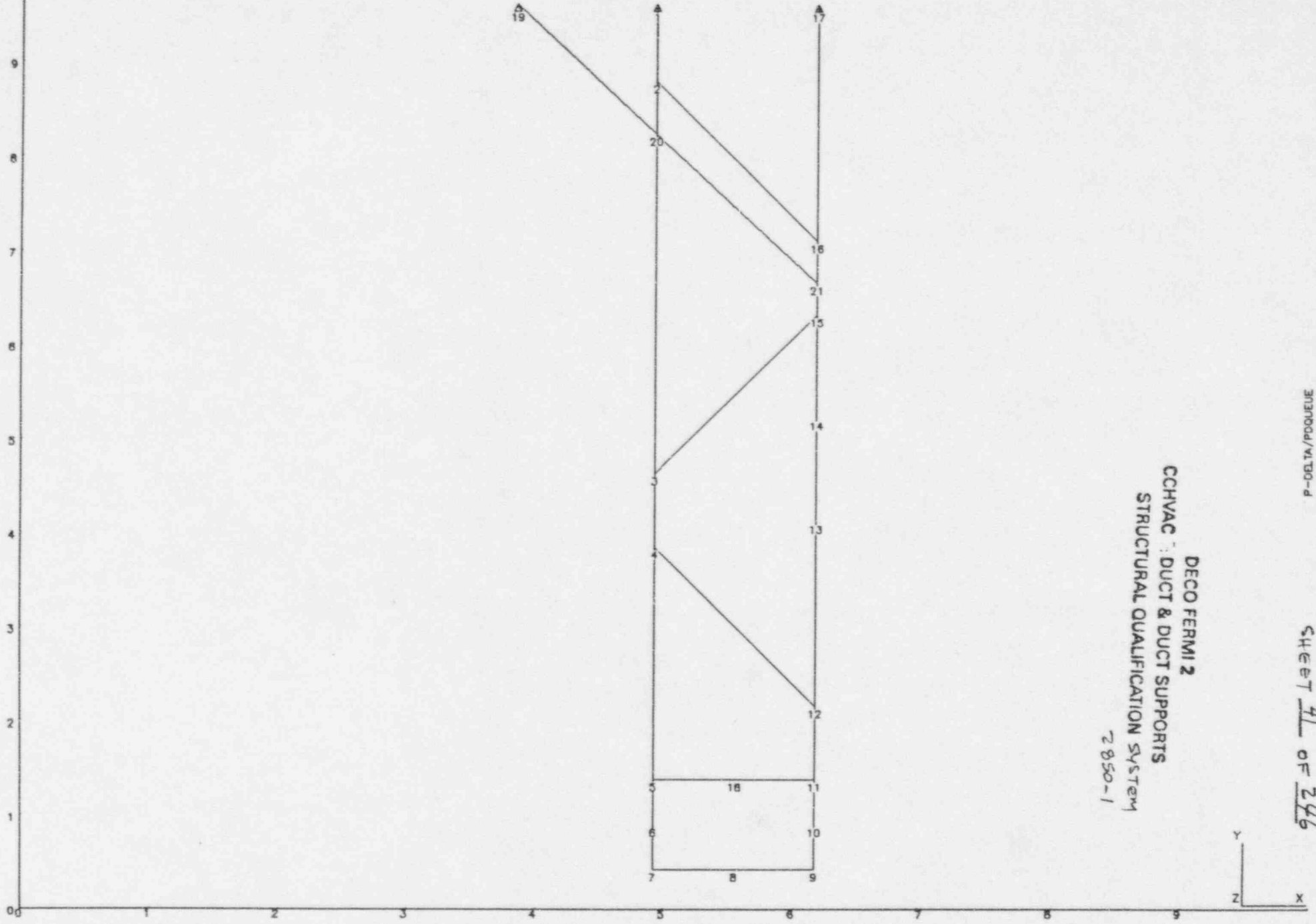
CHK. BY: N. W. 5/17/96

2850-1-H77

DC-5766

J.O. 76230.502

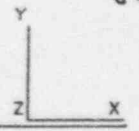
Fri May 17 08:03:56 1996



DECO FERMI 2  
 CCHVAC : DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION SYSTEM  
 2850-1

P-061A/POQUEUE

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**Raytheon**  
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GENERAL  
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6/1/96

6/7/96

PROJECT DECO FERMI 2

CCHVAC DUCT &amp; DUCT SUPPORTS

SUBJECT STRUCTURAL QUALIFICATION SYSTEM  
2850-2

SHEET 42 OF 246

DATE

DATE

JO 76230.502

IN ORDER TO QUALIFY OVER STRESSED ANCHORAGE,  
FIELD MODIFICATIONS ARE INTRODUCED.

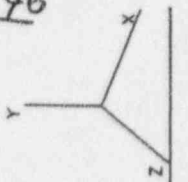
TO JUSTIFY THE PROPOSED MOD'S FOR  
GM-2850-H-60, H-61, H-63 & H-64 A NEW  
STRING ANALYSIS/MOD ASSESSMENT RUN IS  
PERFORMED ON APRIL 22, 96.

CONSERVATIVELY ONLY SUPPORT H-61 (JOINT#350) &  
SUPPORT H-64 (JOINT#420) HORIZONTAL SPRING  
VALUES (X-DIRECTION) REVISED FROM 7308 #-# TO  
1,000,000 #-# TO ACCOUNT FOR THE ADDED  
KICKERS.

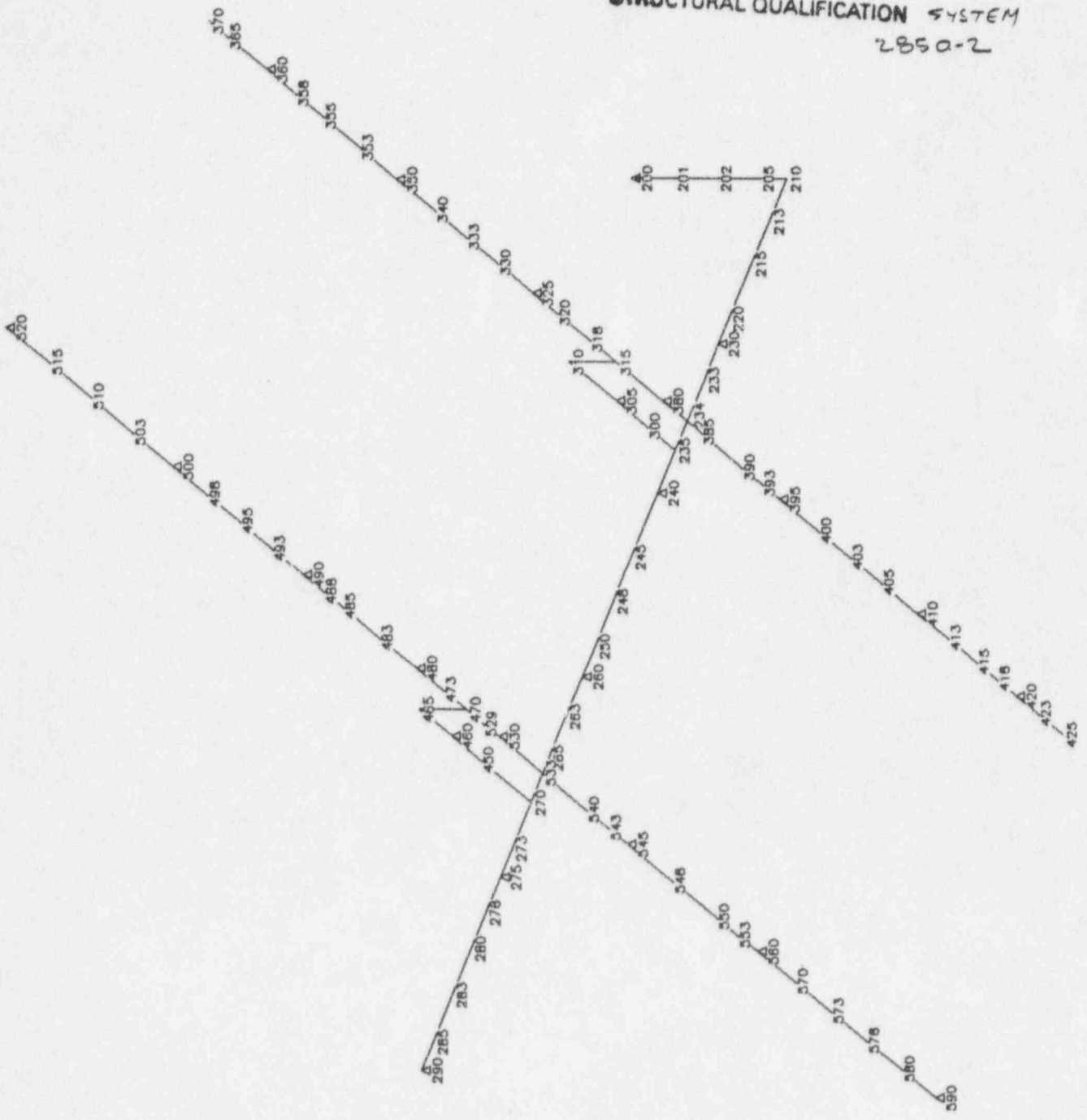
TH1 : --	EBASCO SERVICES, INC.	PREP. BY: R. Cant	CHCK. BY: S. A. Ky 6/1/96
TH2 : 45.	STRING 2850-2 DC-5766	J.O. 76230.502	Fri May 24 15:37:00 1996

P-DELTA/PDQUEUE

SHEET 43 OF 246



DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION SYSTEM  
 2850-2



0 1 2 3 4 5 6 7 8 9

TH1 : 77.

TH2 : 45.

EBASCO SERVICES, INC.

STRING 2850-2 DC-5766

PREP. BY:

J.O. 76230 Soz

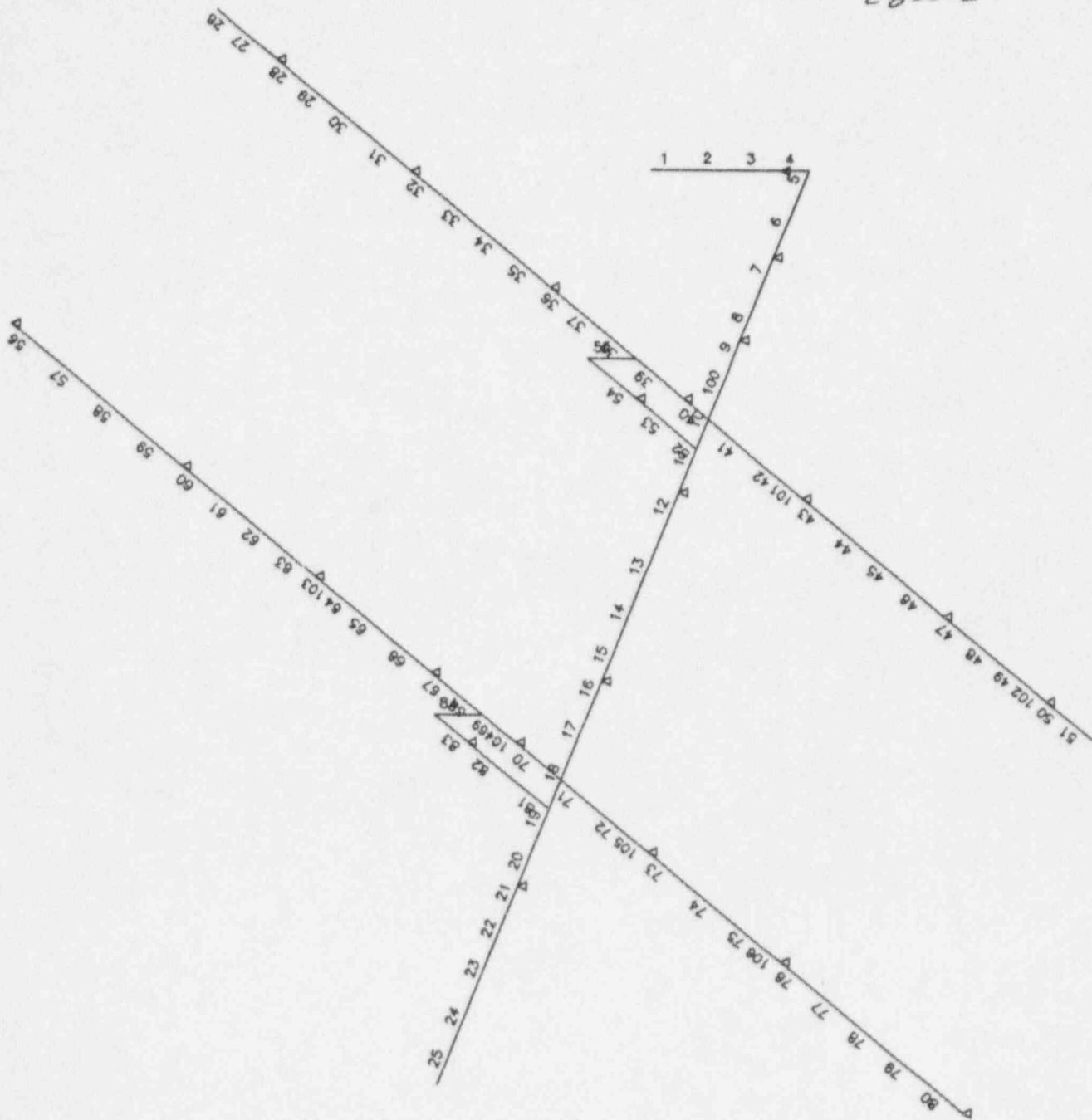
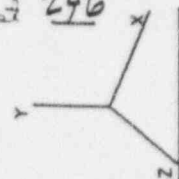
CHK. BY: E JAV

Sat Mar 23 10:09:19 1996

P-DE.1A/PDQUEUE

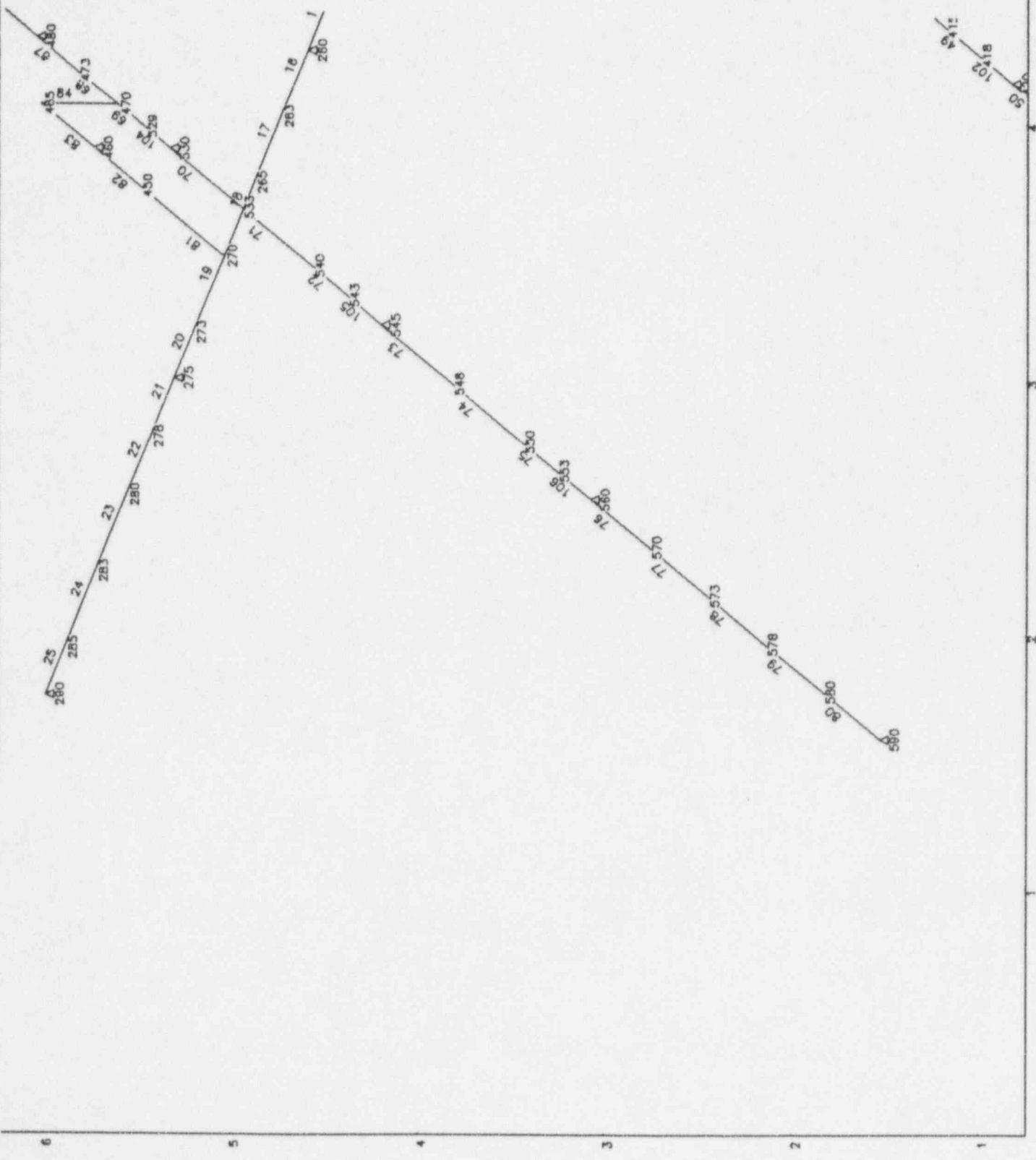
SHEET 44 sheet 246

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-2



TH1 : ~  
 TH2 : 45.

EBASCO SERVICES, INC.	PREP. BY: P. Cera	CHCK. BY: E. ATAY
STRING 2850-2	J.O. 76230.502	1/24/96
DC-5766	Fri May 24 15:38:26 1996	6/1/96



DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION SYSTEM  
 2850-2

Sheet 45 of 246

P-DELTA/PDQUEUE

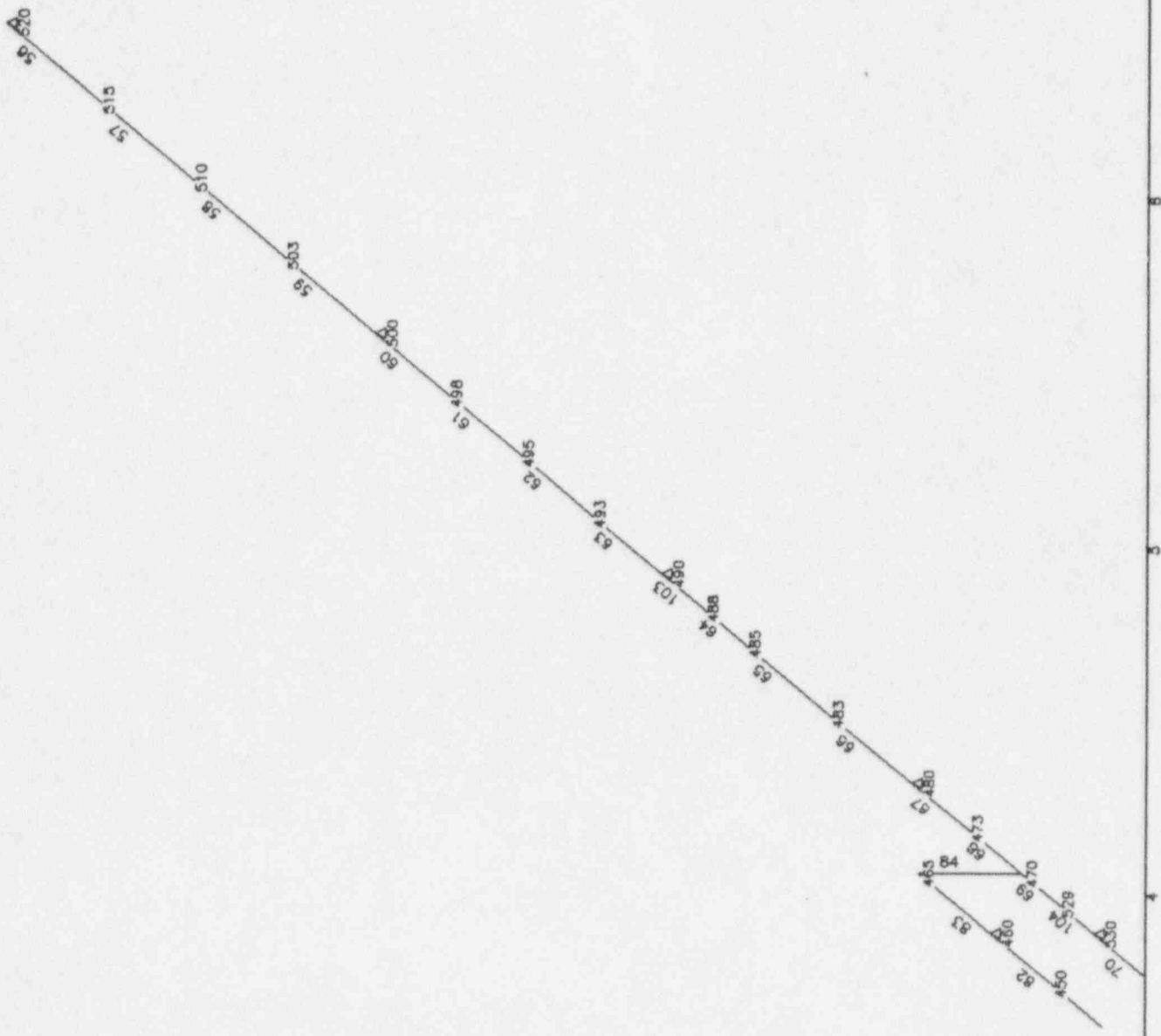
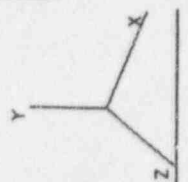
TH1 : ~  
TH2 : 45.

EBASCO SERVICES, INC.      PREP. BY: *R. Cow* 1/24/96      CHCK. BY: *EATY* 6/1/96  
STRING 2850-2    DC - 5766      T.O. 76230.50Z      Fri May 24 15:39:22 1996

P-DL TA/PDQUEE

SHEET 46 OF 246

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-2



TH1 : -1.  
 TH2 : 45.

EBASCO SERVICES, INC.

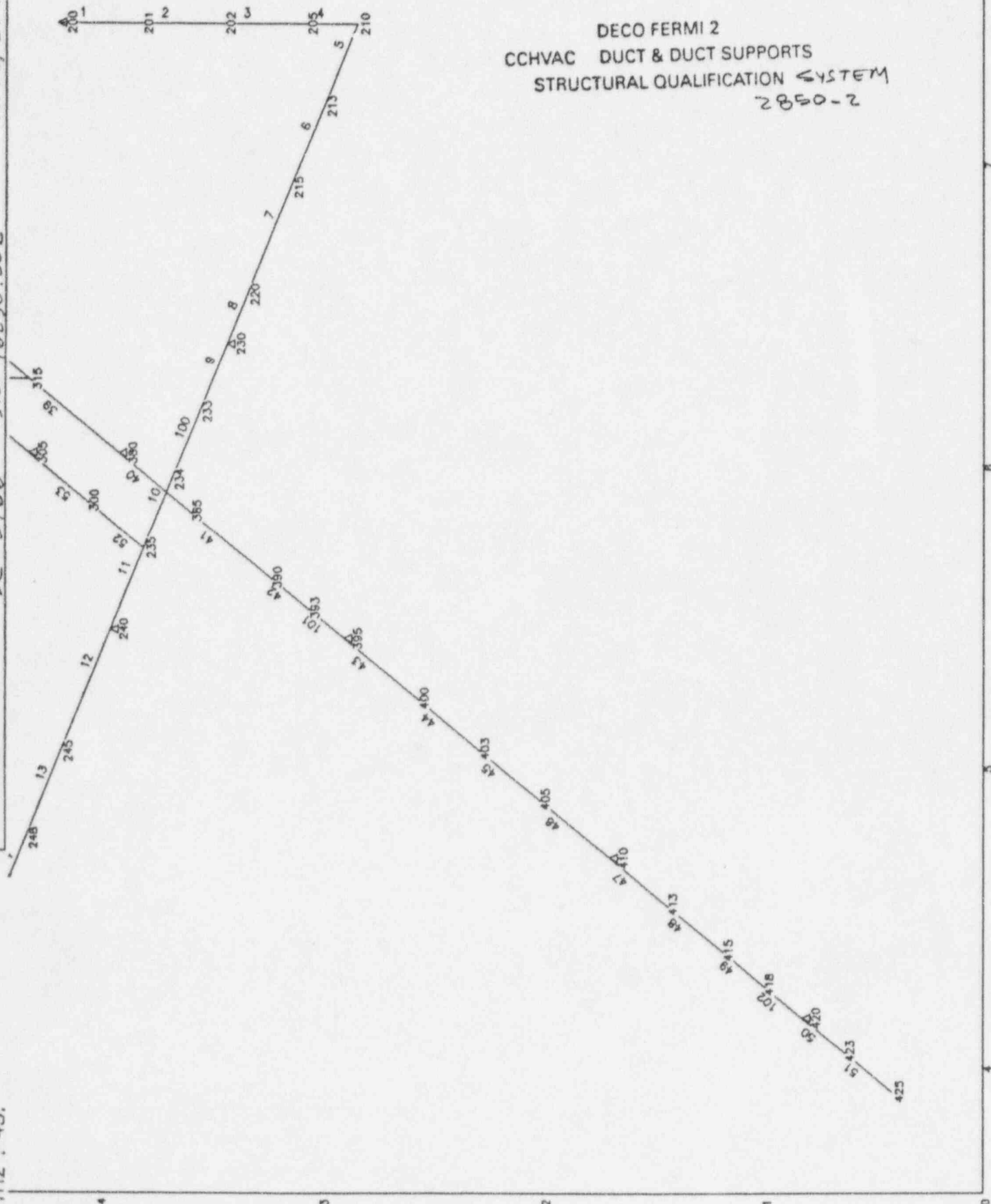
PREP. BY: *R. Cow* 1/24/96

CHCK. BY: *E. ATAY* 6/1/96

STRING 2850-2

DC - 5766 J.O. 76230.502

Fri May 24 15:40:06 1996



DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION SYSTEM  
 2850-2

SHEET 47 OF 246

P-DELTA/POUQUE



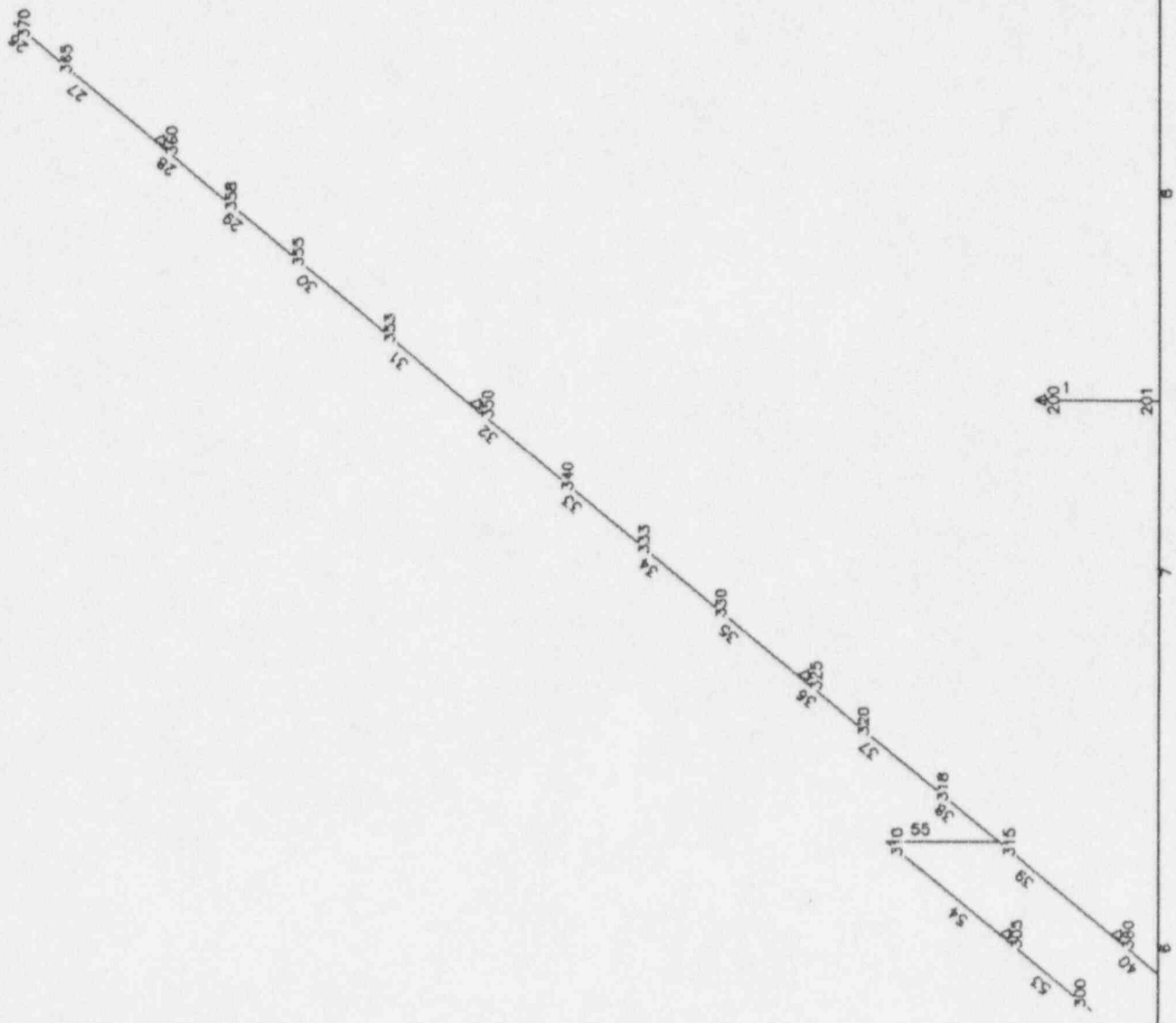
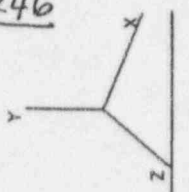
TH1 :  
TH2 : 45.

EBASCO SERVICES, INC.	PREP. BY: <i>R. Law</i> / 24/96	CHCK. BY: <i>E. ATAY</i> / 6/1/96
STRING 2850-2	DC-5766	J.O. 76230.502
		Fri May 24 15:40:43 1996

P-DLTA/PDQUE

SHEET 48 OF 246

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-2



**Raytheon**  
Engineers & Constructors

GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO. DC-5766			REV	COMP BY	CHK'D BY
PRELIM	FINAL ✓	VOID	0	JA DATE 3/12/96	<i>R. Chen</i> DATE 3/11/96
SHEET 49 OF 246				DATE	DATE
JO 76230.502					

PROJECT DECO FERM1 2  
SUBJECT CC HVAC - DUCT/SUPPORT 2B50-2 SYSTEM

DUCT SIZE	(B) <u>WT/FT</u> DUCT + INSUL.	(A) AREA FOR ANALYSIS	(B/A) RELATIVE DENSITY
1) 40x32	25.92 + 9.00 = 34.92	0.026053	1340.345
2) 40x26	23.76 + 8.25 = 32.01	0.024048	1331.088
3) 34x26	21.60 + 7.50 = 29.10	0.022063	1318.950
4) 34x20	19.44 + 6.75 = 26.19	0.020060	1305.583
5) 24x26	18.00 + 6.25 = 24.25	0.018728	1294.853
6) 10x26	12.90 + 4.50 = 17.40	0.014049	1238.522
7) 16x26	15.12 + 5.25 = 20.37	0.016057	1268.606
8) 16x14	10.80 + 3.75 = 14.55	0.012061	1206.368
9) 10x16	9.36 + 3.25 = 12.61	0.010725	1175.758
10) 14x14	10.80 + 3.75 = 14.55	0.011392	1277.212
11) 10x14	8.64 + 3.00 = 11.64	0.010058	1157.288
12) 14x8	7.92 + 2.75 = 10.67	0.009420	1132.696
13) 8x10	6.48 + 2.25 = 8.73	0.008047	1084.876
14) 6x10	5.76 + 2.00 = 7.76	0.007362	1054.061
15) 8x8	5.76 + 2.00 = 7.76	0.007401	1048.507
16) 8x14	7.92 + 2.75 = 10.67	0.009392	1136.073

**Raytheon**  
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 GENERAL  
 COMPUTATION  
 SHEET

CALCULATION SET NO

DC-5766

REV

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FINAL

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DATE

3/12/96

P. Lewis

DATE

3/15/96

PROJECT DECO FERMI 2SHEET 50 OF 246

DATE

DATE

SUBJECT CC HVAC - DUCT / SUPPORT SYSTEM  
2850-2JO 76230.501

REF

 DCN 02834  
 R/A ATT#2  
 PG 1

HANGER 6M-2850-H-49 (NODE 230)

$$L 3 \times 3 \times 1/4 : [(85.4 + 27.3) \times 2 + 24] \div 12 \times 4.9 = 102\#$$

$$L 2 1/2 \times 2 1/2 \times 1/4 : (11.58' + 5.44' + 1.81') \times 4.10 = 78\#$$

$$L 2 \times 2 \times 1/4 : [7.46 + 3.81 + 1] \times 3.19 = 40\#$$

$$\leq 220\#$$

 CONSIDER  $\frac{2}{3}$  OF  $[\frac{2}{3} \times 220] \rightarrow$ 

@ NODE 230 98#

HANGER 6M-2850-H-54 (NODE 240)

$$L 2 1/2 \times 2 1/2 \times 1/4 : (9.5 + 2 \times 9.58 + 2 \times 3.33) \times 4.1 = 145\#$$

$$@ NODE 240 \cdot 145 \times \frac{2}{3} = 97\#$$

 ATT #2  
 PG 2

HANGER 6M-2850-H-55 (NODE 260)

$$L 2 1/2 \times 2 1/2 \times 1/4 : (6.67 + 9.58 \times 2 + 3.33 \times 2) \times 4.1 = 133\#$$

$$@ NODE 260 \cdot 2 \times \frac{133}{3} = 89\#$$

 ATT #2  
 PG 4

HANGER 6M-2850-H-56 (NODE 275)

 SAME AS NODE 260  $\rightarrow$ 

$$@ NODE 275 89\#$$

 ATT #2  
 PG 5

<b>Raytheon</b> Engineers & Constructors  GENERAL COMPUTATION SHEET	CALCULATION SET NO		REV	COMP. BY	CHK'D BY
	DC-5766		0	IA	P. [Signature]
	PRELIM	FINAL ✓		DATE	DATE
	SHEET 51 OF 246			3/12/96	3/15/96
PROJECT <u>DECO FERM1 2</u>	SYSTEM		DATE	DATE	
SUBJECT <u>CC HVAC - DUCT/SUPPORT</u>	2850-2		JO	76230.502	

HANGER 6M-2850-H-58 (NODES 305 & 380)

$$L3 \times 3 \times 1/4: (11.67 + 8.83 + .58) 4.9 = 104\#$$

$$L2 \times 2 \times 1/4: (1.38 \times 3) 3.19 = \frac{14}{\leq 118\#}$$

$$@ 305 \quad \frac{118}{2} \times \frac{2}{3} = 39\#$$

$$@ 380 \quad \frac{118}{2} \times \frac{2}{3} = 39\#$$

HANGER 6M-2850-H-59 (NODE 325)

$$L2 \times 2 \times 1/4: 2 \times (11.56 + 1.25 + 1.5) \times 3.19 = 92\#$$

$$L4 \times 4 \times 1/4: 1' = 7\#$$

$$@ \text{NODE } 325 \quad 2 \times \frac{99}{3} = 66\#$$

HANGER 6M-2850-H-60 (NODE 350)

$$L2 \times 2 \times 1/4: 2 (11.56 + 1.25 + 1.08) 3.19 = 89\#$$

$$@ \text{NODE } 350: 2 \times \frac{89}{3} = 59\#$$

HANGER 6M-2850-H-61 (NODE 360)

SAME AS NODE 350

$$@ \text{NODE } 360 \quad 59\#$$

HANGER 6M-2850-H-62 (NODE 395)

$$L2 \times 2 \times 1/4: 2 (11.56 + 1.5 + 1.19) 3.19 = 91\#$$

$$@ \text{NODE } 395 \quad 91 \times \frac{2}{3} = 61\#$$

REF

DC No 2834  
 Rev A ATT 2  
 pg 7 & 6

ATT #2  
 pg 8

ATT #2  
 pg 10

ATT #2  
 pg 11

ATT #2  
 pg 12

**Raytheon**  
Engineers & Constructors

GENERAL  
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CALCULATION SET NO.

DC-5766

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3/12/96

DATE  
3/15/96

SHEET 52 OF 246

DATE

DATE

JO 76230.502

PROJECT DECO FERM 2

SUBJECT CC HVAC-DUCT / SUPPORT SYSTEM 2850-2

REF

HANGER 6M-2850-H-63 (NODE 410)

SAME AS NODE 350

@ NODE 410 59#

HANGER 6M-2850-H-64 (NODE 420)

SAME AS NODE 350

@ NODE 420 59#

HANGER 6M-2850-H-65 (NODES 460 & 530)

L 2 1/2 x 2 1/2 x 1/4 : (6.88 x 2) 4.1 = 57#

L 2 x 2 x 1/4 : (3 x 1.79) 3.19 = 17#

≤ 74#

@ NODE 460  $\frac{74}{2} \times \frac{2}{3} = 25\#$

@ NODE 530  $\frac{74}{2} \times \frac{2}{3} = 25\#$

HANGER 6M-2850-H-66 (NODE 480)

L 2 1/2 x 2 1/2 x 1/4 : (10.92 + 2.19 + 1.77) x 2 x 4.1 = 122#

@ NODE 480  $122 \times \frac{2}{3} = 81\#$

HANGER 6M-2850-H-67 (NODE 490)

L 2 1/2 x 2 1/2 x 1/4 : (10.92 + 1.96 + 1.61) x 2 x 4.1 = 119#

@ NODE 490  $119 \times \frac{2}{3} = 79\#$

HANGER 6M-2850-H-68 (NODE 500)

SAME AS NODE 490

@ NODE 500 79#

DC No 2834  
Rev A  
ATT #2  
Pg 13

ATT #2  
Pg 14, 15

ATT #2  
Pg 16, 17

ATT #2  
Pg 18, 19

ATT #2  
Pg 20, 21

ATT #2  
Pg 22, 23

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3/12/96

3/11/96

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DATE

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PROJECT DECO FERM 2SUBJECT CC HVAC - DUCT/SUPPORT SYSTEM  
2850-2

REF

HANGER 6M-2850-H-69 (NODE 520)

$$L 2 \times 2 \times 1/4 : 2 (10.92 + 1.25 + 1.03) 3.19 = 84\#$$

$$@ \text{NODE 520} : 84 \times \frac{2}{3} = 56\#$$

DC No: 2834

Rev A

ATT #2

Pg 24, 25

HANGER 6M-2850-H-70 (NODE 545)

$$L 2 \times 2 \times 1/4 : 2 (10.92 + 1.96 + 1.52) 3.19 = 92\#$$

$$@ \text{NODE 545} : 92 \times \frac{2}{3} = 61\#$$

ATT #2

Pg 26, 27

HANGER 6M-2850-H-71 (NODE 560)

$$L 2 \times 2 \times 1/4 : 2 (10.92 + 1.96 + 1.61) 3.19 = 93\#$$

$$@ \text{NODE 560} : 93 \times \frac{2}{3} = 62\#$$

ATT #2

Pg 28, 29

HANGER 6M-2850-H-72 (NODE 590)

$$L 2 \times 2 \times 1/4 : 2 (10.92 + 1.25 + 1.19) 3.19 = 86\#$$

$$@ \text{NODE 590} : \frac{86 \times 2}{3} = 57\#$$

ATT #2

Pg 30, 31

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GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO DC-5766			REV 0	COMP. BY JA	CHK'D BY P. Lewis
PRELIM	FINAL ✓	VOID	DATE 3/12/96	DATE 3/12/96	
SHEET 54 OF 246			DATE	DATE	
JO 76230.502					

PROJECT DECO FERM 2  
SUBJECT CC HVAC-DUCT/SUPPORT 2850-2 SYSTEM

WGHT	NODE	WT
	205	173
	215	77
	220	88
	235	154
	245	47
	250	59
	265	59
	270	72
	273	42
	280	53
	285	63
	300	8
	310	56
	315	39
	320	5
	330	39
	340	41
	355	36
	365	27
	385	24
	390	24
	400	36
	405	39
	415	27
	450	9
	465	33
	470	80
	485	58
	495	55
	510	39
	533	29
	540	29
	550	49
	570	22
	580	17

WGHT	NODE	WT
	230	98
	240	97
	260	89
	275	89
	305	39
	380	89
	325	66
	350	59
	360	59
	395	61
	410	59
	420	59
	460	25
	530	25
	480	81
	490	79
	500	79
	520	56
	545	61
	560	62
	590	57

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GENERAL  
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CALCULATION SET NO

DC-5766

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COMP. BY

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DATE  
3/27/96

CHK'D BY

P. Lamm

DATE  
3/11/96

PROJECT DECO FERM 2

SUBJECT CC HVAC - DUCT/SUPPORT SYSTEM  
2850-2

SHEET 55 OF 246

JO 76230.502

DATE

DATE

DUCT SIZE	A	J	I2	I3	A3/A	A2/A
40 x 32	0.026053	0.104911	0.009491	0.015572	0.815415	1.019307
40 x 26	0.024048	0.075554	0.005927	0.015552	0.717773	1.104291
34 x 26	0.022063	0.060046	0.005922	0.010844	0.782351	1.023070
34 x 20	0.020060	0.039478	0.003221	0.010821	0.661914	1.125224
24 x 26	0.018728	0.035903	0.005907	0.004917	0.921668	0.850758
10 x 26	0.014049	0.008657	0.005839	0.000600	1.228628	0.472560
16 x 26	0.016057	0.018996	0.005883	0.001891	1.074983	0.661518
16 x 14	0.012061	0.007711	0.001362	0.001877	0.770583	0.880690
10 x 16	0.010725	0.004539	0.001864	0.000596	0.990396	0.619021
14 x 14	0.011392	0.006325	0.001359	0.001359	0.815836	0.815836
10 x 14	0.010058	0.003765	0.001349	0.000594	0.924041	0.660072
14 x 8	0.009420	0.002629	0.000342	0.001340	0.563800	0.986624
8 x 10	0.008047	0.001639	0.000585	0.000339	0.825028	0.659198
6 x 10	0.007362	0.001037	0.000578	0.000168	0.901793	0.538171
8 x 8	0.007401	0.001180	0.000336	0.000336	0.717606	0.717606
8 x 14	0.009392	0.002629	0.001340	0.000342	0.989566	0.565481



**Raytheon**  
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GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO. DC-5766			REV	COMP. BY	CHK'D BY
PRELIM	FINAL ✓	VOID	0	DA DATE 3/12/96	R. Law DATE 3/12/96
SHEET 56 OF 246				DATE	DATE
JO 76230.502					

PROJECT DECO FERMIZ  
SUBJECT CC HVAC-DUCT/SUPPORT SYSTEM 2850-2

DUCT SIZE

40x32	BPROP2	1	3.333	2.667
40x26		2	3.333	2.167
34x26		3	2.833	2.167
34x20		4	2.833	1.667
24x26		5	2.000	2.167
10x26		6	0.833	2.167
16x26		7	1.333	2.167
16x14		8	1.333	1.167
10x16		9	0.833	1.333
14x14		10	1.167	1.167
10x14		11	0.833	1.167
14x8		12	1.167	0.667
8x10		13	0.667	0.833
6x10		14	0.500	0.833
8x8		15	0.667	0.667
8x14		16	0.667	1.167

**Raytheon**  
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GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO.

DC-5766

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PRELIM

FINAL

VOID

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JA  
DATE

R. Luv  
DATE

3/12/96

3/12/90

SHEET 57 OF 246

DATE

DATE

JO 76236.502

PROJECT DECO FERMI 2

SUBJECT CC HVAC - DUCT/SUPPORT SYSTEM 2850-2

SPRING RATE (lb/ft)

<u>SUPPORT</u>	<u>NODE</u>	<u>KX</u>	<u>KY</u>	<u>KZ</u>
H 49	230	—	8 266 332	45 420
H 54	240	—	7 818 648	185 916
H 55	260	—	7 565 712	227 664
H 56	275	—	7 565 712	227 664
H 58 UPPER	305	21 120	11 083 116	—
H 58 LOWER	380	16 608	8 753 352	—
H 59	325	7 200	4 780 536	—
H 60	350	7 308	4 780 536	—
H 61	360	* 7 308	4 780 536	—
H 62	395	7 068	4 780 500	—
H 63	410	7 308	4 780 536	—
H 64	420	* 7 308	4 780 536	—
H 65 UPPER	460	40 812	14 633 208	—
H 65 LOWER	530	24 672	10 275 924	—
H 66	480	18 984	6 433 596	—
H 67	490	18 108	6 433 500	—
H 68	500	16 260	6 433 320	—
H 69	520	7 536	5 078 004	—
H 70	545	9 228	5 078 124	—
H 71	560	16 260	6 433 320	—
H 72	590	7 740	5 078 028	—

\* REVISED TO 1,000,000 <sup>#-ft</sup> TO ACCOUNT FOR THE  
ADDED KICKERS (APRIL 22, 96)

TH1 :

TH2 : 45.

EBASCO SERVICES, INC.

STRING 2850-4

DC-5766

J.O. 76230.502

PREP. BY: R.A. 5/2/96

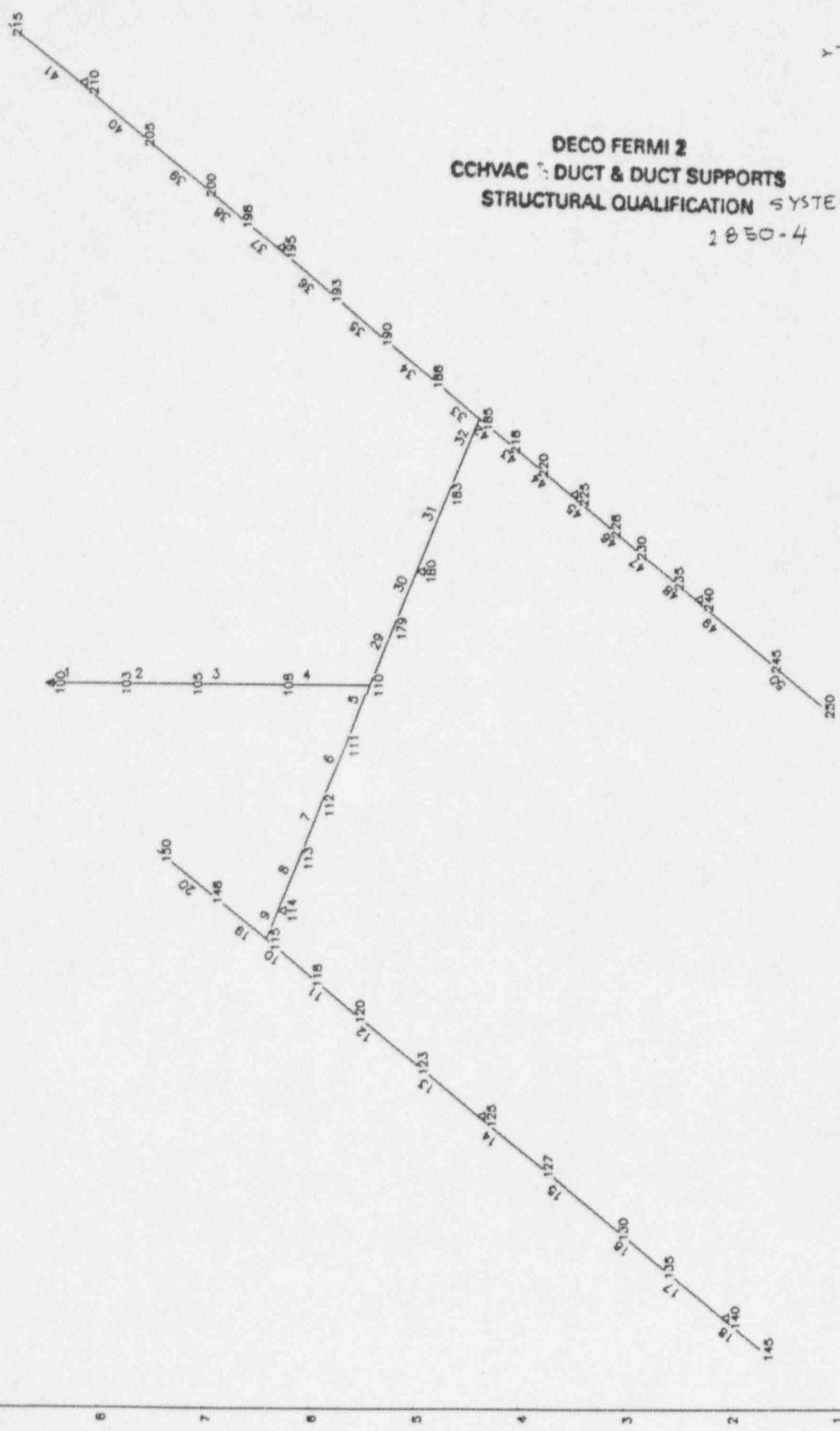
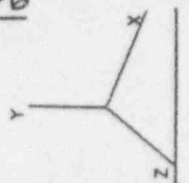
CHCK. BY: E 3/4 5/24/96

Fri May 24 15:48:14 1996

P-DELTA/PDQUEUE

SHEET 58 OF 246

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-4



9  
8  
7  
6  
5  
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0.0

**Raytheon**  
Engineers & Constructors

GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO.

DC-5766

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*R. Lee*

DATE

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2/22/96

5/24/96

SHEET 59 OF 246

DATE

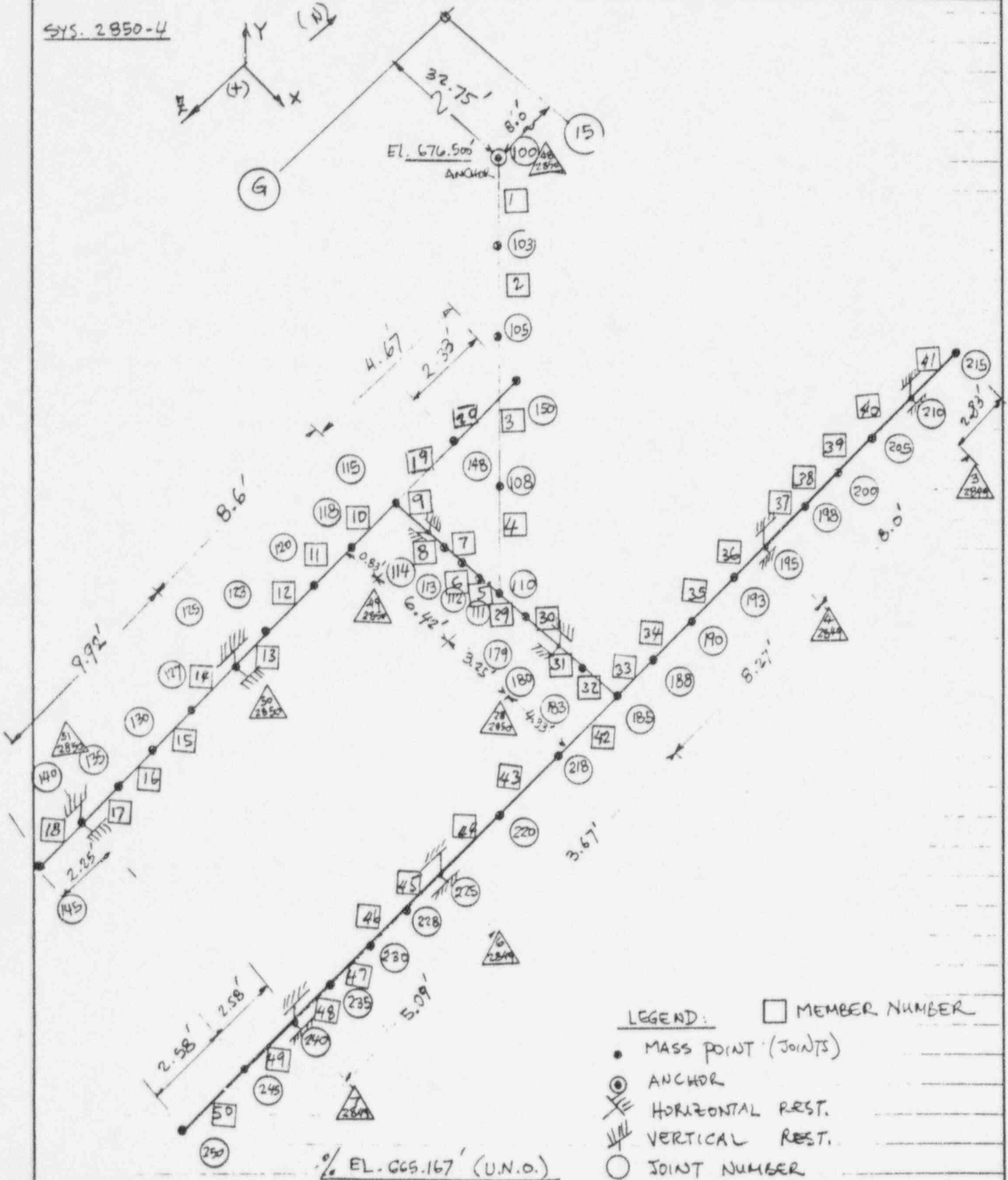
DATE

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PROJECT DECO FERMI 2

SUBJECT CC HVAC - DUCT/SUPPORT SYSTEM

SYS. 2850-4



- LEGEND:
- MEMBER NUMBER
  - MASS POINT (JOINTS)
  - ⊙ ANCHOR
  - ⊗ HORIZONTAL REST.
  - ⊥ VERTICAL REST.
  - JOINT NUMBER
  - △ SUPPORT MARK NO.

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GENERAL  
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SHEET

CALCULATION SET NO. DC-5766			REV.	COMP. BY	CHK'D BY
PRELIM	FINAL	VOID	0	E.A. DATE 2/22/96	R. Curran DATE 5/24/96
SHEET 60 OF 246				DATE	DATE
JO 76230.502					

PROJECT DECO FERMI 2  
SUBJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-4

REF: DC N:2836  
STRESS REPORT  
PAGE 9 OF 97  
DATED 7/4/84

SYSTEM 2850-4  
HANGER STIFFNESS FOR SYSTEM RUN.

<u>JOINT#.</u>	<u>HANGER #</u>	<u>X-DIRECTION OR Z-DIRECTION STIFFNESS LAT. (#/ft)</u>	<u>Y-DIRECTION STIFFNESS VER. (#/ft)</u>
114	GM-2850-H-49	89412	6.627276
125	H-50	19020	7.835964
140	H-51	19020	7.835964
180	H-28	74664	8.919612
195	H-4	14880	2.69052
210	H-3	18480	2.65488
225	H-6	47760	1.389420
240	H-7	6408	2.92524

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GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO.

DC-5766

REV

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R. Low

DATE  
3/14/96

DATE  
3/29/96

PRELIM FINAL VOID  
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SHEET 61 OF 246

JO. 76230-502

DATE

DATE

PROJECT CCMVAC DUCT & DUCT SUPPORTS

DECO FERMI 2

SUBJECT STRUCTURAL QUALIFICATION SYSTEM  
2850-4

DUCT SIZE	(IN) THICKNESS	(A) (WT/ft) DUCT + INSULATION	(B) (Ft <sup>2</sup> ) AREA FOR ANALYSIS	(A)/(B) RELATIVE DENSITY
8x8	0.0516	5.76 + 2.00 = 7.76	0.007391	1049.926
10x6		5.76 + 2.00 = 7.76	0.007412	1046.951
10x8		6.48 + 2.25 = 8.73	0.008068	1082.053
10x12		7.92 + 2.75 = 10.67	0.009416	1136.8
12x12		8.64 + 3.00 = 11.64	0.010080	1158.209
20x16		12.96 + 4.50 = 17.46	0.014070	1240.938
16x9		9.00 + 3.13 = 12.13	0.010400	1166.346
16x12		10.08 + 3.50 = 13.58	0.011400	1191.124

▲ - FOR REF SEE DC NO 2836 ATT 1 PG 15 & 16 of 18

**Raytheon**  
Engineers & Constructors

GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO.

DC - 5766

REV

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SHEET

62 OF 246

DATE

DATE

JO 76230.502

PROJECT

DECO FERMI 2

SUBJECT

CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION SYSTEM  
2850-4

24.0

DUCT SIZE	A	J	I2	I3	A3/A	A2/A
8x8	0.007391	0.001180	0.000336	0.000336	0.718577	0.718577
10x6	0.007412	0.001037	0.000168	0.000578	0.53737	0.895710
10x8	0.008068	0.001639	0.000339	0.000585	0.658280	0.822881
10x12	0.009416	0.003018	0.000925	0.000592	0.846113	0.705077
12x12	0.010080	0.003983	0.000929	0.000929	0.790377	0.790377
20x16	0.014110	0.013110	0.001886	0.003198	0.7526577	0.9411765
16x9	0.010400	0.003824	0.000459	0.001859	0.574519	1.021154
16x12	0.011400	0.006070	0.000934	0.001871	0.698860	0.931579

**Raytheon**  
Engineers & Constructors

GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO. DC - 5766			REV	COMP BY	CHK'D BY
PRELIM	FINAL	VOID	0	<del>EA</del>	<del>CR</del>
	✓			DATE 2/22/96	DATE 1/24/96
SHEET 63 OF 246				DATE	DATE
JO. 76230.502					

PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION SYSTEM  
2850-4

EXTERNAL CONCENTRATED WEIGHT ON PIPING SYSTEM  
REF. ATT#1 PAGE 14 OF 18  
2850-4 DC NO: 3836 REV A

<u>POINT</u>	<u>WEIGHT (LBS)</u>
105	64.0
110	77.0
112	5.0
115	50.0
120	28.0
130	28.0
150	25.0
185	85.0
190	36.0
200	17.0
230	31.0
245	14.0

HANGER 1/2 WT IN X, Y, Z DIRECTIONS LUMP @ POINTS (LINE)  
SEE FOLLOWING SH'S.



**Raytheon**  
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GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO

DC-5766

REV

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1/24/96

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PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION SYSTEM  
2850-4

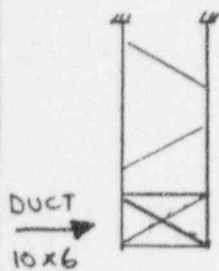
SHEET 64 OF 246

JO. 76230.502

REF

DC No: 2836  
pg 24 of 97  
AISC 7<sup>th</sup> ed.  
pg 1-57

HANGER 6M-2850-H-50 & 51



$L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}$

$[(105 \times 2) + (11.5 \times 2)] \div 12 \times 4.1 \frac{\#}{ft} = 81 \#$

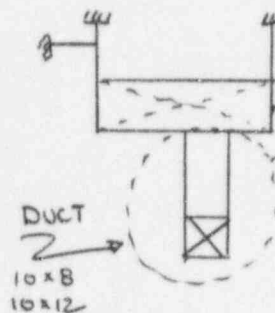
$L 2 \times 2 \times \frac{1}{4}$

$(\sqrt{11.5^2 + 11.58^2} \times 2) \div 12 \times 3.19 \frac{\#}{ft} = 9 \#$

$\leq 90 \#$

USING  $\frac{2}{3}$  OF HGR WT  $\frac{90}{3} \times 2 = \underline{\underline{60 \#}}$  @ JTS  
125 & 140

HANGER 6M-2849-H-3 & 4



$L 2 \times 2 \times \frac{1}{4}$

$[(44.408 + 9.184) \times 2 + 10.184 \times 2] \div 12 \times 3.19 = 34 \#$

USING  $\frac{2}{3}$  OF WT  $34 \times \frac{2}{3} = \underline{\underline{23 \#}}$

@ JTS  
195 & 210

DC No 2836  
pg 31 & 33  
of 97  
AISC 7<sup>th</sup> ed  
pg 1-57

GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO.

DC-5766

REV

COMP. BY

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SHEET 65 OF 246

DATE

DATE

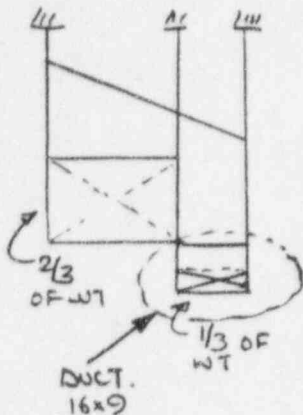
JO. 76230-502

PROJECT DECO FERMI 2  
SUBJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-4

REF

DC No 2836  
pg 44 of 97

HANGER 6M-2850-H-49



L 3x3 x 1/4

$(113.125 + 93.875 + 43.625) \div 12 \times 4.9 = 103\#$

L 2x2 x 1/4

$(\sqrt{62.25^2 + 82.7^2} + 43) \div 12 \times 3.19 = 39\#$

L 2 1/2 x 2 1/2 x 1/4

$(62.25 + 93.875 + 43.625 + 19.25) \div 12 \times 4.1 = 75\#$

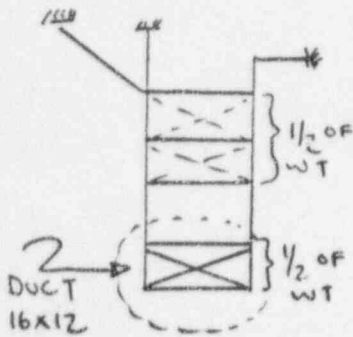
$\leq 217\#$

USE  $\frac{2}{3} \times \frac{1}{3}$  OF HGR WT  $217 \times \frac{2}{9} = 49\#$

@ JT 114

HANGER 6M-2850-H-28

DC No: 2836  
pg 54 of 97



C 4x7.25

$(85 + 12 + 10 + 17 + 13 + \sqrt{85^2 \times 2} + 19) \div 12 \times 7.25 = 167\#$

L 2 1/2 x 2 1/2 x 1/4

$(19 + 43 + 12 + 10 + 17 + 13 + 19) \div 12 \times 4.1 = 46\#$

L 2x2 x 1/4

$(19) \div 12 \times 3.19 = 5\#$

TS 2x2x.25 (SA 411') = 5.4#

$\leq 224\#$

USING  $\frac{1}{2} \times \frac{2}{3}$  OF HGR WT  $\frac{224}{3} = 75\#$

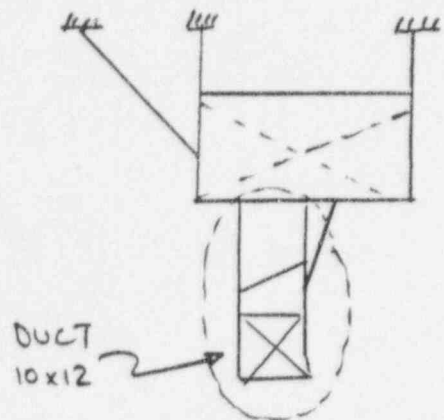
@ JT 180

GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO. DC-5766			REV	COMP. BY	CHK'D BY
PRELIM	FINAL	VOID	0	JA	R Rev
	✓			DATE	DATE
SHEET 66 OF 246					
JO 76230.502					
				DATE	DATE

PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
 SUBJECT STRUCTURAL QUALIFICATION SYSTEM  
2850.4

HANGER 6M-2849-H-6



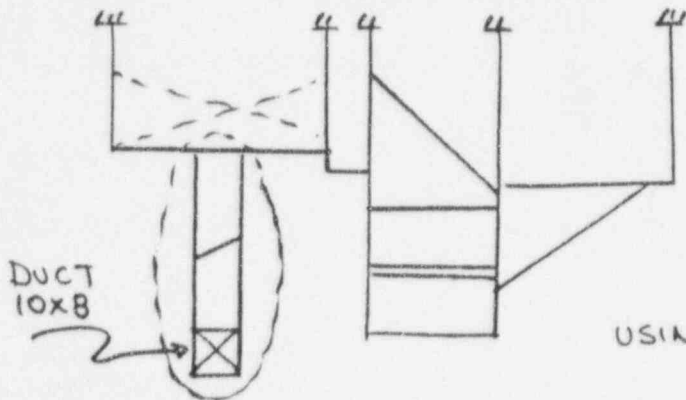
L 3x3x1/4  
 $(98.5 \times 2 + 12.25 \times 2 + \sqrt{12.25^2 \times 2} + \sqrt{16.21^2 + 60.5^2}) \times 4.9 \div 12 = 123\#$

USING  $\frac{2}{3}$  OF WT 123 = 82#  
 @ JT  
 225

REF

DC No: 2836  
 pg 63 of 97

HANGER 6M-2849-H-7



L 2x2x1/4  
 $[(38.5 + 11.5 + 39.5 + 9.5) \times 2 + 11.5 \times 2 + \sqrt{11.5^2 \times 2}] \div 12 \times 3.19 = 63\#$

USING  $\frac{2}{3}$  OF WT 63 = 43#  
 @ JT  
 240

DC No 2836  
 pg 76 of 97

**Raytheon**  
Engineers & Constructors

GENERAL  
COMPUTATION  
SHEET

CALCULATION SET NO			REV	COMP. BY	CHK'D BY
DC-5766			0	EA	RS
PRELIM	FINAL	VOID		DATE	DATE
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SHEET 67 OF 246				DATE	DATE
JO 76230.502					

PROJECT DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 SUBJECT STRUCTURAL QUALIFICATION

B. SUPPORT EVALUATION

**Raytheon**  
 Engineers & Constructors

 GENERAL  
 COMPUTATION  
 SHEET

DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

REV

COMP BY

CHK'D BY

PRELIM

FINAL

VOID

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N. WINTER

K T W/6

DATE  
4/12/96DATE  
4/12/96

SHEET 68 OF 246

DATE

DATE

JO 76 280502

REF.

SUPPORT LOADS FOR SYSTEM 2850-1

SUPPORT MARK NO. NODE POINT	DIRECTION	DEAD LOAD	OBE	SSE	SHARED WITH SYSTEM
		[LBS]	[LBS]	[LBS]	
6H-2849-H-57	VERT.	168	67	71	2849-3 (F)
130	LATERAL	0	212(F <sub>1</sub> )	272(F <sub>1</sub> )	
-H-58	VERT.	260	161	185	2849-3 (F)
120	LATERAL	0	92(F <sub>1</sub> )	109(F <sub>1</sub> )	
-H-61	VERT.	329	192	219	2849-3 (F)
170	LATERAL	0	81(F <sub>3</sub> )	74(F <sub>3</sub> )	
-H-62	VERT.	273	69	60	2849-3 (F)
180	LATERAL	0	216(F <sub>3</sub> )	192(F <sub>3</sub> )	
6H-2850-H-84	VERT.	406	137	137	
138	LATERAL	~0	93(F <sub>1</sub> )	119(F <sub>1</sub> )	
H-73	VERT.	596	241	246	
155	LATERAL	12(F <sub>1</sub> )	431(F <sub>1</sub> )	517(F <sub>1</sub> )	
-H-74	VERT.	353	87	73	
190	LATERAL	0	1367(F <sub>3</sub> )	1162(F <sub>3</sub> )	
-H-75	VERT.	242	60	51	
200	LATERAL	6	567(F <sub>1</sub> )	502(F <sub>1</sub> )	
-H-76	VERT.	237	57	47	
210	LATERAL	0	204(F <sub>1</sub> )	172(F <sub>1</sub> )	
-H-77	VERT.	540	274	314	
270	LATERAL	~0	749(F <sub>1</sub> )	823(F <sub>1</sub> )	
-H-78	VERT.	331	118	107	
370	LATERAL	13(F <sub>3</sub> )	245(F <sub>3</sub> )	298(F <sub>3</sub> )	
-H-79	VERT.	498	162	161	
285	LATERAL	~0	516(F <sub>3</sub> )	457(F <sub>3</sub> )	
-H-80	VERT.	191	64	52	
295	LATERAL	0	170(F <sub>3</sub> )	150(F <sub>3</sub> )	
-H-81	VERT.	359	108	83	
305	LATERAL	0	1476(F <sub>3</sub> )	1303(F <sub>3</sub> )	
-H-82	VERT.	160	153	93	
320	LATERAL	6(F <sub>1</sub> )	320(F <sub>1</sub> )	366(F <sub>1</sub> )	
-H-83	VERT.	377	244	152	
330	LATERAL	0	31(F <sub>1</sub> )	36(F <sub>1</sub> )	

(F<sub>1</sub>) AND (F<sub>3</sub>): DIRECTION OF LOADS BASED ON SPRING DIRECTIONF<sub>1</sub> ≡ E-W (X) F<sub>3</sub> ≡ N-S (Z)
 OUTPUT FOR  
 DYNAMIC  
 ANALYSIS FOR  
 SYSTEM 2850-1  
 4/12/96

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COMPUTATION  
SHEET

DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION SYSTEM 2850-1

CALCULATION SET NO. DC-5766			REV	COMP BY	CHK'D BY
PRELIM	FINAL	VOID	0	NYURTER DATE 4/13/96	KTW DATE 4/13/96
SHEET 69 OF 246				DATE	DATE
J.O. 76 230 502					

SUPPORT LOADS FOR SYSTEM 2850-1 (cont'd)

SUPPORT MARK No. Node Point	DIRECTION	DEAD LOAD [LBS]	OBE [LBS]	SSE [LBS]	SHARED WITH SYSTEM
6M-2850-1-H-57 100	VERT.	371	222	252	
	LATERAL	18 (F3)	435 (F3)	618 (F3)	
	LATERAL	7 (F1)	270 (F1)	323 (F1)	
-H-85 298	VERT.	372	268	323	
	LATERAL	~ 0	100 (F1)	109 (F1)	
-H-86 407	VERT.	64	61	63	
	LATERAL	32 (F1)	126 (F1)	140 (F1)	
	LATERAL	46 (F3)	167 (F3)	193 (F3)	
-H-87 380	VERT.	29	43	95	
	LATERAL	18 (F1)	345 (F1)	358 (F1)	
	LATERAL	4 (F3)	73 (F3)	90 (F3)	
-H-88 395	VERT.	178	119	130	
	LATERAL	9 (F1)	350 (F1)	377 (F1)	
	LATERAL	49 (F3)	339 (F3)	373 (F3)	
6M-2850-1-H-3 230	VERT.	154	37	31	2850-3
	LATERAL	0	397 (F1)	336 (F1)	
-1-H-11 390	VERT.	64	86	51	
	LATERAL	0	161 (F1)	157 (F1)	

REF.

SUPPORTS 6M-2849-H-57, -H-58, -H-61, -H-62 AND 6M-2850-1-H-3 WILL BE ADDRESSED LATER BECAUSE OF OTHER SYSTEMS.

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CCHVAC: DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION  
SUBJECT SUPPORT EVALUATION FOR SYSTEM 2850-1

CALCULATION SET NO DC-5766			REV 0	COMP. BY N. YURTER	CHK'D BY ETW/L
PRELIM	FINAL ✓	VOID		DATE 9/12/96	DATE 5/23/96
SHEET 70 OF 246				DATE	DATE
JO 76230502					

SUPPORT MARK NO.	CONFIGURATION	VERT. MEMBER SIZE	HORIZONTAL MEMBER SIZE	REMARKS
GM-2849-4-57-A		L2 1/2 x 2 1/2 x 1/4	L2 1/2 x 2 1/2 x 1/4	SHARED WITH SYSTEM 2849-3 (F)  SEE DWN No. 10881A
GM-2849-4-58		L3 x 3 x 1/4 L2 1/2 x 2 1/2 x 1/4	L3 x 3 x 1/4 L2 1/2 x 2 1/2 x 1/4	SHARED WITH SYSTEM 2849-3 (F)
GM-2849-4-61-A		L2 1/2 x 2 1/2 x 1/4	L2 1/2 x 2 1/2 x 1/4	SHARED WITH SYSTEM 2849-3 (F)

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PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

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4/12/96

KTWL  
DATE  
5/23/96

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JO 76 230 502

DATE

DATE

SUPPORT MARK No.	CONFIGURATION	VERT. MEMBER SIZE	HORIZONTAL MEMBER SIZE	REMARKS
64-2849-H-62-A		L2 1/2 x 2 1/2 x 1/4	L2 1/2 x 2 1/2 x 1/4	SHARED WITH SYSTEM 2849-3 (F)
64-2850-H-57-B		L3 x 3 x 1/4	L3 x 3 x 1/4	
64-2850-H-73-A		L4 x 4 x 1/4	L4 x 4 x 1/4	



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PROJECT CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT SYSTEM 2850-1 STRUCTURAL QUALIFICATION

CALCULATION SET NO.

DC-5766

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0 N.Y. WALTER  
DATE 4/12/96  
DATE 5/23/96

PRELIM FINAL VOID

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DATE DATE

SUPPORT MARK No.	CONFIGURATION	VERT. MEMBER SIZE	HORIZONTAL MEMBER SIZE	REMARKS
6M-2850-H-74A		L4x4x 1/4	L4x4x 1/4	SEE DCN NO. 10938
6M-2850-H-75		L4x4x 1/4	L4x4x 1/4	
6M-2850-H-76		L2x2x 1/4	L2x2x 1/4	REPRESENTED BY SUCT. 6M-2850-H-63. (SYS. 2850-2) MEMBER FAILS.

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PROJECT CCHVAC DUCT & DUCT SUPPORTS

SUBJECT STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DL-576C

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4/12/96

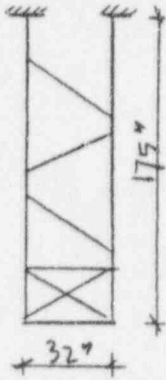
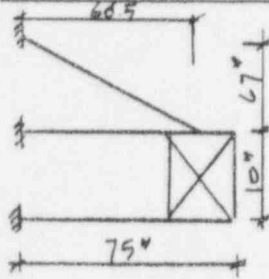
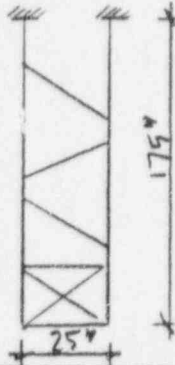
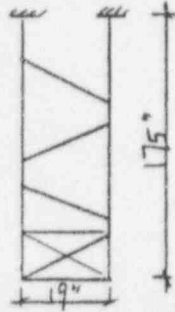
5/23/96

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DATE

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SUPPORT MARK No.	CONFIGURATION	VERT. MEMBER SIZE	HORIZONTAL MEMBER SIZE	REMARKS
6M-2850-H-77-B		L4x4x1/4	L4x4x1/4	
6M-2850-H-78		L2 1/2 x 2 1/2 x 1/4	L2 1/2 x 2 1/2 x 1/4	
6M-2850-H-79-B		L4x4x1/4	L4x4x1/4	
6M-2850-H-80		L2x2x1/4	L2x2x1/4	REPRESENTED BY SUPT. 6M-2850-H-63 (SYS. 2850-2) MEMBER FAILS.

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STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

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DATE

5/23/96

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DATE

DATE

SUPPORT MARK No.	CONFIGURATION	VERT. MEMBER SIZE	HORIZONTAL MEMBER SIZE	REMARKS
64-2850-H-81-A		$L3 \times 3 \times 1/4$	$L3 \times 3 \times 1/4$	
64-2850-H-82		$L4 \times 4 \times 1/4$	$L4 \times 4 \times 1/4$	
64-2850-H-83		$L2 \times 2 \times 1/4$	$L2 \times 2 \times 1/4$	
64-2850-H-84		$L2 \frac{1}{2} \times 2 \frac{1}{2} \times 1/4$	$L2 \frac{1}{2} \times 2 \frac{1}{2} \times 1/4$	

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9/13/96

5/23/96

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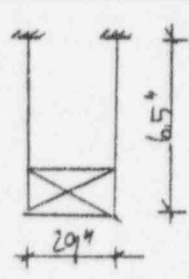
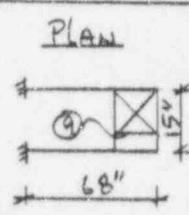
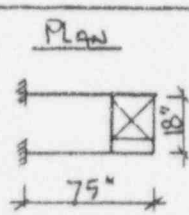
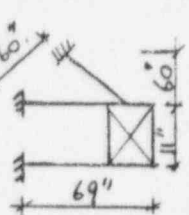

JO 76230502

DATE

DATE

DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION SYSTEM 2850-1

Support MARK No.	CONFIGURATION	VERT. MEMBER SIZE	HORIZONTAL MEMBER SIZE	REMARKS
GM-2850-H-85		L2 1/2 x 2 1/2 x 1/4	L2 1/2 x 2 1/2 x 1/4	
GM-2850-H-86B	<p>PLAN</p> 	L4 x 4 x 1/4	L4 x 4 x 1/4 EXCEPT MEM ⑨ L3 x 3 x 1/4	
GM-2850-H-87-B	<p>PLAN</p> 	L4 x 4 x 1/4	L4 x 4 x 1/4	SEE DCN No. 10939 A
GM-2850-H-88		L4 x 4 x 1/4	L4 x 4 x 1/4	
GM-2850-1-H-3		L2 1/2 x 2 1/2 x 1/4	L2 1/2 x 2 1/2 x 1/4	SHARED WITH SYSTEM 2850-3 (R)

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PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

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9/13/96

K. T. WU  
DATE  
5/25/96

SHEET 76 OF 246

DATE

DATE

JO 76 230502

SUPPORT MARK No.	CONFIGURATION	VERT. MEMBER SIZE	HORIZONTAL MEMBER SIZE	REMARKS
6M-2850-1-H-11-A		L2 1/2 x 2 1/2 x 1/4	L2 1/2 x 2 1/2 x 1/4	

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PROJECT CCHVAC DUCT & DUCT SUPPORTS

SUBJECT STRUCTURAL QUALIFICATION

SYSTEM 2850-1

CALCULATION SET NO.

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DATE  
5/23/96

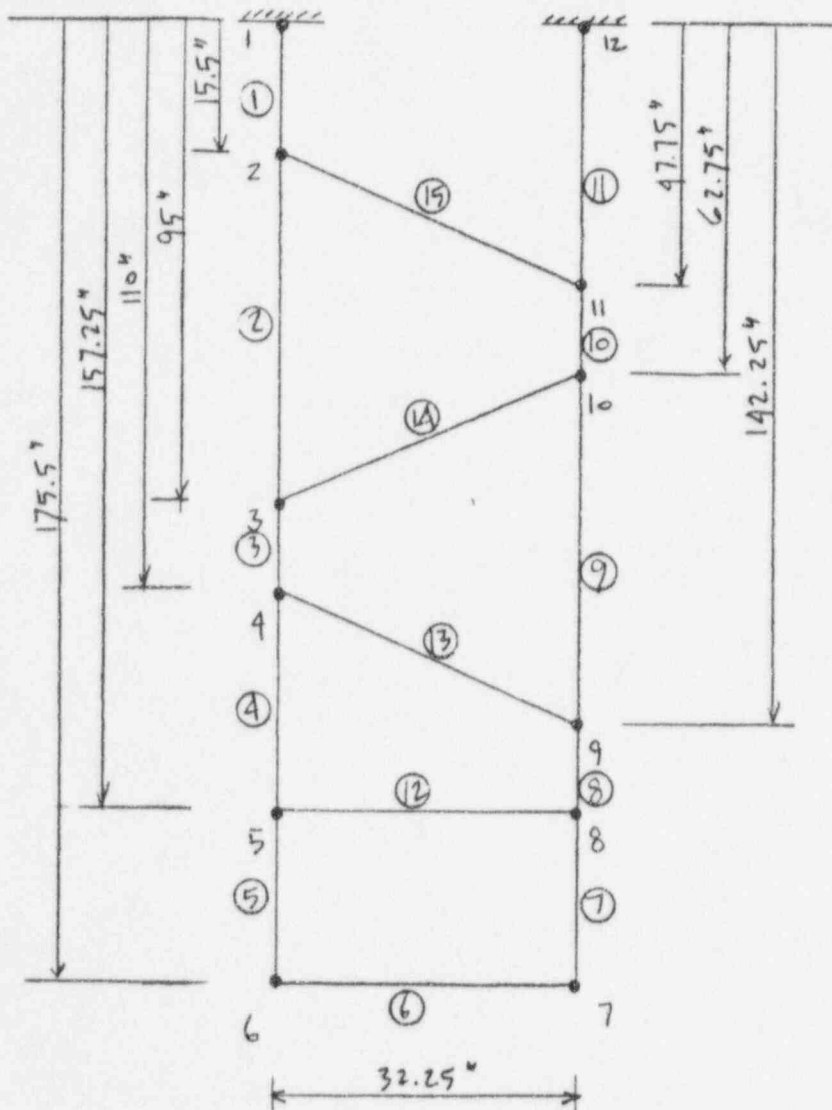
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DATE DATE

REF.

COMPUTER MODEL FOR SUPPORT 6M-2850-H-77



LAST JOINT NO. 12  
LAST MEMBER NO. 15

ALL MEMBERS ARE L4x4x 1/4

DWG.  
10-9221-000-  
6M-2850-77-B

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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION SYSTEM 2850-1

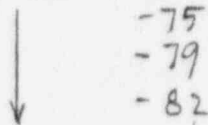
CALCULATION SET NO DC-5766			REV	COMP BY	CHK'D BY
PRELIM	FINAL	VOID	0	N. YURTER DATE 4/15/96	KTW DATE 5/23/96
SHEET 78 OF 246				DATE	DATE
JO 76230502					

RET.

CHECK MEMBER AND WELD STRESSES FOR SUPPORT  
6M-2850-H-77

THIS SUPPORT IS REPRESENTATIVE CASE FOR FOLLOWING SUPPORTS BASED ON GREATER HORIZONTAL SEISMIC LOADS :

6M-2850-H-73



SEE SUPPORT  
LOAD TABLE

FOR STATIC RUN OF SUPPORT 6M-2850-H-77, TWO OUTPUT EXIST

- 1) DEAD LOAD = 540 # (ACTUAL LOAD)
- 2) DEAD LOAD = 160 # (HYPOTHETICAL LOAD FROM SUPPORT 6M-2850-H-82 IS USED TO CREATE MAX COMPRESSION FOR VERTICAL POST AND TO ENVELOPE REPRESENTED SUPPORTS)

STARDYN  
OUTPUT  
4/22/96

FROM OUTPUT WITH DL = 160 #, MAX. COMPRESSION LOAD FOR VERT. POST = 3829 # (LOAD COMBINATION - 6 WITH BENDING MOMENT = 8810 #-ft cJT. 1)

SECTION PROPS FOR L<sub>4x4</sub> = 1/4 :

A = 1.94 in<sup>2</sup>  
S = 1.05 in<sup>3</sup>  
r = 1.25 in (GEOMETRIC AXIS)

REV. 7/4/ED  
Pg. 1-56

GOVERNING SSE-LOADS FOR VERTICAL POST,  
LOAD COMBINATION - 6 :

AXIAL COMPRESSION LOAD = 3829 # cJT.1  
BENDING MOMENT = 8810 #-ft cJT.1

GOVERNING OBE-LOADS FOR VERTICAL POST

LOAD COMBINATION - 2 :  
AXIAL LOAD (COMPRESSION) = 3453 # cJT.1  
BENDING MOMENT = 8019 # cJT.1

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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO

DC-5766

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DATE

4/15/96

5/23/96

SHEET 79 OF 246

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DATE

JO 76230502

REF.

SUPT. 6M-2850-H-77 (CONT'D)

FROM SUPPORT LOAD TABLE FOR SYSTEM 2850-1:

$$\frac{SSE - LATERAL}{OBE - LATERAL} = \frac{823}{749} = 1.10 < 1.45$$

$$\frac{SSE - VERT.}{OBE - VERT.} = \frac{314}{274} = 1.15 < 1.45$$

∴ OBE GOVERNS

$$1.45 = \frac{0.58 F_y (SSE)}{0.40 F_y (OBE)}$$

 DESIGN CRITERIA  
 NO. FERMI-DC-  
 76230-1  
 REV. 0, PG. 19

$$\frac{KL}{r} = \frac{10 \times 175.25}{1.25} = 141 \rightarrow F_A = 7510 \text{ psi}$$

$$f_a = \frac{3453}{1.94} = 1780 \text{ psi}$$

$$\frac{f_a}{F_A} = \frac{1780}{7510} = 0.23 > 0.15$$

$$C_m = 0.85$$

$$F_e' = \frac{12 \times \pi^2 \cdot E}{23 \left( \frac{KL}{r} \right)^2}$$

 HOWEVER, DIVIDE RADIUS OF GYRATION  
 BY FACTOR 1.35 TO CHECK BENDING  
 STRESS ABOUT GEOMETRIC AXES.  
 MAX. BUCKLING LENGTH IN BENDING  
 PLANE  $L = 95 - 15.5 = 79.5'$   
 (SEE COMPUTER MODEL)

 MISC. Q'D ED  
 PG. 5-314

$$F_e' = \frac{12 \times \pi^2 \times 29 \times 10^6}{23 \left( \frac{10 \times 79.5 \times 1.35}{1.25} \right)^2} = 20236 \text{ psi}$$

 $F_b = 19660 \text{ psi}$  ALLOW. BENDING STRESS EVALUATED  
 BASED ON LOCAL BUCKLING (GOVERNS)  
 SEE EVALUATION OF HANGER  
 6M-2850-H-59.

$$f_b = \frac{M}{S} = \frac{8019}{1.05} = 7637 \text{ psi}$$



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STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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NO 76230502

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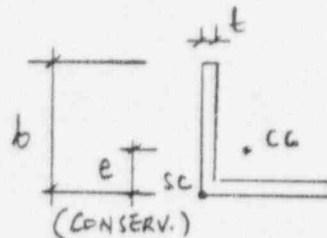
SUPT. 6M-2850-H-77 (CONT'D)

$$\frac{f_a}{F_A} + \frac{C_m f_b}{\left(1 - \frac{f_a}{F_e'}\right) F_b} = 0.23 + \frac{0.85 \times 2637}{\left(1 - \frac{1780}{20236}\right) 19660} = 0.60 < 1.0$$

∴ MEMBERS OK.

OTHER MEMBERS ARE OK BY COMPARISON OF MEMBER LOADS AND BUCKLING LENGTH.

CHECK SHEAR FOR L4x4x1/4



$$e = 1.09''$$

$$\begin{aligned} \text{MAX. TORSIONAL MOMENT} &= \frac{\text{MAX. LOAD (MEMBER), JT. 1}}{\text{HORIZONTAL LOAD} \times e} \\ &= 857 \times 1.09 \\ &= 934 \text{ #} \end{aligned}$$

$$\text{MAX. SHEAR LOAD} = 857 \text{ # (SEE ABOVE)}$$

$$f_v = \frac{1.5 \sqrt{b}}{b t} + \frac{3 M T}{A t} = \frac{1.5 \times 857}{4 \times 0.25} + \frac{3 \times 934}{1.94 \times 0.25}$$

$$f_v = 7063 \text{ psi} < 0.4 \times F_y = 0.4 \times 36000 = 14400 \text{ psi} \quad \therefore \text{OK}$$

WARPING STRESSES ARE NEGLIGIBLE.

REF.

 AISC, 7<sup>th</sup> ED  
 Pg. 1-56

 STADYNE  
 OUTPUT FOR  
 H77.0UT  
 4/22/96  
 (ATT. B2)

 AISC, 9<sup>th</sup> ED  
 Pg. 5-315 & 5-316

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PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

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DATE

5/23/96

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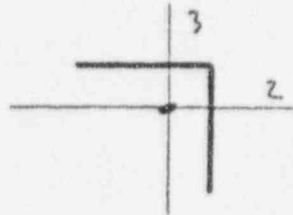
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REF.

SUPT. 64-2850-H-77 (CONT'D)

CHECK WELD AT SUPPORT POINT (JT. 1 OR JT. 12)



$A = 2 \times 4 = 8 \text{ IN}$

$S_3 = \frac{4^2(4 \times 4 + 4)}{12} = 4 \text{ IN}^2$   
MIN  $6(2 \times 4 + 4)$

WELD LOADS :

CONSIDER CG OF WELD = CG OF  $L \times 4 \times 1/4$

$F_1 = 780 \#$   
 $F_2 = 4025 \#$   
 $M_3 = 28025 \# \cdot \text{ft}$  } ENVELOPED LOADS, LOAD COMB-1 THRU 4 SINCE OBE GOVERNS (SEE PREVIOUS PAGES)

BLODGETT, 1972  
PG. 7.4-7  
(REF. 9)

STARDYNE  
OUTPUT  
4/22/96

$f_1 = \frac{4025}{8} + \frac{8025}{4} = 2509 \#/\text{in}$

$f_3 = \frac{780}{8} = 98 \#/\text{in}$

$f_R = \sqrt{2509^2 + 98^2} = 2509 \#/\text{in}$

$w = \frac{2509}{14400} = 0.17 \text{ in}$  ALL-AROUND WELD  $< \sqrt{2 \times 0.25} = 0.50 \text{ in}$

$IR = \frac{0.17}{0.50} = 0.35$

DWG.  
10-4221-000-LH  
-2850-77

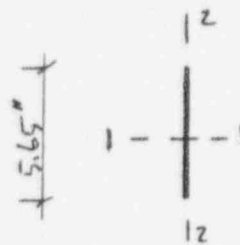
CHECK WELD FOR DIAGONAL BRACES (MEM. 13, 14 & 15)

BRACES HAVE ANGLE WITH HORIZONTAL  $45^\circ$   
CONSIDER CONSERVATIVELY SINGLE LINE WELD,

WELD LENGTH =  $\frac{4}{\cos 45^\circ} = 5.65 \text{ in}$

ENVELOPED LOADS (OBE GOVERNS)

$F_{x1} = 1352 \# (LC-4)$   
 $F_{x2} = 196 \# (LC-4)$   
 $M_{x3} = 1627 \# \cdot \text{ft} (LC-4)$  } BEAM END LOADS



STARDYNE  
OUTPUT FOR  
H77.  
4/22/96  
(ATT. B2)

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DECO FERMI 2

PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

SUBJECT STRUCTURAL QUALIFICATION

SYSTEM 2850-1

CALCULATION SET NO

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Supt. 6M-2850-H-77 (CONT'D)

WELD FORCES:

$$f_1 = \frac{1352}{5.65} + \frac{4627}{\left(\frac{2}{3} \times 5.65\right)} = 1468 \#/\text{in}$$

$$f_2 = \frac{196}{5.65} = 35 \#/\text{in}$$

$$f_R = \sqrt{1468^2 + 35^2} = 1468 \#/\text{in}$$

$$W = \frac{1468}{\text{REQ'D } 14400} = 0.10 < 0.25 \quad \therefore \text{OK}$$

$$IR = \frac{0.10}{0.25} = 0.40 < 1.0$$

By comparison all other welds are OK.

NOTE Supt. H-77 per FEM # 0289 and Supt. H-79 per FEM # 0287 have notch in vertical post. However, these notches are reinforced with L4x4x1/4  $\therefore$  no impact on member stresses.



DWG.  
10-4221-000-  
6M-2850-77-B  
AND  
-79-B

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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

REV

COMP. BY

CHK'D BY

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4/16/96

KTW  
DATE  
5/23/96

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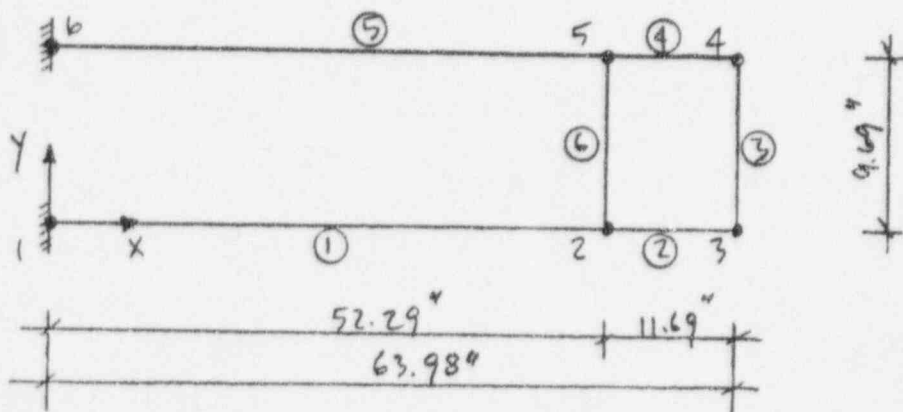
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COMPUTER MODEL FOR SUPPORT 61-2850-1-H-11



LAST JOINT No. 6

LAST MEMBER No. 6

ALL MEMBERS ARE L 2 1/2 x 2 1/2 x 1/4

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DATE

JO 76 230502

REF.

CHECK MEMBER AND WELD STRESSES FOR SUPPORT  
GM-2850-1-H-11

THIS SUPPORT IS REPRESENTATIVE HANGER FOR  
 SUPT. GM-2850-H-84 AND GM-2850-H-85 BASED ON  
 GREATER VERTICAL LOAD (DL + OBE) WHICH CREATES  
 GREATER BENDING MOMENT FOR SUPPORT BASED ON  
 MAGNITUDE OF VERTICAL LOADS ALTHOUGH MOMENT  
 ARM IS SLIGHTLY SHORTER.

ALSO, HORIZONTAL OBE-LOAD = 161 # CREATES GREATER  
 COMPRESSION IN MAIN MEMBER THAN IN THE CASE  
 OF TWO OTHER SUPPORTS

SEE LOAD  
 TABLE FOR  
 SYS. 2850-1

MAX. AXIAL LOAD = 643 # (EJT. 1, LOAD COMB-A, EQUILIBRIUM)

MAX. BENDING M=M. = 2519 #-4 (EJT. 1, LOAD COMB-A, EQUILIBRIUM)

STADYNE  
 OUTPUT FOR  
 H11.  
 (ATT. B6)

AXIAL LOAD = 643 # < 1062 # (Supt. GM-2850-H-59)

MOMENT = 2519 #-4 > 1700 # ( -11 - )

HOWEVER, SECTION MODULUS FOR L2 1/2 x 2 1/2 x 1/4 IS GREATER  
 THAN SECTION MODULUS OF L2 x 2 x 1/4. (0.394 IN<sup>3</sup> > 0.247 IN<sup>3</sup>)

RATIO OF SECTION MODULUS :

$$\frac{S_{L2 1/2 \times 2 1/2 \times 1/4}}{S_{L2 \times 2 \times 1/4}} = \frac{0.394}{0.247} = 1.59$$

RATIO OF DESIGN MOMENTS :

$$\frac{M_{H-11}}{M_{H-59}} = \frac{2519}{1700} = 1.48 < 1.59 \quad \therefore \text{BENDING STRESS OK}$$

THEREFORE ALL MEMBERS FOR SUPT. H-11 ARE OK  
 BY COMPARISON.

BY COMPARISON OF WELD LOADS AND WELD SECTION  
 PROP'S OF SUPT. H-59 V.S. H-11, ALL WELDS FOR  
 SUPT. H-11 ARE OK.

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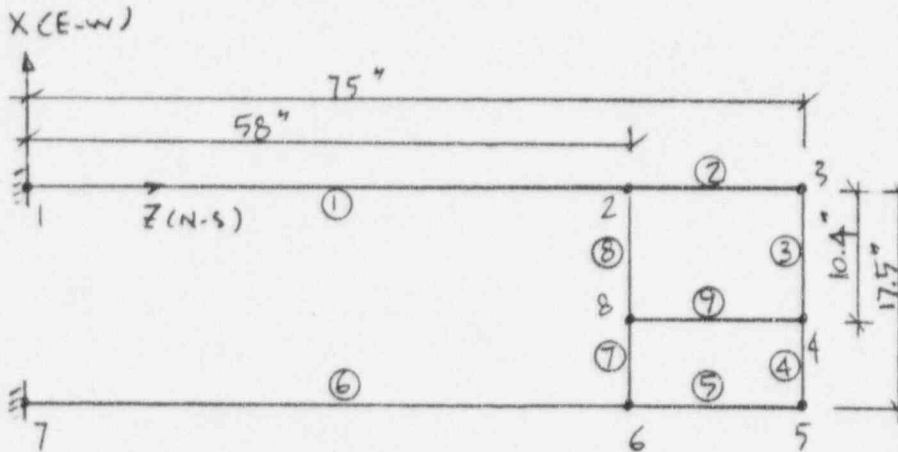
DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION SYSTEM 2850-1

CALCULATION SET NO. DC-5766			REV	COMP BY	CHK'D BY
PRELIM	FINAL	VOID	0	M. WINTER DATE 4/17/96	KTW DATE 5/23/96
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JO 76230502					

REF.

COMPUTER MODEL FOR SUPPORT 6M-2850-H-87



DWG.  
10-4221-000-  
6M-2850-H-87-B

LAST JOINT NO. 8  
LAST MEMBER NO. 9  
ALL MEMBERS ARE L4x4x1/4

THIS SUPPORT IS REPRESENTATIVE FOR SUPT.  
6M-2850-H-86 BASED ON GREATER OBE LOADS.  
FOR ANALYSIS OBE LOADS GOVERN SINCE  
 $\frac{SSE}{OBE} < 1.45 = \frac{0.58 F_y}{0.40 F_y}$

FOR COMPUTER ANALYSIS, ADD LATERAL LOADS, CREATED  
BY DEAD LOAD, TO LATERAL SEISMIC LOADS (CONSERV.)

SEE SUPPORT  
LOAD TABLE  
DESIGN CRITERIA  
NO. FERMI-DC-  
76230-1, R/0  
Pg. 19

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PROJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION  
SUBJECT SYSTEM 2850-1

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JO 76 230502				DATE	DATE

REF.

SUPT. 6M-2850-H-87 (CONT'D)

CHECK MEMBER AND WELD STRESSES FOR SUPPORT  
6M-2850-H-87

AXIAL + BENDING STRESS

MAX AXIAL LOAD = 795 # (MEM. 1, @ JT. 1, LOAD COMB. 4)  
(COMPRESSION) (EQUILIBRIUM CHECK)

BENDING MOM. (X1) = 7917 #-ft  
-11- (X2) = 5587 #-ft



$$\Sigma M = 7917 + 5587 = 13504 \text{ #-ft}$$

BY COMPARISON TO SUPT. 6M-2850-H-77 WITH  
M = 8019 #-ft, AXIAL = 3453 # AND MAX. MEMBER IR = 0.060  
AND WELD IR = 0.35, MEMBER AND WELD STRESSES  
WILL BE WITHIN THE ALLOWABLES.

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COMPUTATION  
SHEET

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STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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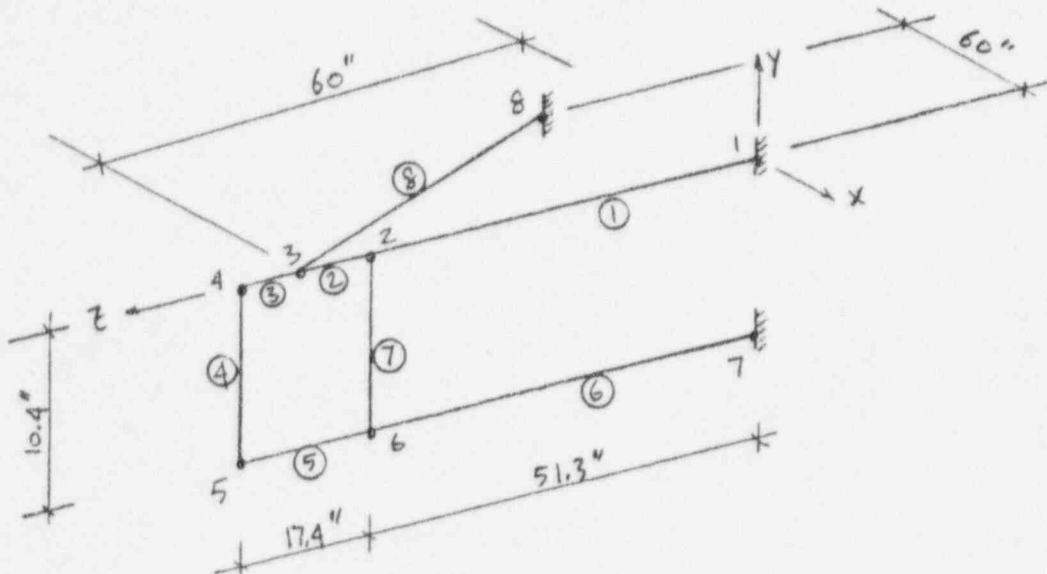
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JO 76230502

REF.

COMPUTER MODEL FOR SUPPORT 6M-2850-H-88



LAST JOINT NO. 8

LAST MEMBER NO. 8

ALL MEMBERS ARE L4x4x1/4

$$\frac{SSEV}{OBEV} = \frac{130}{119} = 1.09 < \frac{0.58 F_y}{0.40 F_y} = 1.45$$

$$\frac{\text{MAX. SSEH}}{\text{MIN. OBEH}} = \frac{377}{339} = 1.11 < 1.45$$

∴ OBE GOVERNS

CONSERVATIVELY ADD LATERAL LOADS CREATED BY DEAD LOADS TO SEISMIC LATERAL LOADS FOR COMPUTER ANALYSIS.

$$F_1 = F_{EW} = 350 + 9 = 359 \#$$

$$F_3 = F_{NS} = 339 + 49 = 388 \#$$

DWG.

10-4221-000-  
6M-2850-H-88

SEE SUPPORT  
LOAD TABLE

DESIGN CRITERIA  
No. FERMI-DC-  
76230-1, R/0,  
Pg. 19

(REF. 1)



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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION  
SYSTEM 2850-1

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JO 76230502					

SUPT 6M-2850-4-88 (CONT'D)

CHECK MEMBER AND WELD STRESSES FOR SUPPORT  
6M-2850-4-88

AXIAL + BENDING STRESS

MAX. AXIAL (COMPRESSION) = 1522# (MEM. 1, EJT. 1, LOAD COMB-4)  
(X3) (EQUILIBRIUM CHECK)

BENDING MOMENT (X1) = 5400# (MEM. 1, EJT. 1, LOAD COMB-4)  
-11- (X2) = 118# (MEM. 1, EJT. 1, LOAD COMB-4)

Σ BENDING MOMENT = 5400 + 118 = 5518#

BY COMPARISON TO SUPT. 6M-2850-4-77 WITH  
AXIAL LOAD = 3453# , MOM = 8019# AND  
MEM. IR = 0.60 AND WELD IR = 0.35 , MEMBER  
AND WELD STRESSES ARE WITHIN ALLOWABLES.

CHECK SHEAR STRESS

MAX TORSIONAL MOMENT = 452# (MEM. 3, LOAD COMB-2,  
BEAM ELEMENT LOADS)

MAX SHEAR (V2) = 902# (MEM. 7, LOAD COMB-3,  
BEAM ELEMENT LOADS)

MAX. SHEAR (V3) = 138# (MEM. 3, LOAD COMB-3,  
BEAM ELEMENT LOADS)

BY COMPARISON TO SUPT. 6M-2850-4-77 , SHEAR  
STRESSES ARE WITHIN THE ALLOWABLES.

REF.

STARDYNE  
OUTPUT FOR  
H88.OUT  
4/22/96  
(ATT. B5)

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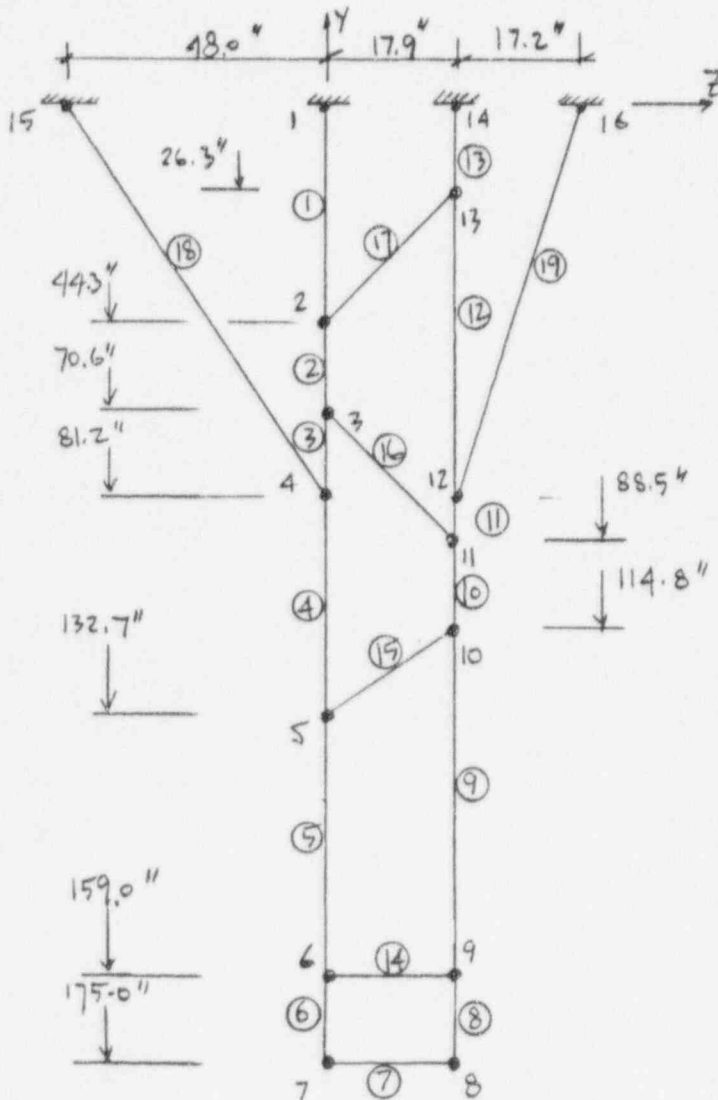
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SHEET

DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION SYSTEM 2850-1

CALCULATION SET NO. DL-5766			REV	COMP BY	CHK'D BY
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JO 76230502					

COMPUTER MODEL FOR SUPPORT GM-2850-H-81



LAST JOINT NO. 16

LAST MEMBER NO. 19

ALL MEMBERS ARE L3x3x1/4

OBE-CASE GOVERNS SINCE OBE-SEISMIC LOADS ARE GREATER THAN SSE-SEISMIC LOADS.

REF.

DWG.  
10-4221-000-  
GM-2850-81-A

SEE SUPPORT  
LOAD TABLE

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 COMPUTATION  
 SHEET

DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

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REF.

SUPT 6M-2850-H-81 (CONT'D)

CHECK MEMBER AND WELD STRESSES FOR SUPPORT  
6M-2850-H-81AXIAL (COMPRESSION) + BENDING

SECTION PROPS FOR L3x3x1/4 =

$$A = 1.44 \text{ IN}^2$$

$$S = 0.977 \text{ IN}^3$$

$$r_x = 0.930 \text{ IN}$$

 AISC-7th ED  
 PG. 1-57  
 (REF. 2)

MEMBER ① OR ③ (CRITICAL)

$$\frac{KL}{r_x} = \frac{10 \times 175}{0.930} = 188 \sim FA = 6400 \text{ PSI}$$

CRITICAL LOADS FOR MEM. ① :

$$\text{COMPRESSION} = 4189 \# \quad (\text{LOAD COMB-2, EQUILIBRIUM CHECK})$$

$$\text{MOMENT} = 495 \# \cdot \text{ft} \quad @ \text{JT. 1}$$

CRITICAL LOADS FOR MEM. ③ :

$$\text{COMPRESSION} = 3323 \# \quad (\text{LOAD COMB-1, EQUILIBRIUM CHECK})$$

$$\text{MOMENT} = 1736 \# \cdot \text{ft} \quad @ \text{JT. 14}$$

 STADYNE  
 OUTPUT FOR  
 H-81  
 4/17/96  
 (ATT. 33)

FOR MEM. ③ :

$$f_a = \frac{3323}{1.44} = 2308 \text{ PSI} \quad \frac{f_a}{F_a} = \frac{2308}{6400} = 0.36 > 0.15$$

$$\frac{f_a}{F_a} + \frac{C_m f_b}{(1 - \frac{f_a}{F_c'}) F_b} \leq 1.0$$

$$C_m = 0.85$$

$$f_b = \frac{M}{S} = \frac{1736}{0.577} = 3009 \text{ PSI}$$

$$F_c' = \frac{12 \pi^2 E}{23 \left( \frac{KL}{r} \right)^2}$$

 HOWEVER, DIVIDE RADIUS OF GYRATION  
 BY FACTOR 1.35 TO CHECK BENDING  
 STRESS ABOUT GEOMETRIC AXES.

 MAX. BUCKLING LENGTH IN PLANE  $L = 44.3$ "  
 (USE  $L = 44.3$ " FOR MEM. 1, CONSERV.)

$$L = 44.3"$$

SEE COMPUTER MODEL

 AISC, 7th ED  
 PG. 5-22,  
 5-23  
 (REF. 2)

 AISC, 9th ED  
 PG. 5-314  
 (REF. 3)

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DECO FERMI 2

PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

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N. Y. WATER

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4/17/96

K. T. W.

DATE

5/25/96

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REF.

SUPT. 6M-2850-H-81 (CONT'D)

$$F_e' = \frac{12 \times \pi^2 \times 29 \times 10^6}{23 \left( \frac{1.0 \times 44.3 \times 1.35}{0.930} \right)^2} = 36074 \text{ PSI}$$

$$0.36 + \frac{0.85 \times 3009}{\left(1 - \frac{2308}{36074}\right) 21600} = 0.49 < 1.0 \therefore \text{OK}$$

BY COMPARISON ALL OTHER MEMBERS ARE OK.

CHECK WELD @ JT. 1, 14, 15 AND 16

$$A = 2 \times 3 = 6 \text{ IN}$$

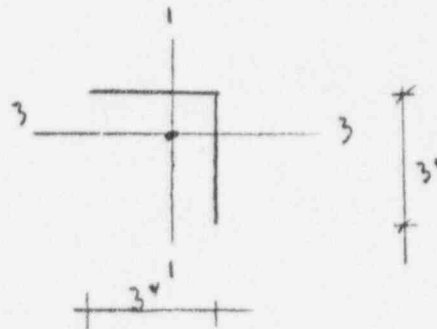
$$S_x = \frac{3^2(4 \times 3 + 3)}{\text{MIN } 6(2 \times 3 + 3)} = 2.5 \text{ IN}^2$$

ENVELOPED WELD LOADS (CONSERV.)

$$F_2 = 4673 \# \text{ (LOAD COMB-3, @ JT. 1)}$$

$$F_3 = 1129 \# \text{ (LOAD COMB-3, @ JT. 15)}$$

$$M_1 = 1961 \# \cdot \text{ (LOAD COMB-1 @ JT. 16)}$$


 STARDYNE  
 OUTPUT FOR  
 4-81  
 4/17/96  
 (ATT. B3)

WELD FORCES:

$$f_2 = \frac{4673}{6} + \frac{1961}{2.5} = 1563 \#/\text{IN}$$

$$f_3 = \frac{1129}{6} = 188 \#/\text{IN}$$

$$f_{R2} = \sqrt{1563^2 + 188^2} = 1574 \#/\text{IN}$$

$$W_{REQD} = \frac{1574}{14400} = 0.11 \text{ IN} < 0.25 \text{ IN} \therefore \text{OK}$$

$$I.R. = \frac{0.11}{0.25} = 0.44 < 1.0$$

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STRUCTURAL QUALIFICATION

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REF.

SUPT. 6M-2850-4-81 (CONTD)

CHECK AXIAL + BENDING STRESS FOR MEMBER ⑮

BENDING MOMENT = 19766 #-ft (@ JT. 5, BEAM ELEMENT LOADS)

COMPRESSION (AXIAL) = 2959 #-ft (LOAD COMB-3)

$$\frac{KL}{r_x} = \frac{10 \times 25}{0.930} = 26 \approx F_A = 20220 \text{ psi}$$

$$f_a = \frac{2959}{1.44} = 2055 \text{ psi} \approx \frac{f_a}{F_A} = \frac{2055}{20220} = 0.10 < 0.15$$

$$f_b = \frac{M}{S} = \frac{19766}{0.577} = 34256 \text{ psi}$$

INCREASE BENDING STRESS 25% TO CHECK AROUND GEOMETRIC AXIS.

$$I.R. = \frac{f_a}{F_A} + \frac{1.25 f_b}{F_b} = 0.10 + \frac{1.25 \times 34256}{21600} = \underline{2.08710}$$

∴ N.G.

SIMILARLY MEMBER 5 @ JT. 6 WITH  $M = 17429$  #-ft  
 MEMBER 10, @ JT. 11 WITH  $M = 16471$  #-ft, MEMBER 14  
 @ JT. 6 WITH  $M = 15184$  #-ft WILL HAVE OVERSTRESS  
 CONDITIONS.

 AISC, 9th Ed  
 Pg. 5-312  
 SECT. 5.2.2.a  
 (REF. 3)

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 PROJECT CCHVAC DUCT & DUCT SUPPORTS  
 SUBJECT STRUCTURAL QUALIFICATION  
SYSTEM 2850-1

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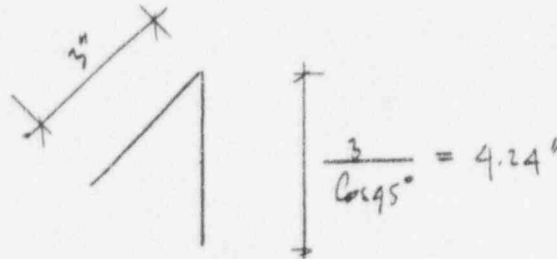
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SUPT. 6M-2850-H-81 (CONF'D)

CHECK WELD FOR MEM. 15 @ JT. 5

$$J = \frac{4.24^3}{12} + \frac{3^3}{12} = 8.6 \text{ IN}^3 \quad (\text{CONSERV.})$$

$$A = 7.24 \text{ IN}$$

WELD LOADS

$$F_{\text{AXIAL}} = 2959 \#$$

$$F_{\text{WEAR}} = 1543 \#$$

$$M_T = 19766 \# \cdot \text{IN}$$

$$f_1 = \frac{1543}{7.24} = 213 \#/\text{IN}$$

$$f_2 = \frac{2959}{7.24} + \frac{19766 \times (2.107)}{8.6} = 5251 \#/\text{IN}$$

$$f_R = \sqrt{213^2 + 5251^2} = 5255 \#/\text{IN}$$

$$W = \frac{5255}{14400} = 0.36 > 0.25 \quad \therefore \text{N.G.}$$

$$IR = \frac{0.36}{0.25} = 1.44$$

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STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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QUALIFICATION OF SUPPORT 6M-2850-H-83

OBE LOADS ARE GOVERNING SINCE MAX. LOAD RATIO  
 $\frac{SSE}{OBE} < \frac{0.58 F_y}{0.40 F_y}$ .

SEE SUPPORT  
LOAD TABLE

SUPPORT ATTACHED TO CEILING. SUPPORT CONFIGURA-  
 TION IS SIMILAR TO THAT OF 6M-2850-H-59.  
 CRITICAL STRESSES ARE AXIAL+BENDING COMBINA-  
 TION. (COMP.)

DEAD LOAD + OBE-VERTICAL (UPWARD) WILL NEVER  
 CREATE COMPRESSION IN VERTICAL POST  
 SINCE DEAD LOAD = 377# > OBE-VERT. = 294#  
 HORIZONTAL LOAD = 31# WHICH IS 3.7 TIMES LESS  
 THAN HORIZONTAL LOAD FOR SUPT. 6M-2850-H-59.  
 (115#)

SEE SUPPORT  
LOAD TABLE

$$\frac{115}{31} = 3.7$$

MAX. MOMENT ARM RATIO =

$$\frac{L_{H-83}}{L_{H-59}} = \frac{175}{136} = 1.29 < 3.7$$

$$\frac{KL}{r} = \frac{10 \times 175}{0.609} = 287 \rightarrow F_A = \frac{12 \times \pi^2 \times E}{23 \left( \frac{KL}{r} \right)^2}$$

$$F_A = \frac{12 \times \pi^2 \times 29 \times 10^6}{23 (287)^2}$$

$$F_A = 1811 \text{ psi}$$

$$\frac{F_A(H-59)}{F_A(H-83)} = \frac{2965}{1811} = 1.64 \rightarrow 1.29 \times 1.64 = 2.1 < 3.7$$

THEREFORE SUPT. H-83 IS OK BY COMPARI-  
 SON TO SUPT. H-59.

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STRUCTURAL QUALIFICATION  
SUBJECT SYSTEM 2850-1

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QUALIFICATION OF SUPPORT 6M-2850-H-57

MEMBERS ARE L3x3x 1/4 WITH MAX. MAIN MEMBER LENGTH 65'.

$$\text{MAX RATIO} : \frac{SSE_{\text{LATERAL}}}{OBE_{\text{LATERAL}}} = \frac{618 (F_3)}{435 (F_3)} = 1.42$$

$$1.42 < \frac{0.58 F_y}{0.40 F_y} = \text{MIN. ALLOW. RATIO} \therefore \text{OBE GOVERNS}$$

THIS SUPPORT HAS FOUR KICKERS RESISTING ALL LATERAL LOADS. BY COMPARISON TO SUPPORT 6M-2850-H-54 (SYSTEM 2850-2) WITH L2 1/2 x 2 1/2 x 1/4 AND MAIN MEMBER LENGTH 112.7" AND LATERAL LOAD = 534 #, MAX. IR = 0.82, SUPT. 6M-2850-H-57 WILL HAVE MEMBER AND WELD STRESSES WITHIN THE ALLOWABLES.

DWG.  
0-424-000-  
6M-2850-57  
DESIGN CRITERIA  
NO. FERMI-  
DC-76230-1,  
REV. 0, Pg. 19



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STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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JO 76230502

QUALIFICATION OF SUPT. 6M-2850-H-76 AND SUPT.  
6M-2850-H-80

SIMILAR CONFIGURATION TO SUPT. 6M-2850-H-63,  
(SYSTEM 2850-2) WITH  $L_{POST} = 136.7"$  AND LATERAL  
LOAD = 204 # AND MEMBER IR = 1.15 > 1.0.

$$F_A = 2965 \text{ psi}$$

H-63

$$F_A = 1811 \text{ psi (SEE SIMILAR SUPT. 6M-2850-H-83)}$$

H-80 OR H-76

MIN. LATERAL LOAD FOR SUPT H-80 OR H-76 IS  
170 # ( $F_3$ ). < 204 #.

HOWEVER, BASED ON LONGER MOMENT ARM AND  
LESS COMPRESSION STRESS ALLOWABLE ( $F_A$ ), SUPPORT  
H-76 AND H-80 WILL HAVE MEMBER STRESS  
FAILURE.

SEE SUPPORT  
LOAD TABLE

QUALIFICATION OF SUPPORT 6M-2850-H-78

BY COMPARISON TO SUPT. H-54 (SYSTEM 2850-2), THIS  
SUPPORT IS ACCEPTABLE BASED ON SHORTER MOMENT ARM  
AND LESS LOADS.

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PROJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION  
SUBJECT SYSTEM 2850-1

CALCULATION SET NO. DC-5766			REV	COMP BY	CHK'D BY
PRELIM	FINAL	VOID	0	N. Y. HETER DATE 4/18/96	← T.W. DATE 5/23/96
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RESULTS OF SUPPORT EVALUATIONS OF SYSTEM 2850-1

SUPT. MARK NO.	MEMBER STRESS	WELD STRESS	BASE R.	REPRESENTED BY SUPT.	REMARKS
6H-2849-H-57					SHARED WITH SYST. 2849-3 (F)
-H-58					
-H-61					
-H-62					
6H-2850-H-84	OK	OK		6H-2850-1-H-11	
-H-73	OK	OK		6H-2850-H-77	
-H-74	NG	NG			FAILS BY COMPARISON TO H-77
-H-75	OK	OK			
-H-76	NG	NG		6H-2850-H-63	← SYSTEM 2850-2
-H-77	OK	OK			ITSELF ANALYSIS SUPPORT
-H-78	OK	OK		6H-2850-H-54	← SYSTEM 2850-2
-H-79	OK	OK		6H-2850-H-77	
-H-80	NG	NG		6H-2850-H-63	← SYSTEM 2850-2
-H-81	NG	NG			ITSELF ANALYSIS SUPPORT
-H-82	OK	OK		6H-2850-H-77	
-H-83	OK	OK		6H-2850-H-54	← SYSTEM 2850-2
-H-57	OK	OK		6H-2850-H-54	← SYSTEM 2850-2
-H-85	OK	OK		6H-2850-H-11	← SYSTEM 2850-1
-H-86	OK	OK		6H-2850-H-67	← SYSTEM 2850-1
-H-87	OK	OK			ITSELF ANALYSIS SUPPORT
-H-88	OK	OK			ITSELF ANALYSIS SUPPORT
6H-2850-1-H-3					SHARED WITH SYST. 2850-3
6H-2850-1-H-11	OK	OK			ITSELF ANALYSIS SUPPORT

FAILING SUPPORTS WILL BE QUALIFIED IN THE NEXT PAGES DUE TO FIELD MODIFICATIONS.

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PROJECT STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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SUPPORT LOADS FOR SYSTEM 2850-1  
BASED ON 1ST STRING ANALYSIS MODIFICATION:

REF

SUPPORT MARK NODE POINT	DIR.	SUPPORT REACTION			SHARED WITH SYSTEM
		DL (LBS)	OBE (LBS)	SSE (LBS)	
GM-2849- H-57 130	VERT	168	62	69	2849-3 (F)
	LATERAL	0	196	247	
GM-2849- H-58 120	VERT	260	160	179	2849-3 (F)
	LAT.	0	77	95	
GM-2849- H-61 170	VERT	329	166	188	2849-3 (F)
	LAT.	0	26	23	
GM-2849- H-62 180	VERT	273	68	59	2849-3 (F)
	LAT	0	46	40	
GM-2850- H-84 138	VERT	406	134	135	
	LAT	0	88	112	
H-73 155	VERT	596	205	210	
	LAT	F <sub>1</sub> =12	422	F <sub>1</sub> =516 F <sub>2</sub> =55	
H-74 190	VERT	353	86	73	
	LAT.	0	320	382	
H-75 200	VERT	242	60	51	
	LAT	5	418	380	
H-76 210	VERT	236	57	47	
	LAT	0	145	119	
H-77 270	VERT	534	240	297	
	LAT.	0	714	F <sub>1</sub> =805 F <sub>2</sub> =58	
H-78 370	VERT	357	131	125	
	LAT.	F <sub>3</sub> 7	248	303	

STRING  
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PROJECT STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO

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SUPPORT LOADS FOR SYSTEM 2850-1  
BASED ON 1 ST STRING ANALYSIS MODIFICATION : CONT

REF

SUPPORT MARK NODE POINT	DIR.	SUPPORT REACTION			SHARED WITH SYSTEM
		DL (LBS)	OBE (LBS)	SSE (LBS)	
6M-2850-H-79 285	VERT	503	154	140	
	LATERAL	0	115	134	
H-80 295	VERT	190	64	51	
	LAT.	0	8	10	
H-81 305	VERT	359	108	82	
	LAT.	F <sub>2</sub> = 7	261	368	
H-82 320	VERT	160	153	92	
	LAT	F <sub>1</sub> = 8	336	381	
H-83 330	VERT	377	244	152	
	LAT	0	31	36	
H-57 100	VERT	371	223	248	
	LAT	F <sub>3</sub> = 19	F <sub>1</sub> = 289 F <sub>2</sub> = 410	F <sub>1</sub> = 341 F <sub>3</sub> = 577	
H-85 248	VERT	377	223	286	
	LAT.	F <sub>1</sub> = 5	85	97.	
H-86 407	VERT	64	54	60.	
	LAT	F <sub>1</sub> = 32 F <sub>2</sub> = 46	F <sub>1</sub> = 112 F <sub>3</sub> = 152	F <sub>1</sub> = 132 F <sub>3</sub> = 185	
H-87 380	VERT	0	0	0	
	LAT	F <sub>1</sub> = 24	F <sub>1</sub> = 266 F <sub>3</sub> = 74	F <sub>1</sub> = 299 F <sub>3</sub> = 63	
H-88 395	VERT	178	119	132	
	LAT.	F <sub>2</sub> = 50	F <sub>1</sub> = 265 F <sub>3</sub> = 260	F <sub>1</sub> = 326 F <sub>3</sub> = 315	
2850-1- H-3 230	VERT	154	37	31	2850-3
	LAT.	0	291	238	

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PROJECT STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO

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SUPPORT LOADS FOR SYSTEM 2850-1  
BASED ON 1ST STRING ANALYSIS MODIFICATION : CONT.

REF

SUPPORT MARK NODE POINT	DIR.	SUPPORT REACTION			SHARED WITH SYSTEM
		DL (LBS)	OBE (LBS)	SSE (LBS)	
GM-2850-H-11 350	VERT	64	86	51	STRING ANALYSIS OUTPUT 4/25/96
	LATERAL	0	74	92	
<del>H-</del>	VERT				
<del>H-</del>	LAT.				
<del>H</del>	VERT				
<del>H</del>	LAT				
<del>H</del>	VERT				
<del>H</del>	LAT				
<del>H-</del>	VERT				
<del>H-</del>	LAT				
<del>H-</del>	VERT				
<del>H-</del>	LAT				
<del>H-</del>	VERT				
<del>H-</del>	LAT				
<del>H-</del>	VERT				
<del>H-</del>	LAT				

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STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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SUPPORT LOADS FOR SYSTEM 2850-1 AFTER FIELD MODIFICATIONS

SUPPORT MARK NO.	DIRECTION	DEADLOAD	OBE	SSE	SHARED WITH SYSTEM
NODE POINT		[LBS]	[LBS]	[LBS]	
6H-2849-H-57	VERT.	167	58	66	2849-3 (F)
130	LATERAL	0	227	282	
-H-58	VERT.	260	110	124	2849-3 (F)
120	LATERAL	0	72	93	
-H-61	VERT.	329	172	201	2849-3 (F)
170	LATERAL	0	13	19	
-H-62	VERT.	273	68	59	2849-3 (F)
180	LATERAL	0	19	26	
6H-2850-H-84	VERT.	406	133	137	
138	LATERAL	0	106	132	
-H-73	VERT.	596	208	226	
155	LATERAL	12	506	610	
-H-74	VERT.	353	86	73	
190	LATERAL	0	265	391	
-H-75	VERT.	242	60	51	
200	LATERAL	5	194	233	
-H-76	VERT.	236	57	47	
210	LATERAL	0	298	409	
-H-77	VERT.	534	239	297	
270	LATERAL	0	673	807	
-H-78	VERT.	357	123	120	
370	LATERAL	7	240	305	
-H-79	VERT.	503	136	120	
285	LATERAL	0	108	130	
-H-80	VERT.	189	61	47	
295	LATERAL	0	10	11	
-H-81	VERT.	359	107	82	
305	LATERAL	7	234	344	
-H-82	VERT.	160	153	92	
320	LATERAL	8	244	294	
-H-83	VERT.	377	244	152	
330	LATERAL	0	25	30	

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PROJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

REF.

SUPPORT LOADS FOR SYSTEM 2850-1 AFTER FIELD MODIFICATIONS (CONT'D)

SUPPORT MARK No.	DIRECTION	DEAD LOAD	OBE	SSE	SHARED WITH SYSTEM
		[LBS]	[LBS]	[LBS]	
6H-2850-H-57 100	VERT.	371	148	162	
	LATERAL	0	141(F1)	184(F1)	
	LATERAL	19(F3)	427(F3)	591(F3)	
-H-85 248	VERT.	377	229	295	
	LATERAL	5	57	70	
-H-86 407	VERT.	64	57	63	
	LATERAL	32(F1)	126(F1)	147(F1)	
	LATERAL	46(F3)	182(F3)	218(F3)	
-H-87 380	VERT.	0	0	0	
	LATERAL	24(F1)	207(F1)	248(F1)	
	LATERAL	0	77(F1)	82(F1)	
-H-88 395	VERT.	178	127	141	
	LATERAL	0	316(F1)	383(F1)	
	LATERAL	50(F3)	250(F3)	340(F3)	
6H-2850-1-H-3 230	VERT.	154	37	31	2850-3 (R)
	LATERAL	0	74	93	
-1-H-11 350	VERT.	64	86	51	
	LATERAL	0	82	97	

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PROJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION  
SUBJECT SYSTEM 2850-1

REF.

QUALIFICATION OF SUPPORTS (SYSTEM 2850-1) AFTER  
FIELD MODIFICATIONS

IN ORDER TO QUALIFY SOME OVERSTRESSED SUPPORTS AND  
BASE PLATES, SOME FIELD MODIFICATIONS ARE INTRODUCED,  
DUE TO NEW STIFFNESS VALUES OF MODIFIED SUPPORTS,  
A NEW STRING ANALYSIS HAS BEEN DONE AND NEW SUPPORT  
LOADS HAVE BEEN DETERMINED. ACCORDING THE NEW LOADS  
FOLLOWING SUPPORTS ARE STILL ACCEPTABLE BY SIMPLE  
LOAD COMPARISON SINCE NEW SUPPORT LOADS ARE EITHER  
SLIGHTLY GREATER THAN ORIGINAL SUPPORT LOADS OR  
THEY BECAME LESS THAN ORIGINAL LOADS;

- Supp. H-57 (SHARED WITH SYSTEM 2849-3)  
H-58 ( )  
H-61 ( )  
H-62 ( )  
H-84  
I-H-11  
H-75  
H-77  
H-78  
H-79  
H-82  
H-83  
H-57  
H-85  
H-86  
H-87  
H-88  
I-H-3



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STRUCTURAL QUALIFICATION  
 SUBJECT SYSTEM 2850-1

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## QUALIFICATION OF SUPPORTS AFTER FIELD MOD. (CONT'D)

QUALIFICATION OF SUPPORT 6M-2850-H-7A

A NEW HORIZONTAL BRACE (L4x4x1/4) AT ALMOST UPPER DUCT ELEVATION HAS BEEN ADDED TO THE SUPPORT. THIS NEW MEMBER (L ≈ 8'-8") RESIST MAINLY HORIZONTAL LOAD AND BENDING MOMENT CREATED BY HORIZONTAL LOAD AND HELPS RELIEVING CRITICAL STRESSES (COMPRESSION + BENDING) IN VERTICAL POSTS.

CHECK HORIZONTAL BRACE (L4x4x1/4)

AXIAL LOAD = 265 # (TOTAL HORIZONTAL LOAD)

$$\text{MOMENT} = H \times L = \underbrace{265}_{\text{#}} \times \underbrace{15}_{\text{\"}} = 3975 \text{ #}\cdot\text{\"}$$

OBE DUCT CENTER TO CG OF L4x4x1/4

## SECTION PROPS OF L4x4x1/4

$$A = 1.94 \text{ in}^2$$

$$r = 1.25 \text{ in}$$

$$S = 1.05 \text{ in}^3$$

$$f_c = \frac{265}{1.94} = 137 \text{ psi}$$

$$\frac{KL}{r} = \frac{10 \times 104}{1.25} = 84 \Rightarrow F_a = 14900 \text{ psi}$$

$$\frac{f_c}{F_a} = \frac{137}{14900} = 0.009$$

$$f_b = \frac{M}{S} = \frac{3975}{1.05} = 3786 \text{ psi}$$

$$I.R. = 0.009 + \frac{1.25 \times 3786}{19660} = 0.25 < 1.0 \Rightarrow L4x4x1/4 \text{ OK}$$

 ALLOW. BENDING STRESS EVALUATED DUE TO LOCAL  
 BUCKLING (SEE CALC. FOR SYSTEM 2850-2)

 \* FACTOR TO CONSIDER BENDING STRESS ABOUT GEOMETRIC  
 AXIS.

 (REF. 3  
 APP. 94 ED  
 PG. 5-312  
 SECT. 5.2.2.1

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 STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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QUALIFICATION OF SUPP. H-74 (Cont'd)

CHECK 1/2" BOLTS (A-325)

MIN DISTANCE BETWEEN TWO BOLTS = 1.5" (CONSERV.)

CONSIDER ONLY TWO BOLTS RESISTING LOADS FROM COMPRESSION AND BENDING.

ALLOW BOLT SHEAR =  $A \times 15000 \text{ (psi)} = (\pi \times 0.25^2) \times (15000) = 2944 \text{ \#}$ MAX SHEAR =  $\sqrt{265^2 + \left(\frac{3975}{15}\right)^2} = 2663 \text{ \#} < 2944 \text{ \#} \therefore \frac{1}{2} \text{ Bolt OK}$ 
 AISC 7th ED  
 Pg. 5-195,  
 Pg. 5-193  
 (REF. 2)
CHECK GUSSET PL (3/8" THICK)

MIN. HEIGHT = 4" (CONSERV.)

 $A = 3/8 \times 4 = 1.5 \text{ in}^2$  $S = \frac{3/8 \times 4^2}{6} = 1.0 \text{ in}^3$  (FOR MOMENT CREATED BY HORIZONTAL LOAD) $I_{\text{MIN}} = \frac{4 \times (3/8)^3}{12} = 0.0176 \text{ in}^4$  $r_{\text{MIN}} = \sqrt{\frac{I_{\text{MIN}}}{A}} = \sqrt{\frac{0.0176}{1.5}} = 0.108 \text{ in}$  $\frac{KL}{r_{\text{MIN}}} = \frac{1.0 \times (40)}{0.108} = 37 \sim F_A = 19420 \text{ psi (FOR A-36)}$  $f_a = \frac{265}{1.5} = 177 \text{ psi} \sim \frac{f_a}{F_A} = \frac{177}{19420} = 0.009 < 0.15$  $f_b = \frac{M}{S} = \frac{3975}{1.0} = 3975 \text{ psi}$  $I_r = 0.009 + \frac{3975}{21600} = 0.19 < 1.0 \therefore \text{OK}$

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SUBJECT SYSTEM 2850-1

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SUPT. 6M-2850-H-79 (CONT'D)

SINCE LATERAL LOAD MAINLY RESISTED BY HORIZONTAL BRACE, THE REST OF SUPPORT IS ABLE TO RESIST DEAD LOAD + VERTICAL SEISMIC AND ACCEPTABLE BY COMPARISON TO SUPT. H-77 (SYSTEM 2850-1).

QUALIFICATION OF SUPPORT 6M-2850-H-76

GOVERNING LOADS

DL = 236 #

VERT (OBE) = 57 #

HORIZONTAL (OBE) = 298 #

MODIFICATION SIMILAR TO SUPT. H-79. LATERAL LOAD AND MOMENT CREATED BY IT MAINLY RESISTED BY HORIZONTAL BRACE. DEADLOAD AND VERTICAL SEISMIC LOADS ARE RESISTED MAINLY BY VERTICAL MEMBERS. THEREFORE, BY COMPARISON TO SUPT. H-76 (SYSTEM 2850-1) HORIZONTAL BRACE AND BY COMPARISON TO SUPT. H-59 (SYSTEM 2850-2) THE REST OF SUPPORT IS ACCEPTABLE.

QUALIFICATION OF SUPPORT 6M-2850-H-80

ENVELOPED LOADS (OBE &amp; SSE)

DL = 189 #

VERT. (OBE) = 61 #

LATERAL (SSE) = 11 #

BY COMPARISON TO SUPT. H-59 (SYSTEM 2850-2), THIS SUPPORT IS ACCEPTABLE BASED ON MUCH LESS LATERAL LOAD (11# << 115#).

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QUALIFICATION OF SUPPORT 6M-2850-H-81

SUPPORT LOADS AFTER FIELD MODIFICATIONS ;

DL = 359 #

VERTICAL SEISMIC (OBE) = 107 #

HORIZONTAL SEISMIC (OBE) = 234 #

} OBE GOVERNS SINCE  
 $\frac{SSE}{OBE} < 1.6$

BEFORE MODIFICATION SUPPORT HAD FOLLOWING GOVERNING LOADS ;

DL = 359 #

VERTICAL SEISMIC (OBE) = 109 #

HORIZONTAL SEISMIC (OBE) = 1476 #

MAX. IR = 2.08 ( COMPRESSION + BENDING FOR MEM. (5) )  
( SEE PREVIOUS PAGES )

MEMBER FAILURE MAINLY CAUSED BY HORIZONTAL LOAD. SINCE NEW HORIZONTAL LOAD MUCH LESS THAN ORIGINAL LOAD ( 234 # V.S. 1476 # ), MEMBER AND WELD STRESSES ARE WITHIN THE ALLOWABLES BY COMPARISON.

QUALIFICATION OF SUPPORT 6M-2850-H-77

SUPPORT IS MODIFIED TO QUALIFY THE OVERSTRESSED BASE PL. MEMBER AND WELD STRESSES WERE WITHIN THE ACCEPTABLES, MODIFICATION HAS FAVORABLE EFFECT ON MEMBER AND WELD STRESSES. THEREFORE SUPPORT IS STILL ACCEPTABLE.

QUALIFICATION OF SUPPORT 6M-2850-H-73

SUPPORT IS MODIFIED TO QUALIFY OVERSTRESSED BASE PL. MODIFICATION HAS FAVORABLE EFFECT ON MEMBER AND WELD STRESSES DUE TO ADDITIONAL BRACE. THEREFORE SUPPORT IS STILL ACCEPTABLE.

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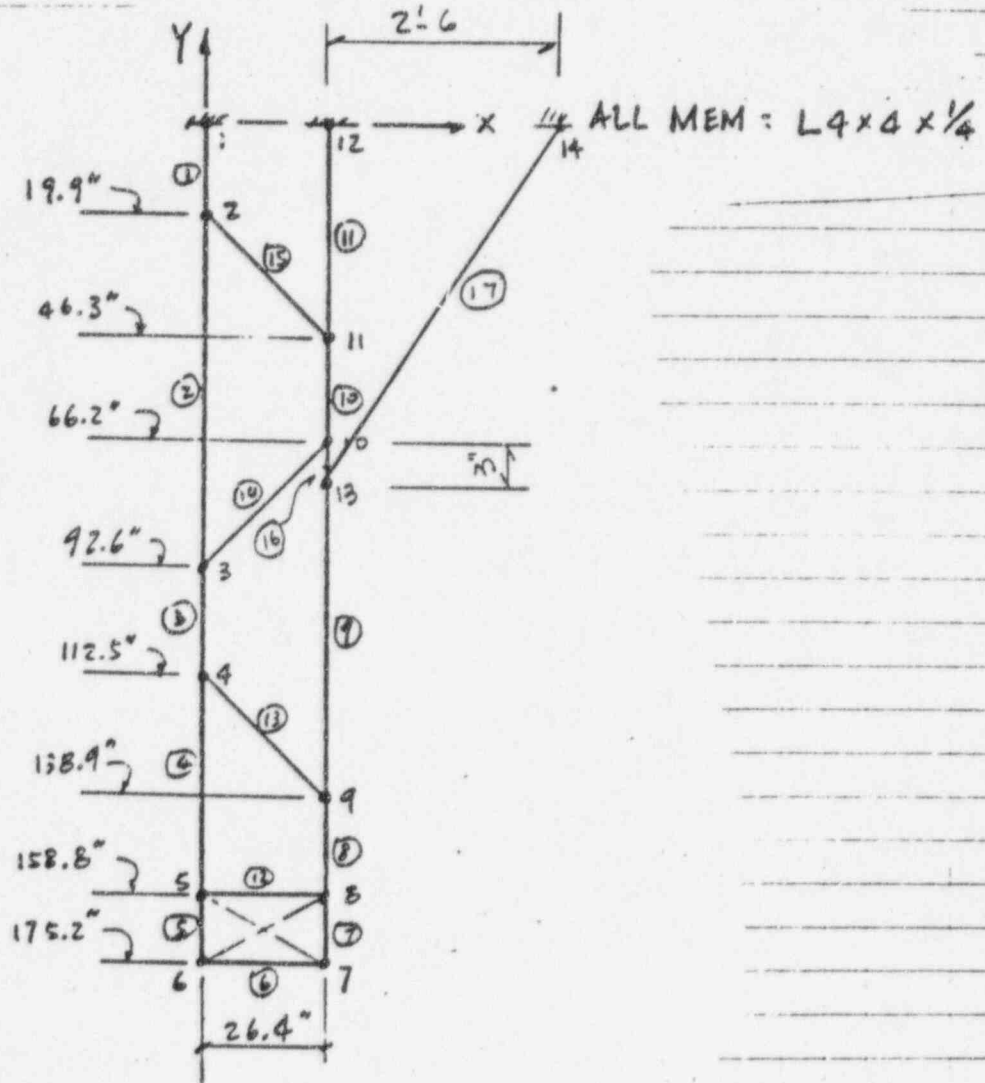
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SYSTEM 2850-1 HANGER 2850-1-473

STADYNE MODEL WITH PROPOSED MODIFICATION (BRACE MEMBER 17) INCLUDED



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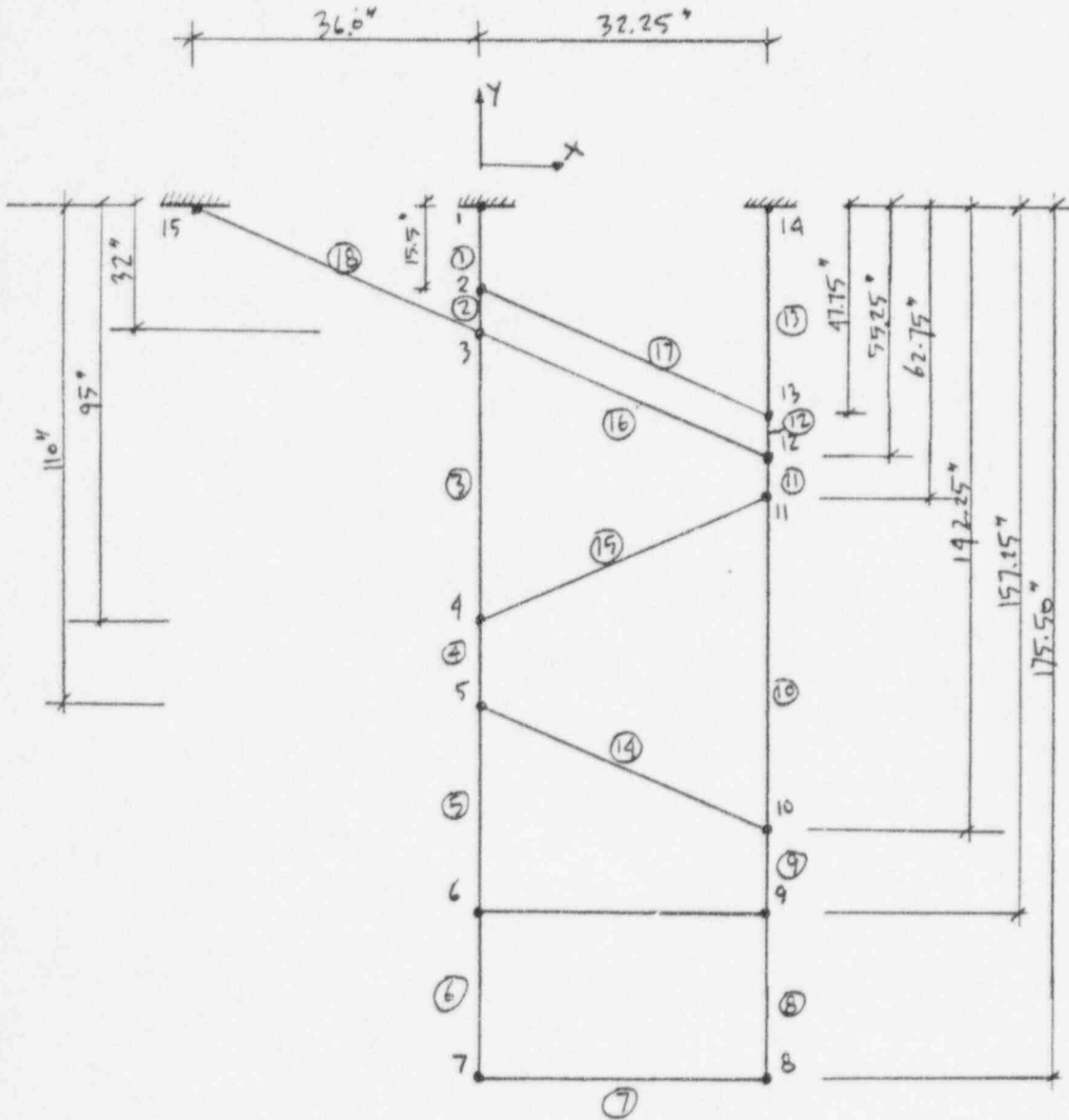
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COMPUTER MODEL FOR SUPPORT 6A-2850-H-77 AFTER FIELD  
MODIFICATION



LAST JOINT NO. 15

LAST MEMBER NO. 18

ALL MEMBERS ARE L4x4x1/4

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SHEET

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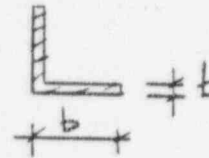
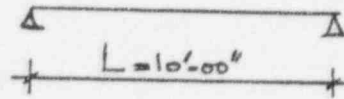
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ALLOWABLE BENDING STRESS FOR EQUAL LEG ANGLE  
BASED ON LATERAL-TORSIONAL BUCKLING



ANGLE SIZE	L [IN]	F <sub>y</sub> [KSI]	F <sub>ob</sub> = $\frac{85900}{(L/b)^2} C_b \left[ 1 + 0.78 \left( \frac{L t}{b^2} \right)^2 - 1 \right]$ [KSI]	C <sub>b</sub> [-]	ALLOWABLE STRESS [KSI]
L2x2x 1/4	120	36	F <sub>ob</sub> = 135.98 > 3 * F <sub>y</sub> = 108	1	F <sub>b</sub> = 0.66 F <sub>y</sub> = 23.76
L2 1/2 x 2 1/2 x 1/4			F <sub>ob</sub> = 125.11 > 3 * F <sub>y</sub> = 108		F <sub>b</sub> = 23.76
L3x3x 1/4			F <sub>ob</sub> = 113.23 > 3 * F <sub>y</sub> = 108		F <sub>b</sub> = 23.76
L3 1/2 x 3 1/2 x 1/4			F <sub>ob</sub> = 101.05 < 3 * F <sub>y</sub> = 108		F <sub>b</sub> = 23.45 *
L4x4x 1/4			F <sub>ob</sub> = 89.19 < 3 * F <sub>y</sub> = 108		F <sub>b</sub> = 22.76 *

AISC, 9<sup>th</sup> ED  
Pg. 5-312,  
5-321.

\*)  $F_b = \left[ 0.95 - 0.50 \sqrt{\frac{F_y}{F_{ob}}} \right] F_y$

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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

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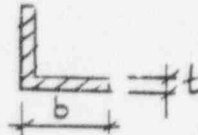
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ALLOWABLE BENDING STRESS FOR EQUAL LEG ANGLE  
BASED ON LOCAL BUCKLING



ANGLE SIZE	b/t	Q [REDUCTION FACTOR]	ALLOWABLE STRESS [KSI]
L2x2x1/4	8	$b/t = 8 < \frac{65}{\sqrt{F_y}} = 10.8$ $\rightarrow Q = 1$	$F_b = 0.66 F_y = \boxed{23.76}$ WITH $F_y = 36$
L2 1/2 x 2 1/2 x 1/4	10	$10 < 10.8$ $\rightarrow Q = 1$	$F_b = \boxed{23.76}$
L3x3x1/4	12	$b/t = 12 > \frac{65}{\sqrt{F_y}}$ $< \frac{76}{\sqrt{F_y}}$ $\rightarrow Q = 1$	$F_b = 0.60 F_y = \boxed{21.6}$
L3 1/2 x 3 1/2 x 1/4	14	$b/t = 14 > \frac{76}{\sqrt{F_y}} = 12.67$ $< \frac{155}{\sqrt{F_y}} = 25.83$ $\rightarrow Q = 1.34 - 0.00447 \left(\frac{b}{t}\right) \sqrt{F_y}$ $Q = 0.96$	$F_b = 0.60 F_y \times Q$ $= 0.60 \times 36 \times 0.96$ $F_b = \boxed{20.73}$
L4x4x1/4	16	$b/t = 16 > \frac{76}{\sqrt{F_y}} = 12.67$ $< \frac{155}{\sqrt{F_y}} = 25.83$ $\rightarrow Q = 1.34 - 0.00447 \left(\frac{b}{t}\right) \sqrt{F_y}$ $Q = 0.91$	$F_b = 0.60 F_y \times Q$ $= 0.60 \times 36 \times 0.91$ $F_b = \boxed{19.66}$

AISC, 9th ED  
PG. 5-311

GOVERNING ALLOWABLES



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SUBJECT SUPPORT EVALUATION FOR  
SYSTEM 2850-2

CALCULATION SET NO.

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SHEET 112 OF 246

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SUPPORT LOADS FOR SYSTEM 2850-2

REF

SUPPORT MARK	DIRECTION	PREVIOUS *			NEW **			
		D.L. (LBS)	OBE (LBS)	I.R. D+OBE	D.L. (LBS)	OBE (LBS)	SSE (LBS)	I.R.
GM-2850-H-49	VERT	324	56		456	219	321	
230	LATERAL	0	40		2	180	300	
H-54	VERT	319	86		422	411	666	
240	LAT.	0	238		2	534	894	
H-55	VERT	376	57		470	129	137	
260	LAT.	0	330		0	390	628	
H-56	VERT	396	52		495	152	184	
275	LAT	0	260		0	310	476	
H-58	VERT	309	83		286	420	687	
305	LAT	0	17		0	47	46	
H-58	VERT	125	16		171	45	45	
380	LAT	0	46		0	56	60	
H-59	VERT	150	148		240	156	247	
325	LAT.	0	44		0	115	101	
H-60	VERT	140	22		199	53	56	
350	LAT	0	90		0	247	205	
H-61	VERT	120	10		175	45	46	
360	LAT	0	142		0	397	327	
H-62	VERT	161	22		215	55	53	
395	LAT.	0	41		0	114	110	
H-63	VERT	122	7		180	43	36	
410	LAT.	0	68		0	204	191	

\* CALC.  
DC 2834

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STRING  
ANALYSIS  
OUTPUT

3/29/96

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PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-Z

CALCULATION SET NO.

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SUPPORT REACTION CONT.

REF

SUPPORT MARK	PREVIOUS *				NEW **			
	DIRECTION	DL (LBS)	OBE (LBS)	I.R. D+OBE	DL (LBS)	OBE (LBS)	SSE (LBS)	I.R.
GM-2850-H-64 420	VERT	56	3		115	28	23	
	LATERAL	0	78		0	220	210	
H-65 460	VERT	265	26		281	131	189	
	LAT.	0	14		0	31	48	
H-65 530	VERT	178	28		200	60	67	
	LAT.	0	15		0	28	42	
H-66 480	VERT	133	136		196	178	267	
	LAT	0	16		0	40	58	
H-67 490	VERT	149	8		251	63	55	
	LAT	0	26		0	83	106	
H-68 500	VERT	178	18		266	69	58	
	LAT	0	43		0	142	162	
H-69 520	VERT	44	7		105	28	24	
	LAT	0	33		0	115	135	
H-70 545	VERT	153	17		209	53	47	
	LAT	0	11		0	35	42	
H-71 560	VERT	173	13		247	63	54	
	LAT	0	43		0	145	166	
H-72 590	VERT	48	6		113	30	26	
	LAT.	0	33		0	128	147	
H-	VERT							
	LAT.							

\* CALC  
DC 2834  
  
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PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-2

CALCULATION SET NO

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SUPPORT MARK No.	VERTICAL MEMBER		HORIZONTAL MEMBER		REMARKS
	SIZE	LENGTH [IN]	SIZE	LENGTH [IN]	
6M-2850-H-49	L3x3x 1/4	136.7	L2x2x 1/4	60.3	
	L2 1/2x2 1/2x 1/4				
-H-54	L2 1/2x2 1/2x 1/4	112.7	L2 1/2x2 1/2x 1/4	35.5	WITH KICKER
-H-55	↓	↓	↓	↓	↓
-H-56	↓	↓	↓	↓	WITH KICKER
-H-58	L3x3x 1/4	≈ 186	L2x2x 1/4	11.7	MULTI DUCT BUT SAME SYSTEM 2850-2
-H-59	L2x2x 1/4	136.6	↓	↓	
-H-60	↓	↓	↓	↓	
-H-61	↓	↓	↓	↓	
-H-62	↓	↓	↓	↓	
-H-63	↓	↓	↓	↓	
-H-64	↓	↓	↓	↓	
-H-65	L2 1/2x2 1/2x 1/4	80.6	↓	≈ 20	MULTI DUCT BUT SAME SYSTEM 2850-2
-H-66	↓	128.8	L2 1/2x2 1/2x 1/4	17.5	
-H-67	↓	128.8	L2 1/2x2 1/2x 1/4	15.5	
-H-68	↓	128.8	L2 1/2x2 1/2x 1/4	15.5	
-H-69	L2x2x 1/4	128.8	L2x2x 1/4	9.2	
-H-70	L2x2x 1/4	128.6	L2x2x 1/4	15.2	
-H-71	L2 1/2x2 1/2x 1/4	128.8	L2 1/2x2 1/2x 1/4	15.5	
↓ -H-72	L2x2x 1/4	128.6	L2x2x 1/4	11.2	

STRESS REPORT  
2850-2  
DWG'S  
10-4221-000-  
6M-2850-H-49  
TO -H-72

BY REVIEWING THE CONFIGURATION AND SUPPORT LOAD TABLE IT IS CONCLUDED THAT HANGER H-59 IS REPRESENTATIVE OF HANGERS H-58, H-62, H-65, H-66, H-67, H-68, H-69, H-70, H-71, H-72 AND H-54 IS REPRESENTATIVE OF H-55 AND H-56 H-49 IS UNIQUE BY ITSELF. SUPPORT H-63 AND BY COMPARISON TO H-64, H-60, H-61 FAIL BASED ON GREATER HORIZONTAL LOAD. (SEE NEXT SHEETS)

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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

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R. SONI

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4/1/96

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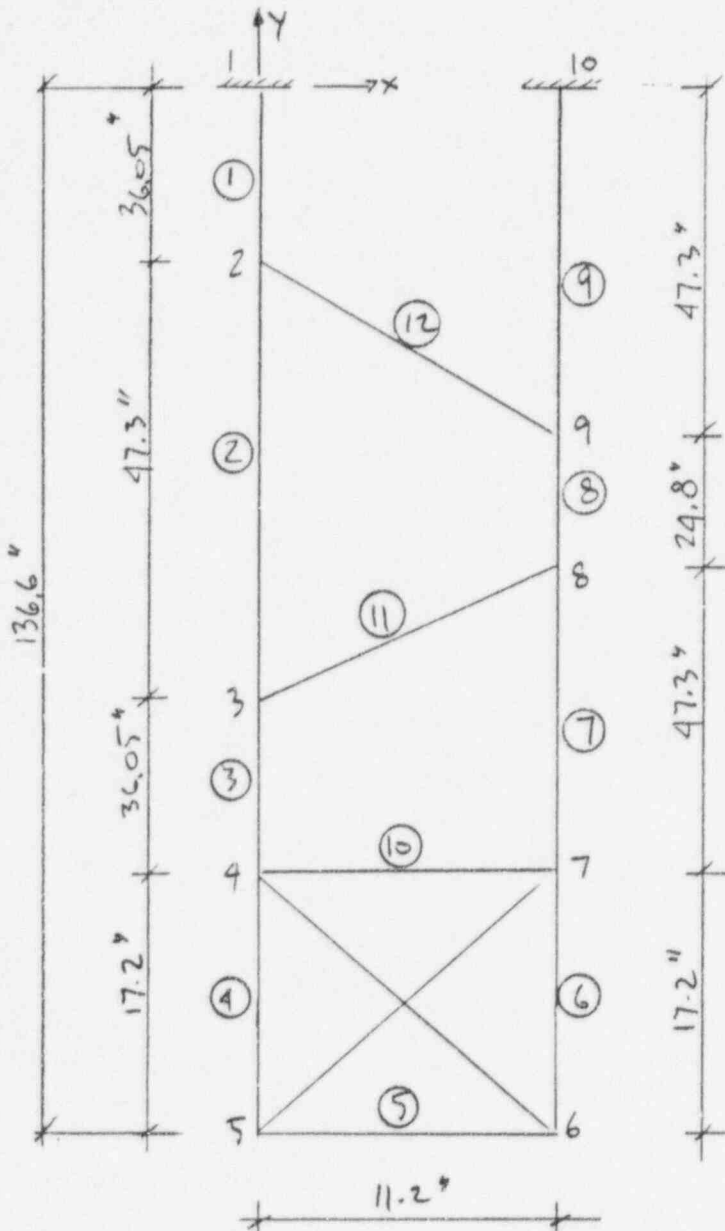
SHEET 115 OF 246

DATE

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STARDYNE - COMPUTER MODEL OF SUPT. 6M-2850-H-59



DWG.  
10-4221-000-  
6M-2850-H-59

MOMENT RELEASE FOR MEMBER 11 & 12 DUE TO SINGLE LINE WELD.

NO. OF LAST MEMBER 12

NO. OF LAST JOINT 10

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REF.

QUALIFICATION OF SUPPORT GM-2850-H-59
CHECK AXIAL + BENDING

ALL MEMBERS ARE L 2x2x1/4.

 MAX. AXIAL (COMPRESSION) LOAD = 996 # } (ENODE 1)  
 MAX. BENDING MOMENT = 1700 #-# } (LOAD COMB. 2)

PROPERTIES OF L 2x2x1/4 :

$$A = 0.938 \text{ IN}^2$$

$$S = 0.247 \text{ IN}^3$$

$$r_x = 0.609 \text{ IN}$$

 $K = 1.0$  (DUCT RESIST THE BUCKLING IN OUT-OF-PLANE)  
 DIRECT.

 $L = 136.6''$  (FULL LENGTH OF VERTICAL POST MEMBER)

$$\frac{KL}{r} = \frac{1.0 \times 136.6}{0.609} = 224.3 > 200$$

$$b/t = 8 < \frac{76}{\sqrt{36}} = 12.7 \quad \text{PREFERABLY SHOULD NOT BE GREATER THAN 200.}$$

 $\sim Q = 1$ 

$$C_c = \sqrt{\frac{2\pi^2 E}{Q \cdot F_y}} = \sqrt{\frac{2 \times \pi^2 \times 29 \times 10^3}{1 \times 36}} = 126$$

$$\frac{KL}{r} > C_c \quad \sim F_a = \frac{12 \pi^2 E}{23 \left(\frac{KL}{r}\right)^2} = \frac{12 \times \pi^2 \times 29 \times 10^6}{23 (224.3)^2}$$

$$F_a = 2965 \text{ PSL}$$

 DWG.  
 10-4221-000-  
 GM-2850-59

 OUTPUT  
 H59.OUT

 AISC, 7th ED  
 Pg. 1-57

 AISC, 9th ED  
 Pg. 5-311

 AISC, 7th ED  
 Pg. 5-16 &  
 5-17

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PROJECT CCHVAC DUC. &amp; DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

CALCULATION SET NO

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REF.

6M-2850-H-59 (CONT'D)

$$f_a = \frac{996}{0.938} = 1062 \text{ psi}$$

$$\frac{f_a}{F_A} = \frac{1062}{2965} = 0.358 > 0.15$$

$$\frac{f_a}{F_A} + \frac{c_m f_b}{(1 - \frac{f_a}{F_e'}) F_b} \leq 1.0$$

$$c_m = 0.85$$

$$f_b = \frac{M}{S} = \frac{1700}{0.247} = 6882 \text{ psi}$$

$$*) F_b = 23760 \text{ psi}$$

$$F_e' = \frac{12 \pi^2 E}{23 \left( \frac{KL}{r} \right)^2}$$

HOWEVER, DIVIDE RADIUS OF  
 GYRATION BY FACTOR 1.35  
 TO CHECK BENDING STRESS ABOUT  
 GEOMETRIC AXES.

MAX. BUCKLING LENGTH IN BENDING  
 PLANE  $L = 47.3''$  (SEE COMPUTER  
 MODEL)

$$F_e' = \frac{12 \times \pi^2 \times 29 \times 10^6}{23 \left( \frac{1.0 \times 47.3 \times 1.35}{0.609} \right)^2}$$

$$F_e' = 13580 \text{ psi}$$

\*)  $F_b$  IS EVALUATED BASED ON BOTH LOCAL  
 BUCKLING AND LATERAL-TORSIONAL BUCKLING.  
 MEMBER SIZES FOR HANGERS IN STRESS REPORT  
 2850-2 FOR BOUNDING CASE (SUPT. H-59) ARE  
 $L2 \times 2 \times 1/4$  WITH ALLOWABLE  $F_b = 23760 \text{ psi}$ .

ALSO, 7<sup>th</sup> ED  
 PG. 5-22,  
 5-23

ALSO, 9<sup>th</sup> ED  
 PG. 5-314

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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT: SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

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SHEET 118 OF 246

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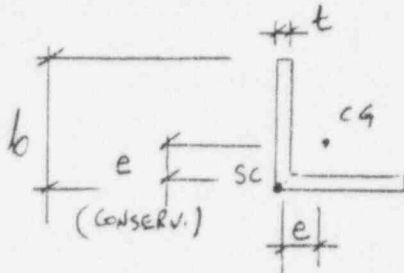
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6M-2850-H-59 (CONT'D)

$$\frac{f_a}{F_A} + \frac{C_m f_b}{(1 - \frac{f_a}{F_e'}) F_b} = \frac{1062}{2965} + \frac{0.85 \times 6882}{(1 - \frac{1062}{13580}) 23760} = 0.63 < 1.0 \therefore \text{OK}$$

OTHER MEMBERS ARE OK BY COMPARISON OF MEMBER LOADS & BUCKLING LENGTH.

CHECK SHEAR FOR L 2x2x1/4



$e = 0.592''$

MAX. TORSIONAL MOMENT =  $\frac{LC - OBE}{(C \text{ VERTICAL POST})}$  HORIZONTAL LOAD  $\times e$

$$= 84 \times 0.592$$

$M_T = 49.7 \#-4$

MAX SHEAR LOAD = 84 # (C NODE 1, LC-2)

$$f_v = \frac{1.5 V_b}{b t} + \frac{3 M_T}{A t}$$

$$= \frac{1.5 \times 84}{2 \times 0.25} + \frac{3 \times 49.7}{0.938 \times 0.25}$$

$f_v = 887.8 \text{ psi} < \frac{0.4 F_y}{*} = \frac{0.4 \times 36000}{*} = 14400 \text{ psi}$

∴ OK

WARPING STRESSES ARE NEGLIGIBLE.

CHECK L 4x4x1/4 (ITEM NO. 9, DWG. 10-4221-000-6M-2850-59-1)

MEMBER STRESSES ARE WITHIN THE ALLOWABLES BY COMPARISON TO L 2x2x1/4 C NODE NO. 1

ASCE 7<sup>th</sup> ED  
Pg. 1-57

SEE OUTPUT  
H59.0HT

ASCE 9<sup>th</sup> ED  
Pg. 5-315 &  
5-316



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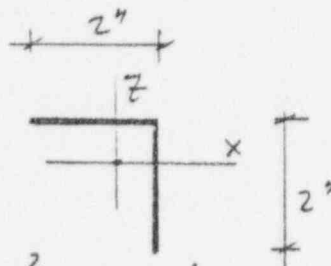
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6M-2850-4-59 (CONT'D)

CHECK WELD

AS MAX LOADS, USE LOADS FROM LC-2, @ NODE 1  
 AND CONSERVATIVELY CONSIDER FOLLOWING WELD  
 CONFIGURATION.



CG OF WELD  $\equiv$  CG OF  $L_{2 \times 2 \times \frac{1}{4}}$   
 (ASSUMED)

WELD PROPS:

$$S_{z_{MAX}} = \frac{4 \times 2 \times 2 + 2^2}{6} = 3.33 \text{ IN}^2$$

$$S_{z_{MIN}} = \frac{2^2(4 \times 2 + 2)}{6(2 \times 2 + 2)} = 1.11 \text{ IN}^2$$

$$A = 2 \times 2 = 4 \text{ IN}$$

WELD LOADS:

$$F_x = 84 \#$$

$$F_y = 1324 \#$$

$$M_z = 1699 \# \cdot \text{IN}$$

(LOAD COMB. 3 @ NODE 1)

$$f_x = \frac{F_x}{A} = \frac{84}{4} = 21 \#/\text{IN}$$

$$f_y = \frac{F_y}{A} + \frac{M_z}{S_{z_{MIN}}} = \frac{1324}{4} + \frac{1699}{1.11} = 1861 \#/\text{IN}$$

$$f_R = \sqrt{f_x^2 + f_y^2} = \sqrt{21^2 + 1861^2} = 1861 \#/\text{IN}$$

$$W_{REQ'D} = \frac{f_R}{0.4 F_T} = \frac{1861}{0.4 \times 36000} = 0.13 \text{ (} \times \text{) } < 2 \times 0.25 = 0.50 \text{ IN} \therefore \text{OK}$$

BASE METAL - SHEAR

$$I_R = \frac{0.13}{0.50} = 0.26$$

 BLODGETT, 1966  
 PG. 7-4-7

 OUTPUT  
 H59.0UT

 AISC, 7th ED  
 PG. 5-16

\*) ALL-AROUND WELD



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CALCULATION SET NO DC-5766			REV.	COMP. BY	CHK'D BY
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SHEET 120 OF 246				DATE	DATE
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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

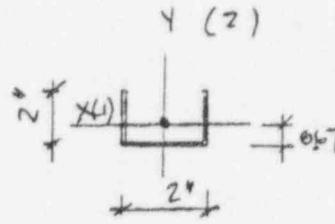
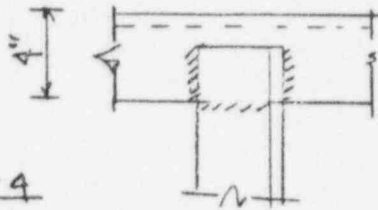
STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

REF.

SUPT. 6M-2850-4-59 (CONFD)

CHECK WELD AT NODE 10



LOAD COMB. - 4

$$\left. \begin{aligned} F_x &= F_1 = 31 \# \\ F_y &= F_2 = 1323 \# \\ M_2 &= M_3 = 928 \#-ft \end{aligned} \right\}$$

$$\begin{aligned} A &= 2 \times 3 = 6 \text{ IN} \\ I_x &= \frac{2^3 (2 \times 2 + 2)}{3 (2 + 2 \times 2)} = 2.67 \text{ IN}^3 \\ I_y &= \frac{2^2 (2 + 6 \times 2)}{12} = 4.67 \text{ IN}^3 \\ I_p &= 2.67 + 4.67 = 7.34 \text{ IN}^3 \end{aligned}$$

$$f_1 = \frac{31}{6} + \frac{928 \times (2 - 0.67)}{7.34} = 174 \#/in$$

$$f_2 = \frac{1323}{6} + \frac{928 \times (2/2)}{7.34} = 397 \#/in$$

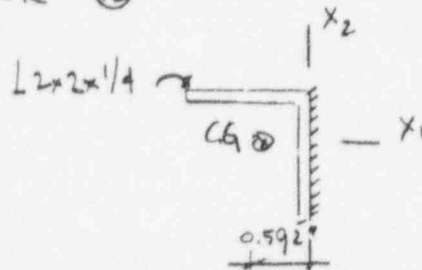
$$f_R = \sqrt{174^2 + 397^2} = 388 \#/in$$

$$w = \frac{f_R}{\frac{0.4 \times F_y}{\text{BASE METAL}}} = \frac{388}{19400} = 0.02 \text{ IN} < 0.25 \text{ IN} \therefore \text{OK}$$

CHECK WELD AT DIAGONAL BRACING

ENVELOPED LOADS FROM LOAD COMB - 1 THRU - 4,  
BEAM END LOADS, MEM. ① OR ②

$$\begin{aligned} F_{x1} &= 73 \# \\ F_{x2} &= 398 \# \\ M_{x3} &= 1930 \#-ft \end{aligned}$$



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DECO FERMI 2

PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

REV

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CHK'D BY

0  
N. YURTER  
DATE  
1/6/96R. SONI  
DATE  
4.8.96PRELIM  
FINAL  
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REF.

SUPT. 6M-2850-H-59 (CONT'D)

CHECK WELD AT DIAGONAL BRACING (CONT'D)F<sub>x2</sub> CREATES ADDITIONAL M<sub>x1</sub> AT WELD.

$$M_{x3} = 1930 \# \cdot \text{ft} \quad , \quad M_{x1} = 398 \times 0.592 = 235.6 \# \cdot \text{ft}$$

$$\text{WELD LENGTH} = \frac{2}{\cos 45^\circ} = 2.828 \text{ ft}$$

WELD FORCES:

$$f_1 = \frac{73}{2.828} + \frac{1930}{1.885} \times \frac{2.828}{2} = 1473.5 \#/\text{in}$$

$$f_2 = \frac{398}{2.828} = 141 \#/\text{in}$$

$$f_3 = \frac{398 \times 0.592}{1.33} = 171 \#/\text{in}$$

$$f_R = \sqrt{1473.5^2 + 141^2 + 171^2} = 1490 \#/\text{in}$$

$$w = \frac{f_R}{0.4 \times F_y} = \frac{1490}{0.4 \times 36000} = 0.103 \text{ in} < 0.25 \text{ in} \quad \therefore \text{OK}$$

$$I_R = \frac{0.103}{0.25} = 0.41 < 1.0$$

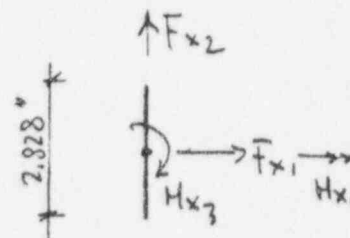
 BY COMPARISON ALL OTHER WELDS FOR SUPT. H-59  
 ARE ACCEPTABLE.

EVALUATION OF MEMBER STRESSES FOR DIAGONAL  
 BRACING (MEM. 11 AND 12)

 AXIAL LOAD CREATES ADDITIONAL BENDING MOMENT  
 BASED ON ECCENTRICITY BETWEEN SINGLE LINE  
 WELD AND C.G. OF ANGLE L 2x2x1/4.

ENVELOPED LOADS FROM LOAD COMB-1 THRU -4

$$\left. \begin{array}{l} \text{AXIAL} = 332 \# \quad (\text{LC-2}) \\ \text{BENDING MOMENT} = 1922 \# \cdot \text{ft} \quad (\text{LC-2}) \end{array} \right\} \text{BEAM ELEMENT LOADS}$$

 OUTPUT  
 H59.OUT


$$A = 2.828 \text{ in}$$

$$S = \frac{2.828^2}{6} = 1.33 \text{ in}^2$$

$$J = \frac{2.828^3}{12} = 1.885 \text{ in}^3$$

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SUPT. 6M-2850-H-59 (CONT'D)

EVALUATION OF MEMBER STRESSES FOR MEM. 11 AND 12  
(CONT'D)

$$\Sigma \text{ BENDING MOMENT} = 1922 + (332 \times 0.592) = 2119 \#-4$$

BY COMPARISON TO MEM. 1 WITH MAX. IR = 0.63  
DIAGONAL BRACES ARE OK BASED ON SHORTER  
BUCKLING LENGTH.

CHECK AXIAL + BENDING FOR SUPT. 6M-2850-H-63

GEOMETRY IS THE SAME AS SUPT. 6M-2850-H-59.  
FROM OUTPUT H63.OUT @ NODE 1, EQUILIBRIUM CHECK

LOAD COMB. - 2 :

AXIAL LOAD = 1804 #  
MOMENT = 2999 #-4

$$f_0 = \frac{1804}{0.938} = 1923 \text{ psi}, \quad F_A = 2965 \text{ psi (SEE H-59)}$$

$$F_{e'} = 13580 \text{ psi (---)}$$

$$\frac{f_0}{F_A} + \frac{C_m f_b}{(1 - \frac{f_0}{F_{e'}}) F_b}$$

$$f_b = \frac{2999}{0.297} = 12142 \text{ psi}$$

$$\frac{1923}{2965} + \frac{0.85 \times 12142}{(1 - \frac{1923}{13580}) 23760} = 1.15 > 1.0 \therefore \text{N.G.}$$

BY COMPARISON SUPT. H-60, H-61 AND H-64 WILL  
FAIL TOO BASED ON GREATER HORIZONTAL LOAD  
AND SAME MEMBER SIZE AND CONFIGURATION.

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 DATE  
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CHECK MEMBERS AND WELDS FOR SSE-CASE

FROM LOAD TABLE

$$DL + V_{SSE} = 240 + 247 = 487 \#$$

$$DL + V_{OBE} = 240 + 156 = 396 \#$$

$$H_{SSE} = 101 \#$$

$$H_{OBE} = 115 \#$$

$$\frac{DL + V_{SSE}}{DL + V_{OBE}} = \frac{487}{396} = 1.22 < 1.6$$

$$\frac{H_{SSE}}{H_{OBE}} = \frac{101}{105} = 0.88 < 1.6$$

 $\therefore$  FOR MEMBER CHECK  
 OBE GOVERNS.

 DESIGN CRITERIA  
 # FERMI-DC-76230-1  
 REV. 1, PG. 19

ALLOWABLE STRESS RATIO FOR SHEAR

$$\frac{0.58 F_y}{0.40 F_y} = 1.45 > 1.22 \quad \left. \begin{array}{l} > 0.88 \end{array} \right\} \therefore \text{OBE GOVERNS}$$

CHECK MEMBER AND WELD STRESSES FOR SUPT. 6M-2850-H-68 AND 6M-2850-H-71

$$\frac{DL + V_{SSE}}{DL + V_{OBE}} = \frac{266 + 58}{266 + 69} = 0.97 < 1.45$$

$$\frac{H_{SSE}}{H_{OBE}} = \frac{162}{142} = 1.14 < 1.45$$

 $\therefore$  FOR SUPT. H-68  
 OBE GOVERNS.

$$\frac{DL + V_{SSE}}{DL + V_{OBE}} = \frac{247 + 54}{247 + 63} = 0.97 < 1.45$$

$$\frac{H_{SSE}}{H_{OBE}} = \frac{166}{145} = 1.14 < 1.45$$

 $\therefore$  FOR SUPT. H-71  
 OBE GOVERNS.

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STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

CALCULATION SET NO

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SUPT. 6M-2850-H-68 AND -H-71 (CONT'D)

VERTICAL AND HORIZONTAL MEMBER SIZES ARE L2½x2½x¼

$$A_{L2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}} = 1.19 \text{ IN}^2$$

$$A_{L2 \times 2 \times \frac{1}{4}} = 0.938 \text{ IN}^2$$

$$S_{L2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}} = 0.394 \text{ IN}^3$$

$$S_{L2 \times 2 \times \frac{1}{4}} = 0.247 \text{ IN}^3$$

 ALSO, 7th ED  
 PG. 1-57

RATIO OF SECTION PROP'S OF ANGLES

$$\frac{A_{L2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}}}{A_{L2 \times 2 \times \frac{1}{4}}} = \frac{1.19}{0.938} = 1.27$$

$$\frac{S_{L2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}}}{S_{L2 \times 2 \times \frac{1}{4}}} = \frac{0.394}{0.247} = 1.59$$

$$\frac{S_{L2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}}}{S_{L2 \times 2 \times \frac{1}{4}}} = \frac{0.394}{0.247} = 1.59$$

FOR REPRESENTATIVE SUPT. 6M-2850-H-59, CRITICAL STRESSES ARE CAUSED BY AXIAL (COMPRESSION) AND BENDING MOMENT WHICH ARE CREATED BY HORIZONTAL LOAD. THEREFORE RATIO OF HORIZONTAL LOADS OF REPRESENTATIVE SUPT. H-59 AND SUPT. H-68, H-71

$$\left. \begin{aligned} \frac{H_{(S\text{UPT. H-68})}}{H_{(S\text{UPT. H-59})}} &= \frac{142 \#}{115 \#} = 1.23 < 1.27 \\ \frac{H_{(S\text{UPT. H-71})}}{H_{(S\text{UPT. H-59})}} &= \frac{145 \#}{115 \#} = 1.26 < 1.27 \end{aligned} \right\} \text{SEE ABOVE}$$

 SEE  
 LOAD TABLE

ALSO, BASED ON COMPARISON OF  $KL/r$  - RATIOS OF CRITICAL MEMBERS (VERTICAL POST) FROM SUPT. H-59, H-68 AND H-71, SUPT. H-59 IS MORE CRITICAL DUE TO LONGER LENGTH AND SMALLER RADIUS OF GYRATION. HENCE, MEMBER STRESSES FOR SUPT. H-68 AND H-71 ARE GOING TO BE WITHIN ALLOWABLES BY COMPARISON TO H-59.

 SEE  
 GEOMETRY TABLE

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PROJECT CCHVAC DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

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SUPT. 6M-2850-H-68 AND -H-71 (CONT'D)

SUPT. H-59 HAS WELD IR = 0.41 (DIAGONAL BRACING)  
MAX

MAX. CRITICAL LOAD (HORIZONTAL) RATIO FOR SUPT.  
H-59 AND H-68 OR H-71 IS 1.26

WELD SIZES FOR ALL THREE SUPT. AT CRITICAL  
LOCATION ARE THE SAME (1/4") AND CONFIGURA-  
TIONS ARE SIMILAR. HENCE MAX. ADJUSTED WELD IR  
FOR SUPT. H-68 AND H-71;

$$\text{ADJUSTED WELD IR} = 0.41 \times 1.26 = 0.52 < 1.0$$

∴ OK

SEE PREVIOUS  
PAGE  
DWG.  
DWG.  
10-4221-000-  
6M-2850-H-68  
AND -H-71

CHECK MEMBER AND WELD STRESSES FOR SUPT. 6M-2850-H-72

$$H_{OBE} = 128 \# \text{ (SUPT. H-72)}$$

$$H_{OBE} = 115 \# \text{ (SUPT. H-59)}$$

CRITICAL MEMBERS, VERTICAL POST, HAVE THE SAME  
SIZE (L 2x2x1/4) AND CRITICAL WELDS WHICH ARE  
AT SUPPORT POINTS, HAVE THE SAME SIZE AND  
CONFIGURATION. HOWEVER, THE LENGTH OF VERTICAL  
POST OF SUPT. H-72 IS L = 128.6" WHICH IS LESS  
THAN L = 136.6" FOR REPRESENTATIVE SUPT. H-59.

$$\frac{KL}{r} = \frac{10 \times 128.6}{0.609} = 211$$

$$F_a = \frac{12 \pi^2 E}{23 \left(\frac{KL}{r}\right)^2} = \frac{12 \times \pi^2 \times 29 \times 10^6}{23 (211)^2} = 3350 \text{ psi}$$

} SUPT. H-72

$$F_a = 2965 \text{ psi FOR SUPT. H-59.}$$

COMPRESSION LOADS CREATED BY DEAD LOAD + UPWARD  
OBE-LOAD IN VERTICAL POST ALMOST THE SAME.

SEE  
LOAD TABLE

SEE TABLE

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SEE

LOAD TABLE

SUPT. GM-2850-4-72 (CONT'D)

$$DL - DBE = 240 - 156 = 84 \# \quad (H-59)$$

$$DL - DBE = 113 - 30 = 83 \# \quad (H-72)$$

COMPRESSION LOAD CREATED BY HORIZONTAL  
 LOAD THROUGH BENDING MOMENT WILL MAKE THE DIFFE-  
 RENCE.

$$\text{COMPRESSION LOAD} = \frac{H \times L}{w} = \frac{128 \times 128.6}{11.2} = 1470 \# \quad (H-72)$$

$$\text{COMPRESSION LOAD} = \frac{115 \times 136.6}{11.2} = 1403 \# \quad (H-59)$$

$1470 \# \approx 1403 \# \quad \therefore$  COMPARE BENDING MOMENTS  
 ONLY.

$$\Sigma \text{BENDING MOMENT} = 128 \times 128.6 \#'' = 16461 \#'' \quad (H-72)$$

$$\Sigma \text{BENDING MOMENT} = 115 \times 136.6 \#'' = 15709 \#'' \quad (H-59)$$

$$\text{RATIO} = \frac{16461}{15709} = 1.05$$

$$\text{ADJUSTED MEMBER IR} = 1.05 \times \frac{H-59}{0.63} = \frac{0.66 < 1.0}{\therefore \text{OK}}$$

$$\text{ADJUSTED WELD IR} = 1.05 \times 0.41 = \frac{0.43 < 1.0}{\therefore \text{OK}}$$

CHECK MEMBER AND WELD STRESSES FOR SUPPORT  
GM-2850-H-54, -H-55 AND -H-56

ALL THREE SUPPORTS HAVE SIMILAR CONFIGURA-  
 TION AND SAME MEMBER SIZES.

BASED ON GOVERNING LOAD (HORIZONTAL LOAD)  
 REGARDING AXIAL + BENDING STRESSES, SUPPORT  
 H-54 IS MORE CRITICAL.

DWG.

10-4221-000-

GM-2850-H-54

-H-55,

-H-56

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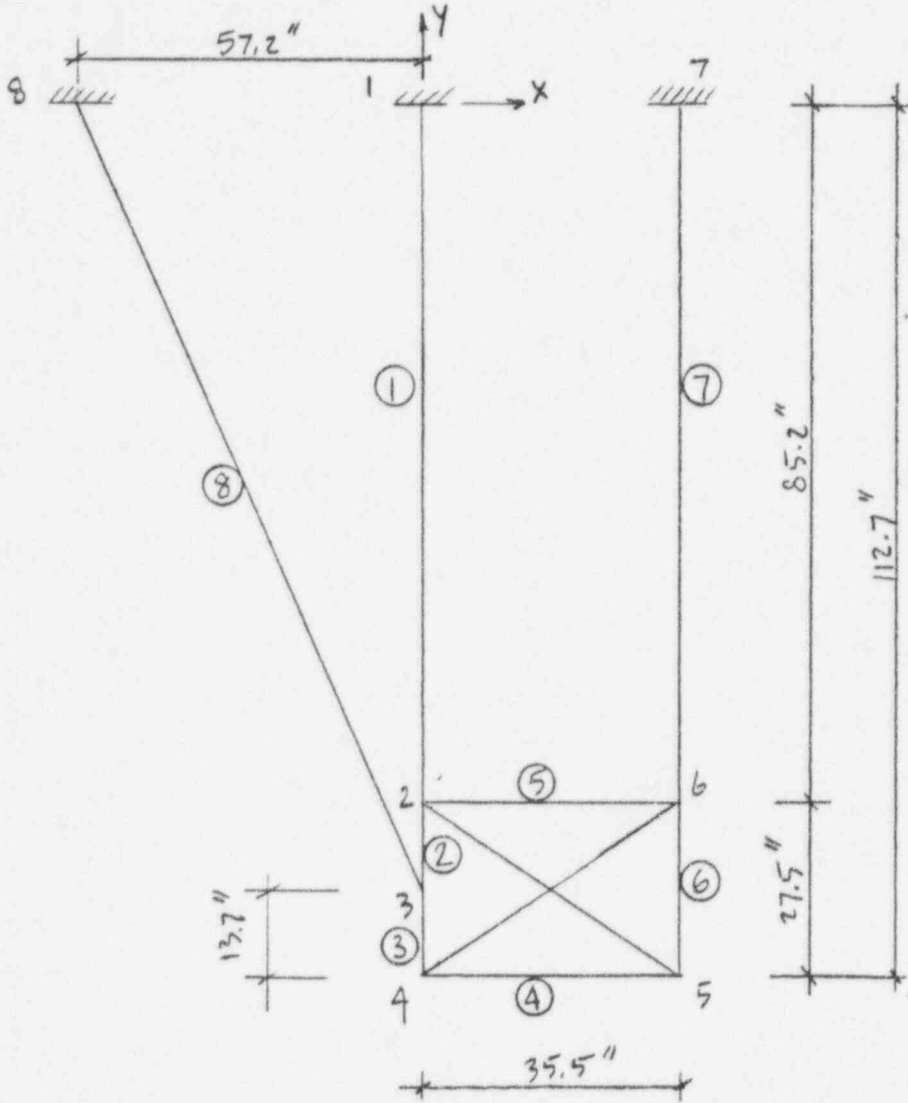
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PROJECT CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION  
SYSTEM 2850-2

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JO 76 230502					

COMPUTER MODEL FOR SUPPORT 6M-2850-H-54



REF  
DWG.  
10-4221-090-6M-  
2850-54-A

No. OF LAST MEMBER 8  
No. OF LAST JOINT 8

ALL MEMBERS ARE  $L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}$



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4.8.96

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SUPT. 6M-2850-H-5A (CONT'D)

FROM COMPUTER OUTPUT (H5A.OUT), REACTIONS  
@ JT. 1, 7 & 8 (GOVERNING LOADS)

	MAX. AXIAL [#]	MAX. MOMENT [#·ft]
LOAD COMB. 1	912	456
2	936	465
3	912	427
4	1345	495
5	1513	776
6	1474	767
7	1514	729
8	2054 (CJT. 1)	814 (CJT. 1)

 WITH GOVERNING LOAD  $V = 2054 \#$  &  $M = 814 \#$   
 FOR MEMBER ① CHECK  $L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}$ .

 REPRESENTATIVE SUPT. H-59 HAS LONGER MEMBER  
 WITH SMALLER SECTION PROPERTIES. ( $L 2 \times 2 \times \frac{1}{4}$ )  
 HENCE LESS AXIAL (COMPRESSION) LOAD ALLOWABLE,  
 HOWEVER, FOR COMPARISON PURPOSE ADJUST  
 THE MEMBER IR OF H-59 BASED ON AXIAL LOAD  
 AND BENDING MOMENT RATIO. (CONSERVATIVE)

$$\begin{aligned}
 IR &= \frac{FA}{FA} + \frac{C_m M}{\left(1 - \frac{FA}{F_c'}\right) F_b} \\
 &= \frac{1062}{2965} \left(\frac{2054}{1062}\right) + \frac{0.85 \times 6882}{\left(1 - \frac{1062}{13580}\right) 23760} \left(\frac{814}{1700}\right) \\
 &\quad \underbrace{\hspace{1.5cm}}_{\text{H-59 LOAD RATIO}} \quad \underbrace{\hspace{1.5cm}}_{\text{H-59}} \quad \underbrace{\hspace{1.5cm}}_{\text{MOM. RATIO}}
 \end{aligned}$$

$$IR = 0.82 < 1.0 \quad \therefore \text{OK}$$

 MEMBER SHEAR STRESSES & WELD STRESSES ARE  
 ACCEPTABLE BY COMPARISON TO SUPT. H-59.

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CHECK MEMBER AND WELD STRESSES FOR SUPT.  
GM-2850-H-58

$\Sigma$  HORIZONTAL LOAD ACTING @ SUPT = 96 + <sup>MAX.</sup> 60 = 106 #

SEE  
LOAD TABLE  
SEE pg. 2

HORIZONTAL LOADS @ SUPT. H-58 ARE LESS THAN HORIZONTAL LOAD FOR SUPT. H-59 (115 #) AND HAVE SHORTER MOMENT ARM TO SUPPORT POINTS. HENCE BENDING MOMENTS FOR SUPT. H-58 WILL BE LESS.

ALSO, CRITICAL MEMBER (VERTICAL POST) FOR SUPT. H-58 HAS GREATER SECTION PROP'S THAN VERTICAL POST OF SUPT. H-59.

(L3x3 1/4 V.S. L2x2 1/4)

DWG.  
10-4221-000-  
GM-2850-59A  
§ -58

LENGTH AND WIDTH OF SUPPORTS ARE THE SAME. THEREFORE BY COMPARISON TO SUPT. H-59, MEMBER AND WELD STRESSES FOR SUPT. H-58 ARE WITHIN THE ALLOWABLES.

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STRUCTURAL QUALIFICATION  
SUBJECT SYSTEM 2850-2

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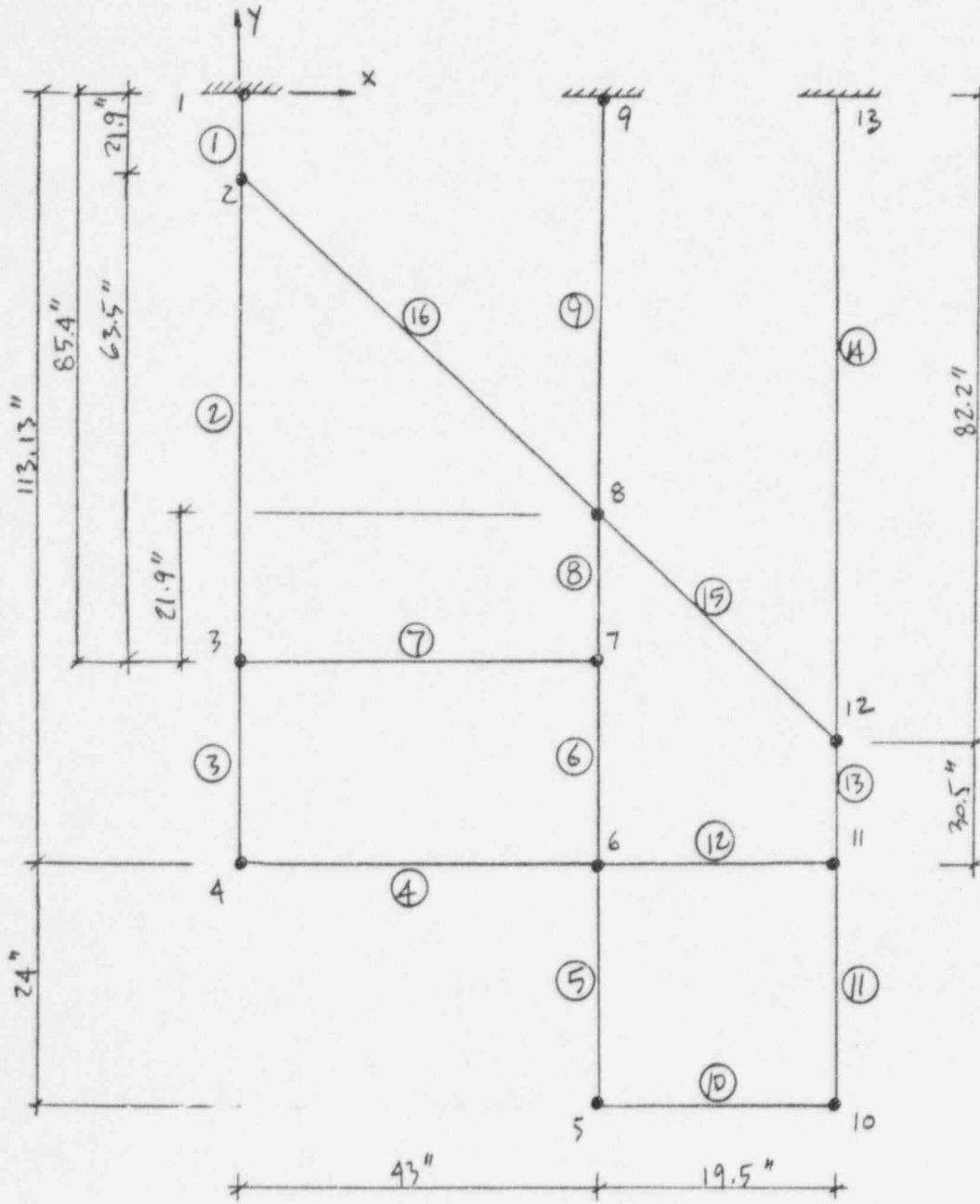
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COMPUTER MODEL FOR SUPT. 6M-2850-H-49



LAST NODE NO. 13

LAST MEMBER NO. 16

DIMENSIONS FOR COMPUTER MODEL ENVELOPS THE VALUES USED IN STRESS REPORT 2850-2 AND 2850-4 SINCE THERE IS NO FIELD INFO. REGARDING DIMENSIONS.

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SUPT. 6M-2850-11-49 (CONT'D)

MEMBER NO. ①, ②, ③, ⑪, ⑬, ⑭ ARE L3x3x1/4  
MEMBER NO. ④, ⑤, ⑥, ⑧, ⑨, ⑩, ⑫ ARE L2 1/2x2 1/2x1/4  
MEMBER NO. ⑦, ⑮, ⑯ ARE L2x2x1/4

MEMBER PROPS FOR L3x3x1/4 :

$A = 1.44 \text{ IN}^2$

$I = 1.24 \text{ IN}^4$

$R = \beta b d^3$  (TORSIONAL RESISTANCE)

$\beta = 0.333$  FOR  $b/d = 3/0.25 = 12$

$R = 2(0.333 \times 3 \times 0.25^3) = 0.031 \text{ IN}^4$

MEMBER PROPS FOR L 2 1/2x2 1/2x1/4 :

$A = 1.19 \text{ IN}^2$

$I = 0.703 \text{ IN}^4$

$R = 2(0.313 \times 2.5 \times 0.25^3) = 0.024 \text{ IN}^4$  FOR  $b/d = 10$

MEMBER PROPS FOR L 2x2x1/4 :

$A = 0.938 \text{ IN}^2$

$I = 0.348 \text{ IN}^4$

$R = 2(0.307 \times 2 \times 0.25^3) = 0.019 \text{ IN}^4$

DEAD LOAD FOR NODE 230 (SYSTEM 2850-2) = 456 #

" " " " " " 114 ( " " " " 2850-4) = 290 #

OBE

VERT. = 219 # } SYSTEM 2850-2  
HORIZ. = 180 # } NODE 230

OBE

VERT. = 147 # } SYSTEM 2850-4  
HORIZ. = 370 # } NODE 114

SSE

VERT = 321 # } SYSTEM 2850-2  
HORIZ = 300 # } NODE 230

SSE

VERT = 136 # } SYSTEM 2850-4  
HORIZ. = 430 # } NODE 114

DL = 456 # (SYSTEM 2850-2, NODE 230)

DL = 290 # ( " " 2850-4, NODE 114)

REF.

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10-9221-000-  
6M-2850-49

ALSC 7MED

PG. 1-57

BUDGETT

PG. 2/10-2

LOAD TABLE

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PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

CALCULATION SET NO.

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6M-2850-H-49 (CONT'D)

CHECK L2x2x1/4 (MEM. ⑦, ⑮ AND ⑯) (DESIGN FOR ENVELOPED LOADS)

 MAX. AXIAL LOAD (SSE) = 1194 # (LOAD COMB.-7, MEM. 16)  
 (BEAM ELEMENT LOADS)
OUTPUT  
H49.0UT
 MAX. BENDING MOMENT (SSE) = 3647 #-ft (LOAD COMB. 8 - MEM. 15)  
 (BEAM ELEMENT LOAD)

 LENGTH OF MEM. ⑯ =  $\sqrt{(35-21.9)^2 + 43^2} = 60"$ 
 $K_x = 0.609"$  $S = 0.247"$  $A = 0.938 \text{ IN}^2$ 
 ALSO CHECKED  
 PG. 1-57 &  
 5-84

 $\frac{KL}{r} = \frac{1.0 \times 60}{0.609} = 99 \sim F_A = 13100 \text{ psi}$ 
 $f_a = \frac{1194}{0.938} = 1273 \text{ psi}$ 
 $\frac{f_a}{F_A} = \frac{1273}{13100} = 0.097 < 0.28 = \frac{f_a}{F_A}$ 

SUPT. H-59

 ALSO, MAX. BENDING MOMENT FOR SUPT H-59 IS  
 3985 #-ft > 3647 #-ft  $\therefore$  L2x2x1/4 IS OK  
 BY COMPARISON TO SUPT. H-59.

CHECK L2 1/2 x 2 1/2 x 1/4 (MEM. ④, ⑤, ⑥, ⑧, ⑨, ⑯, ⑰)

MAX AXIAL LOAD = 877 # (JT #9, LOAD COMB.-3)

MAX. BENDING MOM = 2820 #-ft (MEM. 5, LOAD COMB.-3)

 BY COMPARISON TO L2x2x1/4, L2 1/2 x 2 1/2 x 1/4 IS OK  
 DUE TO LESS LOADS AND GREATER SECTION PROP'S.

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DECO FERMI 2

PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

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6M-2850-H-49 (CONT'D)

CHECK L3x3x1/4 (MEM. ①, ②, ③, ⑪, ⑬, ⑭)

$$A = 1.44 \text{ IN}^2$$

$$S = 0.577 \text{ IN}^3$$

$$r = 0.930 \text{ IN}$$

CONSERVATIVE LOAD COMBINATION SINCE THEY ARE ENVELOPED.

$$\text{MAX. AXIAL LOAD} = 1518 \text{ \# (CJT. 13, LOAD COMB-6)} \left. \vphantom{\text{MAX. AXIAL LOAD}} \right\} \text{SSE}$$

$$\text{MAX. BENDING MOM.} = 11898 \text{ \#-IN (CJT. 1, LOAD COMB-7)} \left. \vphantom{\text{MAX. BENDING MOM.}} \right\}$$

$$\text{MAX. AXIAL LOAD} = 1374 \text{ \# (CJT. 13, LOAD COMB-4)} \left. \vphantom{\text{MAX. AXIAL LOAD}} \right\} \text{OBE}$$

$$\text{MAX. BENDING MOM.} = 8942 \text{ \#-IN (CJT. 1, LOAD COMB-3)} \left. \vphantom{\text{MAX. BENDING MOM.}} \right\}$$

$$\frac{KL}{r} = \frac{10 \times 137.13}{0.930} = 147 \approx F_A = 6910 \text{ PSI}$$

$$f_a = \frac{1374}{1.44} = 954 \text{ PSI}$$

$$\frac{f_a}{F_A} = \frac{954}{6910} = 0.138 < 0.15$$

$$f_b = \frac{M}{S} = \frac{8942}{0.577} = 15498 \text{ PSI}$$

$$F_b = 21600 \text{ PSI (BASED ON LOCAL BUCKLING)}$$

 INCREASE  $f_b$  25% TO CHECK BENDING STRESSES  
 AROUND GEOMETRIC AXIS (CONSERVATIVE)

$$I.R. = \frac{f_a}{F_A} + \frac{1.25 f_b}{F_b} = 0.138 + \frac{1.25 \times 15498}{21600} \approx 1.0 \therefore \text{OK.}$$

NOTE: CONSERVATIVE ALLOW. USED

$$\frac{M_{SSE}}{M_{OBE}} = \frac{11898}{8942} = 1.33 < \frac{\text{FACTOR TO ADJUST FOR SSE-ALLOW}}{1.6}$$

$$\frac{\text{AXIAL (SSE)}}{\text{AXIAL (OBE)}} = \frac{1518}{1374} = 1.1 < 1.6 \left. \vphantom{\frac{\text{AXIAL (SSE)}}{\text{AXIAL (OBE)}}} \right\} \therefore \text{OBE GOVERNS.}$$

HENCE, L3x3x1/4 IS OK

AISC-7th ED  
Pg 5-84

SEE TABLE

AISC, 9th ED  
Pg. 5-312  
SECT. 5.2.2.2DESIGN CRITERIA  
No. FERMI-DC-76230-1  
Pg. 19 REV.0

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GM-2850-H-49 (CONT'D)

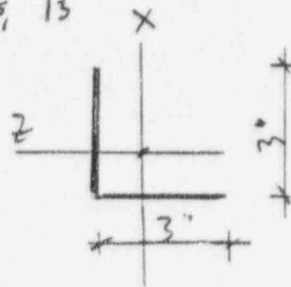
CHECK WELDS @ JT. 1, 9 AND 13

WELD SECTION PROPS @ JT. 1, 9 &amp; 13

$$A = 2 \times 3 = 6 \text{ in}$$

$$S_{\text{MAX}} = \frac{4 \times 3 \times 3 + 3^2}{6} = 7.5 \text{ in}^2$$

$$S_{\text{MIN}} = \frac{3^2(4 \times 3 + 3)}{6(2 \times 3 + 3)} = 2.5 \text{ in}^2$$



USE SSE-LOADS VS OBE ALLOWABLES (CONSERVATIVE)

FOR ENVELOPED AXIAL LOAD & BENDING MOMENT  
SEE PREVIOUS PAGE.FOR MAX SHEAR LOAD @ JT. 1, 9 AND 13 ; SEE  
LOAD COMB. - 7 , @ JT. 1 ;

$$F_x = 756 \#$$

WELD FORCES

$$f_x = \frac{756}{6} = 126 \#/\text{in}$$

$$f_y = \frac{1518}{6} + \frac{11898}{2.5} = 5013 \#/\text{in}$$

$$f_R = \sqrt{126^2 + 5013^2} = 5013 \#/\text{in}$$

$$w = \frac{5013}{\frac{0.4 \times F_y}{\text{OBE}}} = \frac{5013}{14900} = 0.335^* < 2 \times 0.25 = 0.50^* \therefore \text{OK}$$

\*) WELD IS ALL-AROUND  $\therefore$  DOUBLE THE WELD SIZE.

REF.

OUTPUT  
H54.OUT

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SUBJECT SYSTEM 2850-2

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SHEET 135 OF 246				DATE	DATE
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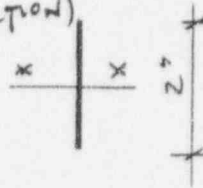
GM-2850-H-49

CHECK SINGLE LINE WELD (MEMBER ④, ⑦, ⑩, ⑬)

MIN WELD LENGTH = 2" (IN VERT. DIRECTION)

$A = 2 \text{ IN}$

$S_x = \frac{2^2}{6} = 0.67 \text{ IN}^2$



MAX. WELD LOADS FOR MEM. ④, ⑦, ⑩ AND ⑬ (SSE)

$M = 2940 \text{ #}\cdot\text{ft}$   
T (JT. 6, MEM. 12, LOAD COMB-8)  
BEAM END LOADS

AXIAL = 291 #

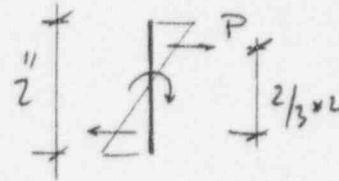
SHEAR = 432 #

$f_x = \frac{291}{2} + \frac{2940}{2/3 \times 2} = 2351 \text{ #/in}$

$f_y = \frac{432}{2} = 216 \text{ #/in}$

$f_r = \sqrt{2351^2 + 216^2} = 2361 \text{ #/in}$

$w_{SSE} = \frac{2361}{0.58 F_y} = \frac{2361}{0.58 \times 36000} = 0.113 \text{ in} < 0.25 \text{ in} \therefore \text{OK}$



OUTPUT  
H49.00T

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6M-2850-H-49 (CONT'D)

CHECK WELD FOR MEMBER (15) &amp; (16)

ANGLE BETWEEN MEMBERS & HORIZONTAL  
 $\approx 45^\circ$ 

$$\text{WELD LENGTH} = \frac{Z}{\cos 45^\circ} = 2.828''$$

$$A = 2.828 \text{ IN}$$

$$S = \frac{2.828^2}{6} = 1.33 \text{ IN}^2$$

FROM BEAM END LOADS, LOAD COMBINATION - 8, MEM. 15,  
 JT. 12 (GOVERNING LOAD COMB. FOR SSE);

$$F_x = 433 \#$$

$$F_y = 761 \#$$

$$M_z = 3650 \#-in$$

WELD FORCES

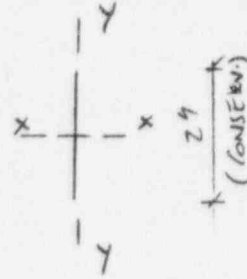
$$f_x = \frac{F_x}{A} + \frac{M}{S} = \frac{433}{2.828} + \frac{3650}{\frac{2}{3} \times 2.828} = 2089 \#/\text{in}$$

$$f_y = \frac{F_y}{A} = \frac{761}{2.828} = 269 \#/\text{in}$$

$$f_R = \sqrt{2089^2 + 269^2} = 2106 \#/\text{in}$$

USE SSE LOADS VS OBE ALLOWABLES (CONSERV.)

$$U_{\text{Ratio}} = \frac{f_R}{\frac{0.9 \times F_y}{\text{OBE}}} = \frac{2106}{0.9 \times 36000} = 0.15 < 0.25 \therefore \text{OK}$$



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CHEW STIFFNESS OF REPRESENTATIVE SUPPORT  
6M-2850-H-59
STIFFNESS IN VERTICAL DIRECTION

 FROM OUTPUT H59T.OUT DEFLECTIONS @ NODE 5 AND 6  
 LOAD CASE -1. (DEAD LOAD OF SUPPORT & DUCT)

$$\Delta = 0.6023657 \times 10^{-3} \text{ ( @ JT. 5 )}$$

$$\Delta = 0.6023757 \times 10^{-3} \text{ ( @ JT. 6 )}$$

$$\text{AVERAGE } \Delta = \left( \frac{0.6023657 + 0.6023757}{2} \right) 10^{-3}$$

$$= 0.6023707 \times 10^{-3}$$

APPLIED LOAD IN VERTICAL DIRECTION = 240 #

$$K_y = \frac{240}{0.6023707 \times 10^{-3}} = 398426 \text{ #/in}$$

$$K_y = 398378 \text{ #/in (FROM STRESS REPORT 2850-2 PG. 14 TO 16)}$$

 THEREFORE SUPPORT STIFFNESS IN VERTICAL  
 DIRECTION IS ACCEPTABLE.

STIFFNESS IN HORIZONTAL DIRECTION

 FROM OUTPUT H59T.OUT DEFLECTIONS @ NODE 4, 5, 6, 7  
 LOAD CASE 3 (HORIZONTAL OBE TX DIRECTION)

$$K_x = (2) \left( \frac{28.8}{0.15377} + \frac{28.8}{0.167} \right) = 720 \text{ #/in}$$

@ JT 4                      @ JT 5

$$K_x = 600 \text{ #/in (FROM STRESS REPORT 2850-2 PG. 14 TO 16)}$$

 BY COMPARISON (720 #/in V.S. 600 #/in) STIFFNESS  
 DIFFERENCE BETWEEN TWO RESULTS ACCEPTABLE.

 \*) DEFLECTIONS @ JT. 4 & 7, @ JT. 5 & 6 AND APPLIED  
 @ ALL FOUR JOINTS ARE THE SAME.

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PROJECT CCHVAC DUCT & DUCT SUPPORTS

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SUBJECT SYSTEM 2850-2

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REF.

RESULTS OF SUPPORT EVALUATIONS OF SYSTEM 2850-2

SHT. MK. NO.	MEMBER STRESS	WELD STRESS	BASE PL.	REPRESENTED BY SUPT.	REMARKS
6H-2850-H-48					QUALIFY LATER, HAS DIFFERENT SYST.
-49	OK	OK		ITSELF	ANALYSIS SUPT.
-54	OK	OK		ITSELF	ANALYSIS SUPT.
-55	OK	OK		H-54	
-56	OK	OK		H-54	
-57					QUALIFY WITHIN 2850-1
-58	OK	OK		H-59	
-59	OK	OK		ITSELF	ANALYSIS SUPT.
-60	NG				FAILED BY COMPARISON TO H-63
-61	NG				↓
-62	OK	OK		H-59	
-63	NG			ITSELF	ANALYSIS SUPPORT
-64	NG				FAILS BY COMPARISON TO H-63.
-65	OK	OK		H-59	
-66	OK	OK		H-59	
-67	OK	OK		H-59	

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DECO FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

SUBJECT STRUCTURAL QUALIFICATION

SYSTEM 2850-2

CALCULATION SET NO

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RESULTS OF SUPPORT EVALUATIONS OF SYSTEM 2850-2

SUPT. MK. No.	MEMBER STRESS	WELD STRESS	BASEP.	REPRESENTED BY SUPT.	REMARKS
6M-2850-H-68	OK	OK		H-59	SEE EVALUATION FOR ADJUSTED IR.
-69	OK	OK		H-59	
-70	OK	OK		H-59	
-71	OK	OK		H-59	SEE EVALUATION FOR ADJUSTED IR.
-72	OK	OK		H-59	↓

FOR QUALIFICATION OF FAILING SUPPORTS SEE NEXT PAGES

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GENERAL  
COMPUTATION  
SHEET  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

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PROJECT

SUBJECT

SUPPORT EVALUATION FOR  
SYSTEM 2850-2

REF

EVALUATION OF BASE PLATE USING NEW MOD ASSESSMENT  
STRING ANALYSIS RUN

SUPPORT MARK NODE POINT	DIR.	SUPPORT REACTION			REMARKS
		DL (LBS)	OBE (LBS)	SSE (LBS)	
GM-2850-H-49 230	VERT	456	232	333	STRING ANALYSIS OUTPUT 4/22/96
	LATERAL	2	182	302	
H-54 240	VERT	422	410	667	
	LAT.	2	534	893	
H-55 260	VERT	470	130	138	
	LAT.	0	390	628	
H-56 275	VERT	495	154	185	
	LAT	0	312	478	
H-58 305	VERT	286	425	693	
	LAT	0	50	57	
H-58 380	VERT	171	45	45	
	LAT	0	83	95	
H-59 325	VERT	240	155	245	
	LAT.	0	77	85	
H-60 350	VERT	199	52	53	
	LAT	0	89	99	
H-61 360	VERT	175	43	39	
	LAT	0	67	97	
H-62 395	VERT	215	55	53	
	LAT.	0	103	116	
H-63 410	VERT	180	43	36	
	LAT.	0	104	117	

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PROJECT

SUPPORT EVALUATION FOR  
SYSTEM 2850-2

SUBJECT

REF

EVALUATION OF BASE PLATE USING NEW MOD ASSESSMENT  
STRING ANALYSIS RUN

SUPPORT MARK NODE POINT	DIR.	SUPPORT REACTION			REMARKS
		DL (LBS)	OBE (LBS)	SSE (LBS)	
GM-2850-H-64 420	VERT	115	28	23	
	LATERAL	0	73	83	
H-65 460	VERT	281	133	192	
	LAT.	0	31	48	
H-65 530	VERT	200	60	67	
	LAT.	0	28	42	
H-66 480	VERT	196	178	267	
	LAT	0	40	58	
H-67 490	VERT	251	63	55	
	LAT	0	85	109	
H-68 500	VERT	266	69	58	
	LAT	0	147	169	
H-69 520	VERT	105	28	24	
	LAT.	0	120	140	
H-70 545	VERT	209	53	47	
	LAT	0	36	43	
H-71 560	VERT	247	63	54	
	LAT	0	151	173	
H-72 590	VERT	113	30	26	
	LAT.	0	133	153	
H-	VERT				
	LAT.				

STRING  
ANALYSIS  
OUTPUT  
4/22/96

4  
↓

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QUALIFICATION OF SUPPORTS 6M-2850-H-60, 6M-2850-H-61,  
 6M-2850-H-63 AND 6M-2850-H-64

THESE SUPPORTS WERE FAILED (SEE PREVIOUS PAGES).  
 AFTER ADDING KICKER TO SUPPORTS 6M-2850-H-61 AND  
 6M-2850-H-64, THE SUPPORT LOADS FOR ALL FAILING  
 SUPPORTS (H-60, H-61, H-63 & H-64) BECAME LESS.  
 WITH ENVELOPED LOADS FOR THE ABOVE MENTIONED SUPPORTS  
 CONSIDERING COMPRESSION + BENDING AS CRITICAL;

MIN. DEAD LOAD = 115 # (SUPT. H-64)	} OBE GOVERNS SINCE SSE LOAD < 1.6 OBE LOAD
MAX. UPWARD (OBE) = 52 # (SUPT. H-60)	
MAX. LATERAL (OBE) = 104 # (SUPT. H-63)	

SEE STRING  
 ANALYSIS OUT  
 PUT 4/22/96

SEE SUPPORT  
 LOAD SHEET  
 AFTER MOD'S.

BY COMPARISON TO SUPT. H-59, ALL ABOVE SUPPORTS ARE  
 ACCEPTABLE BASED ON LESS LOADS.

NOTE

FIELD MODIFICATIONS CONSIDER KICKER NOT ONLY FOR SUPPORTS  
 H-61 AND H-64 BUT ALSO FOR SUPPORTS H-60 AND H-63.  
 THIS WILL HAVE FAVORABLE EFFECT ON MEMBER AND WELD  
 STRESSES OF FAILING SUPPORTS AFTER 1<sup>ST</sup> STRING ANALYSIS  
 BY LESS LOADS. THEREFORE FIELD MODIFICATIONS ARE  
 ACCEPTABLE.

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GENERAL COMPUTATION SHEET  
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DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION  
SUPPORT EVALUATION FOR  
SUBJECT SYSTEM 2850-4

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SHEET 143 OF 246		DATE	DATE	DATE
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SUPPORT NO.	SHARED WITH SYSTEM(S)	TYP	MEM SIZE L	L IN	W IN	DL 16		OBE 16		SSE 16		REMARKS
						VERT	LAT.	VERT.	LAT.	VERT.	LAT.	
6M-2850 -H-28	2850-5 (R) 2850-6 (R)		-	-	-	289	7	822	1291	813	1176	SEE RIGID SYSTEM EVALUATION
6M-2850 -H-48	2850-2 (F) 2850-5,6,7 & 9 (R)		PENETRATION SUPPORT			362	11	680	326	670	322	SEE RIGID SYSTEM EVALUATION
6M-2850 -H-49	2850-2 (F)		-	-	-	289	3	147	369	136	431	SEE SYS. 2850-2
6M-2850 -H-50	NONE		2 1/2	106	12	172	0	51	168	44	185	SUPPORT QUALIFIED
6M-2850 -H-51	NONE	DITTO	2 1/2	106	12	117	0	29	88	24	129	BOUNDED BY H-50 SUPPORT QUALIFIED
6M-2849 -H-3	2849-5 (F)		-	-	-	76	0	28	91	23	87	BOUNDED BY H-4
6M-2849 -H-4	2849-5 (F)		-	-	-	174	0	74	306	67	281	SEE 2849-5
6M-2849 -H-6	2849-5 (F) 2849-9 (F)		-	-	-	268	2	91	574	85	622	SEE 2849-9
6M-2849 -H-7	2850-7 (R) 2850-9 (R)		2	99	12	122	0	55	127	54	127	SUPPORT IS BOUNDED BY 6M-2850-H-59 AND IS QUALIFIED SEE RIGID SYSTEM EVAL FOR OTHER MEMBERS
6M- -H-												

REF: DC 2836 & STRING ANALYSIS FOR SYSTEM 2850-4 3/29/96



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PROJECT STRUCTURAL QUALIFICATION  
SUPPORT EVALUATION FOR  
SUBJECT SYSTEM 2850-4

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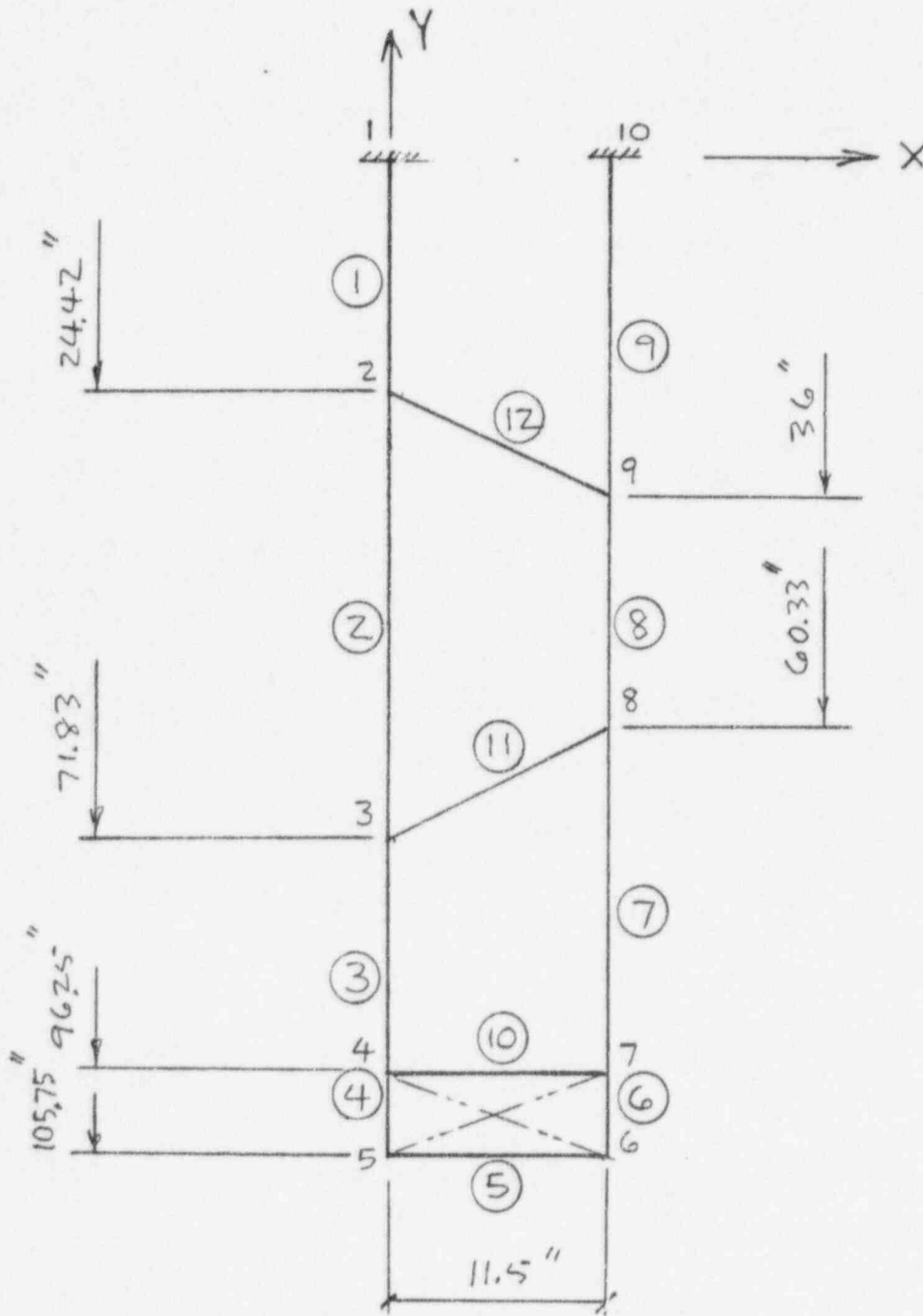
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NOTE :

INFORMATION IS TAKEN FROM DWG. 10-4221-000-GM-2850-50 REV 0  
DC NO. 2836 PAGE 24.

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SUPPORT EVALUATION FOR  
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SUPPORT 2850-H-50

MEMBER CHECK

CONTROLLING MEMBER :

BEAM # 1, JOINT # 1

LOADS :

LOAD	LOAD COMB	AXIAL COMP (LBS)	MOMENT (IN.-LBS)	SHEAR (LBS)	$\frac{DL+SSE}{DL+OBE}$
DL+OBE	Z	1105	2277	147	AXIAL: 1.11 M: 1.11
DL+SSE	G	1231	2521	163	SHEAR: 1.11

 STARDYNG  
 RUN, 4/10/96  
 H-50  
 (ATT. B12)

 SINCE  $\frac{DL+SSE}{DL+OBE} < 1.6$ , THEREFORE OBE GOVERNS
MEMBER SIZE :  $L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}$ 
 PROPERTIES :  $A = 1.19 \text{ in}^2$   
 $S = 0.394 \text{ in}^3$   
 $r = 0.769 \text{ in}$ 
 $L = 105.75 \text{ in}$ ,  $K = 1$ 

$$\frac{KL}{r} = \frac{105.75}{0.769} = 137.5, \quad F_a = 7.84 \text{ KSI}$$

USE 138. = 7840 PSI

$$f_a = \frac{1105}{1.19} = 929 \text{ PSI}$$

$$\frac{f_a}{F_a} = \frac{929}{7840} = 0.12 < 0.15$$

 AISC 7<sup>TH</sup> ED.  
 PAGE 5-84  
 (REF. 2)

 AISC 7<sup>TH</sup> ED  
 PAGE 5-22  
 (REF. 2)

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DECO FERMI 2 SHEET

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM Z850-4

CALCULATION SET NO.

DC-5766

REV

COMP BY

CHK'D BY

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JFS

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DATE

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REF.

SUPPORT Z850-H-50 : CONT

$$f_b = \frac{2277}{0.394} = 5779 \text{ PSI}$$

$$F_b = 21600 \text{ PSI}$$

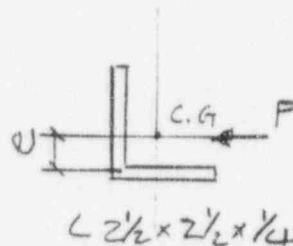
$$\frac{f_a}{F_a} + \frac{f_b}{F_b} = 0.12 + \frac{5779}{21600} = 0.39 < 1.0 \quad \text{OK}$$

CHECK SHEAR

$$e = X - \frac{t}{2} = 0.717 - \frac{0.25}{2} = 0.592 \text{ ''}$$

$$P = 147 \text{ \#}$$

$$M_T = P \cdot e = 147 \times 0.592 = 87 \text{ ''\#}$$



$$f_v = \frac{1.5 V_b}{b \cdot t} + \frac{3 M_T}{A t} = \frac{1.5 \times 147}{2.5 \times 0.25} + \frac{3 \times 87}{1.19 \times 0.25} = 1230 \text{ PSI}$$

$$< 0.4 F_y = 14400 \text{ PSI} \quad \text{OK}$$

OTHER  $L 2\frac{1}{2} \times 2\frac{1}{2} \times \frac{1}{4}$  MEMBERS ARE ADEQUATE BY  
 COMPARISON OF MEMBER LOADS & LENGTHS.

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PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-4

CALCULATION SET NO

DC-5766

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H-50 : CONTMEMBER CHECK : CONT

BRACING MEMBER # 11 &amp; # 12

CONTROLLING MEMBER # 11

DL + OBE, LOAD COMB. #1

AXIAL COMP = 417 #

MOMENT  $M_3 = 1958$  "#
 MOMENT  $M_1 = (\text{AXIAL FORCE}) \times (\text{ECCENTRICITY})$   
 $= 417 \times 0.592 = 247$  "#

MEMBER SIZE : L 2x2x 1/4

PROPERTIES :  $A = 0.938$  IN<sup>2</sup> $S = 0.247$  IN<sup>3</sup> $r = 0.609$  " $L = 11.5 \times \sqrt{2} = 16.26$  "
 $\frac{KL}{r} = \frac{16.26}{0.609} = 27$  ,  $F_a = 20.15$  KSI = 20150 PSI
 $f_a = \frac{417}{0.938} = 445$  PSI $\frac{f_a}{F_a} = \frac{445}{20150} = 0.02 < 0.15$ 
 $f_{b3} = \frac{1958}{0.247} = 7927$  PSI ;  $f_{b1} = \frac{247}{0.247} = 1000$  PSI ,  $F_b = 21600$ 
 $\frac{f_a}{F_a} + \frac{f_{b1} + f_{b3}}{F_b} = 0.02 + \frac{7927 + 1000}{21600} = 0.43 < 1.0$  OK

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DECO FERMI 2

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PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850 - 4

CALCULATION SET NO

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H-50 : CONT

CHECK WELD

CONTROLLING JOINT : JOINT # 1, BEAM # 1

DL+OBE : LOAD COMB # 3

$$F_x = 147 \text{ \#}$$

$$F_y = 1362 \text{ \#} \quad \text{TENSION}$$

$$M_z = 2275 \text{ \#"} \text{ \#}$$

$$M_T = 87 \text{ \#"} \text{ \#}$$

WELD PROPERTIES : 1/4" WELD ALL AROUND

$$A_w = 2.5 \times 2 \times 2 = 10 \text{ \#} \quad b = d = 2.5 \text{ \#}$$

$$S_{z_1} = \left( \frac{4bd + d^2}{6} \right) \times 2 = \left[ \frac{4 \times (2.5) + (2.5)^2}{6} \right] \times 2 = 10.42 \text{ \#}^2$$

$$S_{z_2} = \left[ \frac{d^2(4b+d)}{6(2b+d)} \right] \times 2 = \frac{5}{18} b^2 \times 2 = 3.48 \text{ \#}^2$$

$$N_x = N_y = \frac{d^2}{2(b+d)} = \frac{b}{4} = 0.625 \text{ \#} \approx X = 0.717 \text{ \#}$$

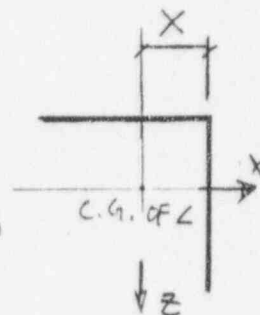
$$f_{a+b} = \frac{F_y}{A_w} + \frac{M_z}{S_{z_2}} = \frac{1362}{10} + \frac{2275}{3.48} = 790 \text{ \#}/\text{ \#}$$

$$f_v = \frac{F_x}{A_w} + \frac{M_T(2.5 - N_x)}{J_w} = \frac{147}{10} + \frac{87 \times 1.875}{13.02} = 27 \text{ \#}/\text{ \#}$$

$$J_w = \left[ \frac{(b+d)^4 - 6b^2d^2}{12(b+d)} \right] \times 2 = \frac{5 \times 2}{12} \cdot b^3 = 13.02 \text{ \#}^3$$

$$f_r = (f_{a+b}^2 + f_v^2)^{1/2} = 790 \text{ \#}/\text{ \#}$$

$$w_{REQ'D} = 790 / 14400 = 0.055 \text{ \#} < 0.25 \text{ \#} \quad \text{OK}$$

DWG 6M-  
2850-50 R/0BUDGETT  
PG 7.4-7  
(REF. 9)

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DECO FERMI 2 SHEET

CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-4

CALCULATION SET NO

DC-5766

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JO. 76230.502

H-50 : CONT

WELD CHECK : CONT

BRACING MEMBERS #11 & #12 HAVE ONE LINE  
WELD OF  $\frac{1}{4}$ "

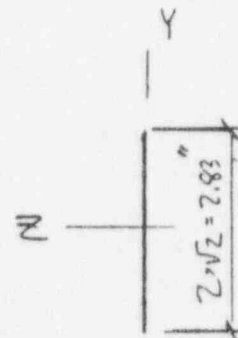
JOINT #8, BEAM #11

DL+OBE GOVERNS

$$F_x = 133 \text{ \#}$$

$$F_y = 457 \text{ \#}$$

$$M_z = 1960 \text{ \#\#}$$



ELEVATION

WELD PROPERTIES :

$$A_w = 2.83 \text{ \"}$$

$$S_w = \frac{d^2}{6} = \frac{2.83^2}{6} = 1.33 \text{ in}^2$$

$$f_{u+b} = \frac{133}{2.83} + \frac{1960}{1.33} = 1521 \text{ \#\#}$$

$$f_u = \frac{457}{2.83} = 161 \text{ \#\#}$$

$$f_r = \sqrt{(1521^2 + 161^2)^{\frac{1}{2}}} = 1530 \text{ \#\#}$$

$$w_{REQD} = \frac{f_r}{0.4 f_y} = \frac{1530}{14,400} = 0.106 \text{ \"} < 0.25 \text{ \"} \text{ OK}$$

<b>Raytheon</b> Engineers & Constructors DECO FERM# 2 CCHVAC DUCT & DUCT SUPPORTS	GENERAL COMPUTATION SHEET			CALCULATION SET NO. DC-5766	REV 0	COMP BY JFS	CHK'D BY ETW
	PROJECT <u>STRUCTURAL QUALIFICATION</u> SUBJECT <u>SUPPORT EVALUATION FOR SYSTEM 2850-4</u>	PRELIM SHEET 150 OF 246 JO. 76230.502	FINAL ✓	VOID	DATE 4-10-96	DATE 4/16/96	DATE _____

SUPPORT STIFFNESS EVALUATION

SUPPORT 2850-H-50 :

LATERAL STIFFNESS :

$$K_X = \frac{42 \times 4}{0.8130183 \times 10^{-1}} = 2066 \text{ #/"}$$

$$K_X = \frac{1000}{0.6453} = 1550 \text{ #/"}$$

VERTICAL STIFFNESS :

$$K_Y = \frac{86.4 \times 2}{0.2647394 \times 10^{-3}} = 652,717 \text{ #/"}$$

$$K_Y = \frac{1000}{0.0015314} = 652,997 \text{ #/"}$$

REF

STARDYNE  
 RUN 4/10/96  
 (ATT. B12)

DC 2836  
 REV A  
 PAGES 27 &  
 29

STARDYNE  
 RUN 4/10/96  
 (ATT. B12)

DC 2836  
 REV A  
 PAGES 27 & 29

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PROJECT DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
SUBJECT STRUCTURAL QUALIFICATION

CALCULATION SET NO <b>DC - 5766</b>			REV	COMP BY	CHK'D BY
PRELIM	FINAL	VOID	0	<b>EA</b>	<b>RS</b>
	✓			DATE <b>6/6/96</b>	DATE <b>6/7/96</b>
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JO	76230.502				

C. ANCHOR AND BASE PLATE EVALUATION



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GENERAL  
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DECO FERMI 2

CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR  
SYSTEM 2850-1

CALCULATION SET NO

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SUPPORT MARK	JOINT NO.	BASE PL. SIZE	ANCHOR BOLT	SUPPORT MEMBER	BASE PLATE LOADS				RESULT	REMARKS
					TENSION (LBS)	MOMENT (FT-LBS)	SHEAR (LBS)	GOVERNING LOAD (CASE)		
6M-2850 -H-57-β	9	8x8x8	4-1/2φ	L3x3x6	2054	68	23	FROM SUPP. 6M-2850 H-54-A	PASS	SEE CALC. FOR BASE PL QUALIFICATION DCN 10881A
	11									
	12									
	13									
	14									
	1			3-1/2φ 1-5/8φ WEDGE ANCH						
6M-2850 -H-73-A		9x8x9	4-1/2φ	L4x4x6					PASS	MODIFIED SUPPORT
			"						PASS	MODIFIED SUPPORT DCN 10938A
			3-1/2φ 1-5/8φ WEDGE ANCH						PASS	SEE CALC.
6M-2850 -H-75			4-1/2φ						PASS	
			"							
6M-2850 -H-76		6x8x6	4-3/8φ	L2x2x4					PASS	MODIFIED SUPPORT
			"	"						

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PROJECT STRUCTURAL QUALIFICATION

SUBJECT SUPPORT EVALUATION FOR SYSTEM 2850-1

CALCULATION SET NO.

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JO 76 230 502

SUPPORT MARK	JOINT NO.	BASE PL. SIZE	ANCHOR BOLT	SUPPORT MEMBER	BASE PLATE LOADS				RESULT	REMARKS
					TENSION (LBS)	MOMENT (FT-LBS)	SHEAR (LBS)	GOVERNING LOAD CASE		
6M-2850 -H-77-B	1	9x5/8x9	4-1/2φ	L4x4x1/4					PASS	MODIFIED SUPPORT
	12	"	"	"						SEE CALC.
6M-2850 -H-78	1	8x1/2x8	4-3/8φ	L2x2x1/2x1/4					PASS	SEE CALC. FOR BASE PL. QUALIFICATION
	7									
6M-2850 -H-79-B	8								PASS	SEE CALC. FOR BASE PL. QUALIFICATION
		9x5/8x9	4-1/2φ	L4x4x1/4						
6M-2850 -H-80		"	"	"					PASS	SEE CALC.
		6x3/8x6	4-3/8φ	L2x2x1/4						
6M-2850 -H-81-A									PASS	MODIFIED SUPPORT
		8x5/8x8	4-1/2φ	L3x3x1/4						
6M-2850 -H-82									PASS	SEE CALC. FOR BASE PL. QUALIFICATION
		9x5/8x9	4-1/2φ	L4x4x1/4						

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DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION  
SUBJECT SUPPORT EVALUATION FOR SYSTEM 2850-1

CALCULATION SET NO

DC-5766

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5/23/96

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SUPPORT MARK	JOINT NO.	BASE PL. SIZE	ANCHOR BOLT	SUPPORT MEMBER	BASE PLATE LOADS				RESULT	REMARKS
					TENSION (LBS)	MOMENT (FT-LBS)	SHEAR (LBS)	GOVERNING LOAD CASE		
6M-2850 -H-83		6x3/8x6	4-3/8φ	L2x2x1/4					PASS	SEE CALC. FOR BASE PL. QUALIFICATION
		"	"	"					PASS	BOUNDED BY 6M-2850-1 -H-11-A
6M-2850 -H-85		8x1/2x8	4-3/8φ	L2 1/2x2 1/2x1/4					PASS	BOUNDED BY 6M-2850-1 -H-11-A
		"	"	"					PASS	SEE CALC. FOR BASE PL. QUALIFICATION
6M-2850 -H-86-B		9x5/8x9	4-1/2φ	L4x4x1/4					PASS	SEE CALC. FOR BASE PL. QUALIFICATION
		A. 9x5/8x9	4-1/2φ	L4x4x1/4	689	403	158	158	PASS	SEE CALC.
6M-2850 -H-87-B	1	B. "	3-1/2φ 1-58φ WEDGE ANCH	"	"	"	"	"	PASS	DEN 10939A
		9x5/8x9	4-1/2φ	L4x4x1/4	1522	X1: 450 X2: 10	204	D TO BE L.C. #4	* PASS	BASE PLATE CHART C, ZONE I SSE > OBE SHEAR IS NEAR
6M-2850 -H-88-	7	"	"	"	comp.				PASS	SSE > OBE SHEAR IS NEAR
		9x5/8x9	4-3/8φ	L4x4x1/4	270	X1: 99 X2: 14	263	D TO BE L.C. #1	* PASS	BASE PLATE CHART C ZONE III. SSE < OBE SHEAR IS NEAR
6M-2850-1 -H-11-A	8	"	"	"	589	12	14	L.C. 1 DL TO BE	PASS	
		8x1/2x8	4-3/8φ	L2 1/2x2 1/2x1/4	642	210	100	L.C. 3 DL TO BE	PASS	

\* BASE PL. LOADS ARE FROM STADYNE OUTPUT FOR H-88 & H-11 (ATT. B5 & ATT. B6)

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SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-1

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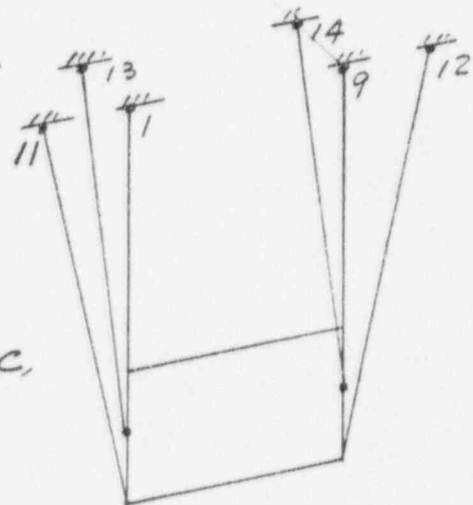
REF: DC 2833 REV. B

BASE PLATE CHECK FOR SUPPORT 6M-2850-H-57-B

SINCE THIS SUPPORT IS ENVELOPED BY SUPPORT 6M-2850-H-54-A, THE MOST CRITICAL BASE PLATE LOADS FOR 6M-2850-H-54-A ARE UTILIZED.

TENSION = 2054 lb  
MOMENT = 68 FT-LB  
SHEAR = 23 lb

BASE PLATES AT JOINTS 9, 11, 12, 13 & 14 ARE QUALIFIED BY BASE PLATE CHART C, ZONE II.



BASE PLATE @ JT. 1 (PLATE A)

ASSUME THAT THE 5/8" WEDGE ANCHOR WILL RESIST 100% TENSION.

$$2054 \text{ lb} < 2858 \text{ lb/ANCHOR}$$

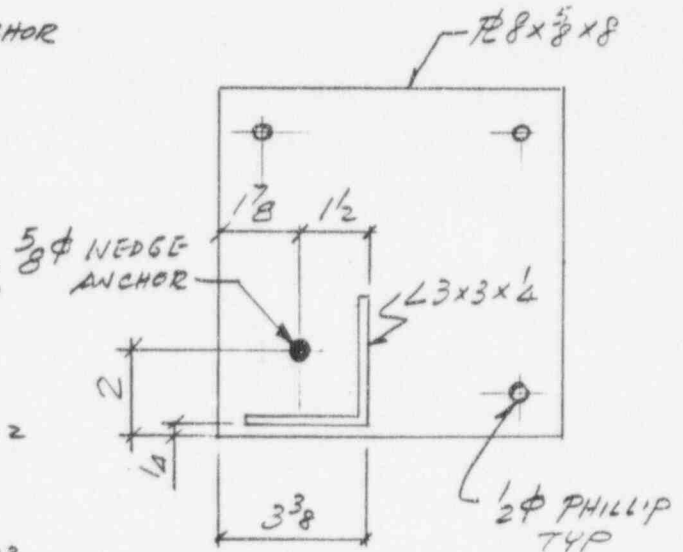
\* REDUCED CAPACITY DUE TO SPACING VIOLATION SEE NEXT SHEET.

CHECK BASE PLATE STRESS

$$f_s = \frac{2054 \times 1.5}{8 \times \frac{0.625^2}{6}} = 5916 \text{ lb/in}^2$$

$$5916 \text{ lb/in}^2 < 27,000 \text{ lb/in}^2 \text{ (ALLOWABLE)}$$

BASE PLATE & ANCHOR BOLTS ARE ADEQUATE.



BASE PLATE @ JT. 1

REF: DCN 10881 REV. A

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PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

SUBJECT STRUCTURAL QUALIFICATION  
SYSTEM 2850-1

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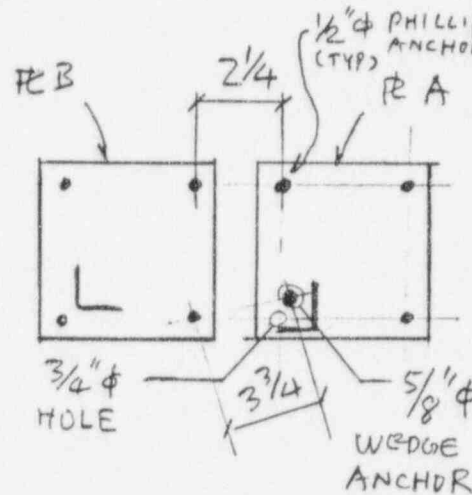
 #  
 DCN 10881  
 REV A

 RED HEAD  
 ENG. BULLETIN  
 # 111

GM-2850-H-57 : CONT

 ANCHOR BOLT SPACING VIOLATIONS WERE DOCUMENTED  
 IN DCN # 10881 REV A

 SPACING BETWEEN TWO  $\frac{1}{2}$ "  $\phi$   
 PHILLIPS ANCHOR IS DOCUMENTED  
 AS  $2\frac{1}{4}$ " < 5" REQ'D.

 SINCE THESE TWO BOLTS ARE  
 AWAY FROM THE SUPPORTING  
 MEMBERS AND SUBJECT TO  
 LOW TENSION. THE REDUCED  
 BOLT TENSILE CAPACITY IS STILL  
 SUFFICIENT TO CARRY THE LOADS.  
 IT IS OK.

 SPACING VIOLATION BETWEEN  $\frac{5}{8}$ "  $\phi$  WEDGE ANCHOR  
 &  $\frac{1}{2}$ "  $\phi$  PHILLIPS ANCHOR IS DOCUMENTED AS  $3\frac{3}{4}$ "  
 WHICH IS <  $(\frac{5}{2} + \frac{4.375}{2}) = 4.69$ " (REQ'D)

 ALLOWABLE TENSION FOR  $\frac{5}{8}$ "  $\phi$  W.A. IS REDUCED  
 $3575 \# \times \frac{3.75}{4.69} = 2858 \# > 2054 \#$  (ACTUAL)  
 OK

 SINCE BASE PLATE STRESS (5,916 PSI) IS LOW  
 COMPARE WITH ALLOWABLE (27,000 PSI),  
 THEREFORE ONE OPEN HOLE OF  $\frac{3}{4}$ "  $\phi$  IN  
 THE PLATE DOES NOT ALTER THE PLATE'S  
 STRUCTURAL ADEQUACY.

THESE VIOLATIONS ARE ACCEPTABLE

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PROJECT STRUCTURAL QUALIFICATIONSUBJECT SYSTEM 2850-1

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RIF

BASE PLATE CHECK FOR SUPPORT 6M-2850-H-73-A

RATIOS OF LOADS FOR SUPPORTS H-73 &amp; H-77

$$\text{DEAD LOAD} = \frac{596}{540} = 1.104$$

$$\text{SSE VERTICAL} = \frac{246}{314} = 0.783$$

$$\text{SSE HORIZONTAL} = \frac{517}{825} = 0.628$$

PROPORTIONED BASE PLATE LOADS FOR SUPPORT H-73  
ARE:

$$F_x = 857 \times 0.628 = 538 \text{ lb}$$

$$F_y = 271 \times 1.104 + 158 \times 0.783 + 3957 \times 0.628 \\ = 2907 \text{ lb}$$

$$M_2 = 25 \times 1.104 + 15 \times 0.783 + 8807 \times 0.628 \\ = 5570 \text{ IN-LB} = 464 \text{ FT-LB}$$

BASED ON THE AS BUILT SKETCH,  
THE BASE PLATE FAILS BY USING BASE PLATE  
CHART C, ZONE IDC 2833  
ABM-0507

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PROJECT STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO			REV	COMP BY	CHK'D BY
DC-5766			0	QFS	KTW
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JO 76230.50Z					

GM-2850-H-73 (MODIFIED)

REF

BASE PLATE LOADS : FROM STARDYNE 2850-H-73  
(FIELD MOD)

ATT. B 7

ATT. D 1

DL + SSE

JOINT # 1 OR # 12

$$\left. \begin{aligned} F_x &= 33 \text{ \#} \\ F_y &= 2921 \text{ \#} \\ M_z &= 313 \text{ \#} \\ &= 26 \text{ \#} \end{aligned} \right\} \text{ JOINT \# 1, L.C. \# 3}$$

CHART C, ZONE I IS UTILIZED

DC 2833  
ABM-0507

$$IR = \frac{2921}{3150} + \frac{26}{1023} = 0.95 < 1.0$$

SHEAR IS NEGLIGIBLE

EVALUATION OF NEW ADDED BASE PLATE AS PER FIELD MOD.

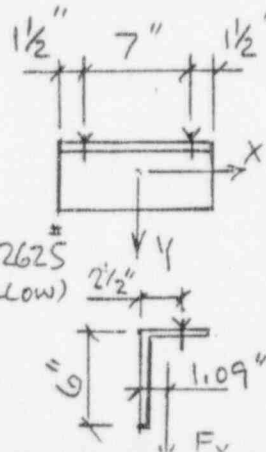
JOINT # 14 : DL + SSE, L.C. # 4  
 $F_x = 648 \text{ \#}, F_y = 1514 \text{ \#}, M_z = 56 \text{ \#}$   
 BASE ANGLE :  $\angle 6 \times 4 \times \frac{1}{4} = 5' \text{ \#}$

A.B. : 2 -  $\frac{1}{2}$ "  $\phi$  HK B II BOLTS

$$\text{BOLT TON} = \frac{1514 \times 3.5 + 56}{7} = 765 < \frac{10500}{4} = 2625 \text{ \# (ALLOW)}$$

CHECK  $\angle 6 \times 4 \times \frac{1}{4}$

$$\text{STRESS} = \frac{1514 \times (2.5 \cdot 1.09)}{10 \times 0.25^2 / 6} = 20,494 \text{ PSI} < 27,000 \text{ OK}$$



ATT B 7  
ATT. D 1

DECO SPEC  
# 3071-226  
REV J

**Raytheon**  
 Engineers & Constructors

 GENERAL  
 COMPUTATION  
 SHEET

DECO FERMI 2

PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

REV

COMP BY

CHK'D BY

PRELIM

FINAL

VOID

0

DATE

5-8-96

DATE

5/23/96

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DATE

DATE

JO 76 230502

GM-2850-H-74

BASED ON MODIFIED STRING ANALYSIS OUTPUT

BASE PLATE LOADS:

$$F_x = 857 \times \left( \frac{382}{823} \right) = 398 \#$$

$$F_y = 271 \times \left( \frac{353}{574} \right) + 158 \times \left( \frac{73}{314} \right) + 3957 \times \left( \frac{382}{823} \right)$$

$$= 2051 \#$$

$$M_z = 8807 \times \left( \frac{382}{823} \right) = 4088 \text{ " \#} = 341' \#$$

ONE  $\frac{5}{8}$ "  $\phi$  WEDGE ANCHOR IS USED AT THE NEAREST SUPP.MEMBER FOR EACH BASE PLATE. CONSERVATIVELY USE CHART C,  
ZONE I

$$I R = \frac{2051}{3150} + \frac{341}{1023} = 0.984$$

SHEAR IS NEGLIGIBLE

BOLT SPACING VIOLATIONS WERE DOCUMENTED IN  
DCN 10938A.MIN SPACING BETWEEN  $\frac{5}{8}$ "  $\phi$  W.A. &  $\frac{3}{8}$ "  $\phi$  BOLT  
IN THE EXISTING CONDUIT SUPT IS  $3\frac{1}{2}$ ".

$$\text{REQ'D SPACING} = \frac{4.375}{2} + \frac{4}{2} = 4.19 \text{ "}$$

$$\text{REDUCED } \frac{5}{8} \text{ " } \phi \text{ W.B. TEN} = 3575 \times \left( \frac{3.5}{4.19} \right) = 2986 > 2051 \# \#$$

$$\text{PLATE STRESS} = \frac{2051 \times (2-1.125)}{9 \times \frac{0.625^2}{6}} = 3063 \# < 27,000 \#$$

ALL BASE PLATE TENSION IS TAKEN BY ONE W.A.

SINCE PLATE STRESS IS LOW, ONE OPEN HOLE IN PLATE B IS ACCEPTABLE.

REF

STRING  
ANALYSIS  
OUTPUT  
4/25/96DCN #  
10938A

SHZ

ABM-0507

REV A

SH. 11



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DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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PRELIM	FINAL	VOID	0	RFI	KTW
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JO. 76230 502				DATE	DATE
				5/21/96	5/23/96

GM-2850-H-74 (MODIFIED)

REF

EVALUATION OF NEW ADDED BASE PLATE  
BASED ON FIELD MODIFICATION.

ATT. D 1

BASE PLATE :  $10 \times 10 \times \frac{3}{4}$

A.B. : 4 -  $\frac{1}{2}$ "  $\phi$  HKB II HILTI BOLTS

FROM MODIFIED STRING ANALYSIS OUTPUT 5/15/96  
LATERAL SUPPORT LOAD DUE TO DL + SSE IS 391 #.  
THE CAPACITY OF THIS BASE PLATE IS MUCH  
HIGHER THAN 391 #, THEREFORE THIS BASE PLATE  
IS ADEQUATE.

ATT. A  
STRING  
ANALYSIS  
OUTPUT  
5/17/96

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DECO FERMI 2 SHEET

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PROJECT STRUCTURAL QUALIFICATIONSUPPORT EVALUATION FORSUBJECT SYSTEM 2850-1

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BASE PLATE CHECK FOR SUPPORT 6M-2850-H-75BASE PLATE LOADS FOR SUPPORT 6M-2850-H-77-B  
ARE UTILIZED AS BENCHMARK LOADS.

JOINT 1.0R12

DEAD LOAD (LC #1) 540 lb

$$F_x = 0$$

$$F_y = 271 \text{ lb}$$

$$M_z = 25 \text{ W-lb}$$

SSE VERTICAL (LC #5) 314 lb

$$F_x = 0$$

$$F_y = 158 \text{ lb}$$

$$M_z = 15 \text{ W-lb}$$

SSE HORIZONTAL (LC #6) 833 lb

$$F_x = 857 \text{ lb}$$

$$F_y = 3957 \text{ lb}$$

$$M_z = 8807 \text{ W-lb}$$

STARDYNE  
OUTPUT FOR  
H-77  
4/22/96  
(ATT. B2)

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DECO FERMI 2 SHEET

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATIONSUBJECT SYSTEM 2850-1

CALCULATION SET NO

DC-5766

REV

COMP. BY

CHK'D BY

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 ETW  
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JO. 76230502

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REF

 BASE PLATE CHECK FOR SUPPORT 6M-2850-H-75  
 (CONT'D)

RATIOS OF LOADS FOR SUPPORTS H-75 &amp; H-77

$$\text{DEAD LOAD} = \frac{242}{540} = 0.448$$

$$\text{SSE VERTICAL} = \frac{60^*}{314} = 0.191$$

$$\text{SSE HORIZONTAL} = \frac{567^*}{823} = 0.689$$

\* USE OBE LOADS SINCE OBE &gt; SSE

 PROPORTIONED BASE PLATE LOADS FOR SUPPORT H-75  
 ARE:

$$F_x = 857 \times 0.689 = 590 \text{ lb}$$

$$F_y = 271 \times 0.448 + 158 \times 0.191 + 3957 \times 0.689$$

$$= 2878 \text{ lb}$$

$$M_z = 25 \times 0.448 + 15 \times 0.191 + 8807 \times 0.689$$

$$= 6082 \text{ IN-LB} = 506 \text{ FT-LB}$$

 BASED ON THE AS-BUILT SKETCH  
 THE BASE PLATES FAIL BY USING BASE PLATE  
 CHART C, ZONE I.

 STRING  
 ANALYSIS  
 OUTPUT  
 4/4/96

 DC 2833  
 66M-0507

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 COMPUTATION  
 SHEET

DECO FERMI 2

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

REV

COMP. BY

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KTL  
DATE

5/20/96

JFS  
DATE

5-21-96

PRELIM

FINAL

VOID

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JO 76230.502

DATE

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 BASE PLATE CHECK FOR SUPPORT 6M-2850-H-75 (CONT'D)

BASED ON MODIFIED STRING ANALYSIS OUTPUT 5/17/96 (ATT.A)

RATIO OF LOADS FOR SUPPORTS H-75 &amp; H-77 ARE:

$$\text{DEAD LOAD} = \frac{242}{540} = 0.448$$

$$\text{SSE VERTICAL} = \frac{51}{314} = 0.162$$

$$\text{SSE HORIZONTAL} = \frac{233}{823} = 0.283$$

PROPORTIONED BASE PLATE LOADS FOR SUPPORT H-75 ARE:

$$F_x = 857 \times 0.283 = 243 \text{ lb}$$

$$F_y = 271 \times 0.448 + 158 \times 0.162 + 3957 \times 0.283 + 15$$

$$= 1282 \text{ lb}$$

$$M/2 = 25 \times 0.448 + 15 \times 0.162 + 8807 \times 0.283$$

$$= 2506 \text{ IN-LB} = 209 \text{ FT-LB}$$

 BASED ON THE AS BUILT SKETCH IN ADM-0507 (DC-2833),  
 THE BASE PLATE CHART C, ZONE I IS USED FOR QUALIFICATION

$$IR = \frac{1282}{3150} + \frac{209}{1023} = 0.61 < 1.0 \quad \text{O.K.}$$

THE BASE PLATE &amp; ANCH. BOLTS ARE ADEQUATE.

SHEAR IS NEGLIGIBLE

<b>Raytheon</b> Engineers & Constructors GENERAL COMPUTATION DECO FERMI 2 SHEET CCHVAC DUCT & DUCT SUPPORTS			CALCULATION SET NO.	REV	COMP BY	CHK'D BY
			DC-5766	0	KTW	OFS
PRELIM	FINAL	VOID	DATE		DATE	
	✓		5/20/96		5-21-96	
PROJECT STRUCTURAL QUALIFICATION			SHEET 164 OF 246		DATE	DATE
SUBJECT SUPPORT EVALUATION FOR			.076230.502			
SYSTEM 2850-1						

BASE PLATE CHECK FOR Support 6M-2850-H-76

BASED ON MODIFIED STRING ANALYSIS OUTPUT 5/17/96 (ATT. A)

THE SUPPORT LOADS ARE:

	DL	SSE
VERTICAL	236	47
LATERAL	0	409

ASSUME THAT THE VERTICAL LOADS ARE TAKEN BY THE TWO VERTICAL MEMBERS AND LATERAL LOADS ARE TAKEN BY THE ADDED BRACING. THEREFORE.

BASE PLATE LOADS FOR PLATE A & B ARE

$$\frac{236 + 47}{2} = 142 \text{ lb (TENSION)}$$

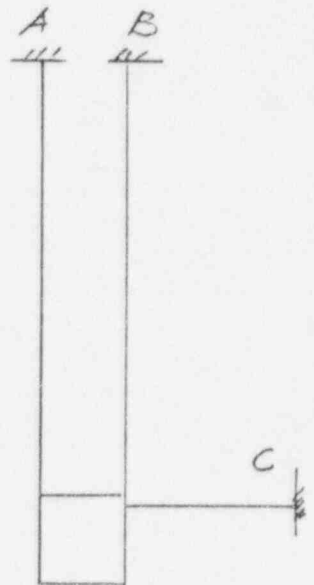
$$\text{MOMENT} = 0$$

PLATES A & B ARE QUALIFIED BY USING BASE PLATE CHART C, ZONE IV.  $IR = \frac{142}{2110} = 0.07 < 1.0$

BASE PLATE LOADS FOR PLATE C ARE

$$\text{TENSION} = 409 \text{ lb}$$

$$\text{MOMENT} = 0$$



MODIFIED SUPPORT

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GENERAL  
COMPUTATION  
SHEET

DECO FERMI 2

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-1

CALCULATION SET NO

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REV

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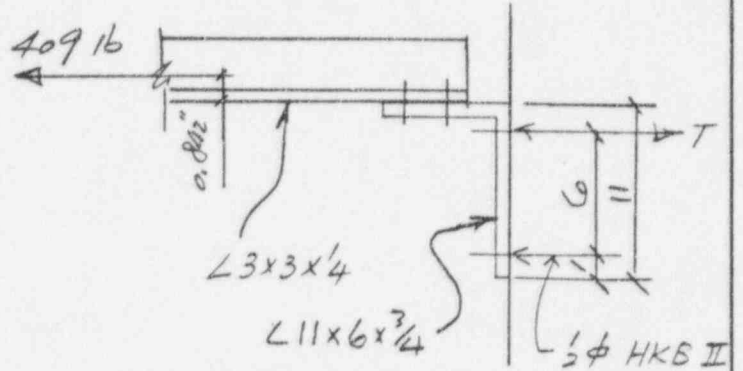
JO 76230.502

DATE

DATE

H76 (CONT'D)

PLATE C.



$$\text{BOLT TENSION} = \frac{409 \times 10.842}{6} = 739 \text{ lb} < 2625 \text{ lb} \quad \text{O.K.}$$

$$\text{PLATE STRESS} = \frac{409 \times 4.842}{6 \times \frac{0.75^2}{6}} = 3521 \text{ lb/in}^2 < 27000 \text{ lb/in}^2 \quad \text{O.K.}$$

$$\text{IR FOR BOLT} = \frac{739}{2625} = 0.28 < 1.0 \quad \text{OK}$$

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REF

BASE PLATE CHECK FOR SUPPORT 6M-2850-H-77

BASE PLATE LOADS BASED ON MODIFIED SUPPORT ANALYSIS

(ATT. B 8)

JT. 1 (L.C. #3)

$$F_x = 115 \text{ lb}$$

$$F_y = 2822 \text{ lb}$$

$$M_z = 890 \text{ in-lb} = 74 \text{ FT-lb.}$$

USE BASE PLATE CHART C, ZONE I

$$\frac{2822}{3150} + \frac{74}{1023} = 0.97 < 1.0 \text{ O.K.}$$

JT. 14 (L.C. #4)

$$F_x = 14 \text{ lb}$$

$$F_y = 3679 \text{ lb}$$

$$M_z = 23 \text{ in-lb} = 2 \text{ FT-lb.}$$

BASED ON AS BUILT SKETCH, AVERAGE VALUE OF BASE PLATE CHART B &amp; C, ZONE I ARE USED.

ABM-0507  
(DC-2833)

$$\frac{3679}{(5560+3150)/2} + \frac{2}{(1541+1023)/2} = 0.84 < 1.0 \text{ O.K.}$$

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	✓			DATE 5/20/96	DATE 5-21-96
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JO 76230502					

REF.

H-77 (CONT'D)

JT 15. (L.C. #3)

$$F_x = 939 \text{ lb}$$

$$F_y = 859 \text{ lb}$$

$$M_2 = 368 \text{ in-lb} = 31 \text{ FT-lb}$$

(ATT. B 8)

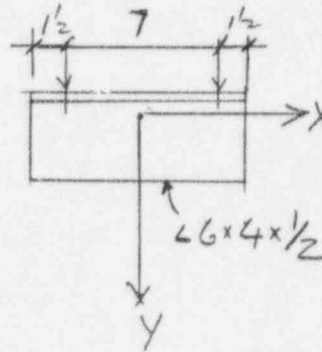
BOLT TENSION:

$$\frac{859 \times 3.5 + 368}{7} = 482 \text{ lb} < 2625 \text{ lb} \text{ o.k.}$$

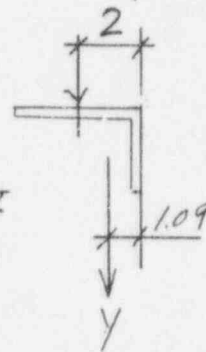
$$IR = \frac{482}{2625} = 0.2$$

PLATE STRESS:

$$\frac{859 \times 0.91}{10 \times \frac{0.5^2}{6}} = 1876 \text{ PSI} < 27,000 \text{ PSI} \text{ o.k.}$$



ATT. D 1





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 SHEET

DECO FERMI 2

PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

SUBJECT STRUCTURAL QUALIFICATION  
SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

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DATE

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6M-2850-H-78

BASED ON MODIFIED STRING RUN 4/25/96

SUPPORT LOADS:

DL + SSIE

$$\text{VERT (MAX)} = 357 + 125 = 482 \#$$

$$\text{VERT (MIN)} = 357 - 125 = 232 \#$$

$$\text{LAT} = 7 + 303 = 710 \#$$

$$\text{TEN @ JT. 8} = \frac{482 \times 67.5}{\left(67 + \frac{10}{2}\right)} = 452 \#$$

BASE PL.: CHART 'C' ZONE II

$$\text{IR} = \frac{452}{2110} = 0.21 < 1.0 \text{ OK}$$

SHEAR &amp; MOMENT ARE NEGLIGIBLE

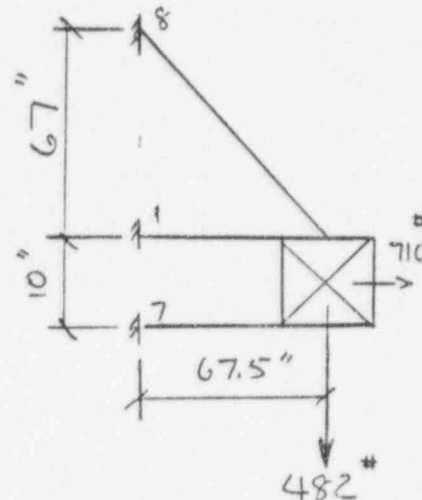
JOINT 1 &amp; 7:

$$\text{TENSION} = \frac{710 \times 72 - 232 \times 67.5}{72 \times 2} = 246 \#$$

CHART 'A', ZONE III

$$\text{IR} = \frac{246}{1400} = 0.18 < 1.0 \text{ OK}$$

SHEAR &amp; MOMENT ARE NEGLIGIBLE



REF

STRING  
ANALYSIS  
OUTPUT  
4/25/96ABM-0507  
SH 15

<b>Raytheon</b> Engineers & Constructors GENERAL COMPUTATION DECO FERMI 2 SHEET CCHVAC DUCT & DUCT SUPPORTS PROJECT <u>STRUCTURAL QUALIFICATION</u> SUBJECT <u>SUPPORT EVALUATION FOR SYSTEM 2850-1</u>	CALCULATION SET NO. DC-5766			REV	COMP BY	CHK'D BY
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BASE PLATE CHECK FOR SUPPORT 6M-2850-79-B.

RATIOS OF LOADS FOR SUPPORTS H-79 & H-77

$$\text{DEAD LOAD} = \frac{498}{540} = 0.922$$

$$\text{SSE VERTICAL} = \frac{161}{314} = 0.512$$

$$\text{SSE HORIZONTAL} = \frac{457}{823} = 0.555$$

PROPORTIONED BASE PLATE LOADS FOR SUPPORT H-79 ARE :

$$F_x = 857 \times 0.555 = 476 \text{ lb}$$

$$F_y = 271 \times 0.922 + 158 \times 0.512 + 3957 \times 0.555 = 2560 \text{ lb}$$

$$M_z = 25 \times 0.922 + 15 \times 0.512 + 8807 \times 0.555 = 4919 \text{ IN-lb} = 410 \text{ FT-lb}$$

REF

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ANALYSIS  
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4/4/96

**Raytheon**  
Engineers & Constructors

GENERAL  
COMPUTATION

DECO FERMI 2 SHEET

CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

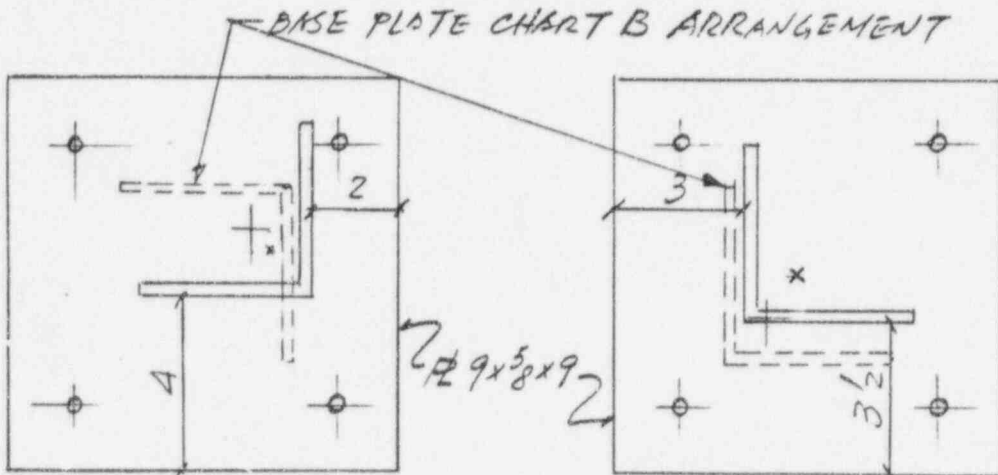
SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-1

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BASE PLATE CHECK FOR SUPPORT 6M-2850-H-79-B  
(CONT'D)



DC 2833  
ABM-0507

ALL ATTACHED MEMBERS = L 4x4x1/2

PLATE B

PLATE A

BY OBSERVATION, PLATE B IS MORE CRITICAL.

USING BASE PLATE CHART B, ZONE I.

$$IR = \frac{2560}{5560} + \frac{410}{1541} = 0.726 < 1.0$$

THE BASE PLATES & ANCHORS ARE ADEQUATE

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PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

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DATE

DATE

GM-2850-H-80

THIS SUPPORT IS SIMILAR WITH GM-2850-H-75

RATIO OF LOADS FOR H-80  $\frac{1}{2}$  H-75

MODIFIED STRING ANALYSIS OUTPUT IS UTILIZED

$$D.L. = \frac{190}{242} = 0.785$$

$$SSE VERT = \frac{51}{51} = 1.0$$

$$SSE HORIZ = \frac{10}{380} = 0.026$$

$$F_x = 435 \times 0.785 = 342 \#$$

$$F_y = 271 \times 0.448 \times 0.785 + 158 \times 0.191 \times 1.0 + 3957 \times 0.508 \times 0.026 + 15 = 193 \#$$

$$M_z = 25 \times 0.448 \times 0.785 + 15 \times 0.191 \times 1.0 + 8807 \times 0.508 \times 0.026 = 128'' \# = 11' \#$$

CHART C, ZONE IV IS UTILIZED

$$IR = \frac{193}{2110} + \frac{11}{420} = 0.12 < 1.0 \quad OK$$

SHEAR IS NEGLIGIBLE

REF

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ANALYSIS  
OUTPUT  
4/25/96

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 DECO FERMI 2

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GM-2850-H-81

BASED ON MODIFIED STRING ANALYSIS OUTPUT

RATIO OF MOD OUTPUT &amp; ORIGINAL OUTPUT

$$D.L = 1$$

$$\frac{SSE(MOD.) HORIZ}{OBE(ORIG.) HORIZ} = \frac{368}{1476} = 0.249$$

$$\frac{SSE(MOD.) VERT}{OBE(ORIG.) VERT} = \frac{82}{108} = 0.759$$

DL + SSE : L.C. 3

$$F_z = 1129 \times 0.249 = 281 \#$$

$$F_y = (146 + 96) + 44 \times 0.759 + 4387 \times 0.249 = 1368 \#$$

$$M_x = 1992 \times 0.249 = 496 \# = 41' \#$$

CHART C, ZONE II IS UTILIZED

$$IR = \frac{1368}{3150} + \frac{41}{920} = 0.48 < 1.0 \quad OK$$

SHEAR IS NEGLIGIBLE

REF

 STRING  
 ANALYSIS  
 OUTPUTS  
 4/25/96  
 (MOD.) &  
 4/4/96  
 (ORIGINAL)

 STARDYNE  
 H-81 RUN  
 4/17/96  
 (ATT. B3)

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COMPUTATION  
SHEET

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

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GM-2850-H-81 : (MODIFIED)

REF

PER THE MODIFICATION SCHEME, A BRACING IS ADDED TO THIS SUPPORT. THE IMPACT TO THE BASE PLATES IS INSIGNIFICANT. ALSO THE BASE PLATE IR OF 0.48 IS LOW, THEREFORE THESE BASE PLATES ARE ADEQUATE.

ATT D1

<b>Raytheon</b> <b>Engineers &amp; Constructors</b> GENERAL COMPUTATION DECO FERMI 2 SHEET CCHMAC DUCT & DUCT SUPPORTS PROJECT STRUCTURAL QUALIFICATION SUBJECT <u>SUPPORT EVALUATION FOR</u> <u>SYSTEM 2850-1</u>	CALCULATION SET NO. DC-5766			REV	COMP. BY	CHK'D BY
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	JO. 76 230 902					

REF

BASE PLATE CHECK FOR Support 6M-2850-H-82

RATIOS OF LOADS FOR Supports H-82 & H-77

$$\text{DEAD LOAD} = \frac{160}{540} = 0.296$$

$$\text{SSE VERTICAL} = \frac{93}{314} = 0.296$$

$$\text{SSE HORIZONTAL} = \frac{381}{823} = 0.463$$

PROPORTIONED BASE PLATE LOADS FOR Support H-82 ARE:

$$F_x = 857 \times 0.463 = 397 \text{ lb}$$

$$F_y = 271 \times 0.296 + 158 \times 0.296 + 3957 \times 0.463 = 1959 \text{ lb}$$

$$M_z = 25 \times 0.296 + 15 \times 0.296 + 8807 \times 0.463 = 4089 \text{ in-lb} = 341 \text{ Ft-lb}$$

BASED ON THE ATTACHMENT LOCATION SHOWN ON AS BUILT SKETCH, THE BASE PLATES ARE QUALIFIED BY USING BASE PLATE CHART C, ZONE I.

STRING  
ANALYSIS  
OUTPUT  
4/25/96  
(MOD)

DC 2833  
ABM-0507

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COMPUTATION  
SHEET

DECO FERMI 2

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

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CHK'D BY

PRELIM

FINAL

VOID

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KTW

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DATE

DATE

4/27/96

5-8-96

SHEET

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JO. 76 230 502

DATE

DATE

REF

BASE PLATE CHECK FOR SUPPORT 6M-2850-H-83

BASE PLATE LOADS FOR SUPPORT 6M-2850-H-59-A

JOINT # 1

DEAD LOAD

$$F_x = 0$$

$$F_y = 164 \text{ lb}$$

$$M_z = 0$$

OBE VERTICAL

$$F_x = 0$$

$$F_y = 78 \text{ lb}$$

$$M_z = 0$$

OBE HORIZONTAL

$$F_x = 140 \text{ lb}$$

$$F_y = 874 \text{ lb}$$

$$M_z = 3985 \text{ IN-LB}$$

SYSTEM  
2850-2



**Raytheon**  
 Engineers & Constructors

 GENERAL  
 COMPUTATION

DECO FERMI 2 SHEET

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATIONSUPPORT EVALUATION FORSUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DL-5766

REV

COMP. BY

CHK'D BY

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FINAL

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DATE

DATE

4/27/96

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DATE

DATE

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REF

BASE PLATE CHECK FOR SUPPORT GM-2850-H-83  
 (CONT'D)

RATIOS OF LOADS FOR SUPPORTS H-83 & H-59

$$\text{DEAD LOAD} = \frac{377}{240} = 1.570$$

$$* \text{ OBE VERTICAL} = \frac{240}{156} = 1.564$$

$$* \text{ OBE HORIZONTAL} = \frac{31}{115} = 0.270$$

PROPORTIONED BASE PLATE LOADS FOR SUPPORT H-83  
 ARE:

$$F_x = 140 \times 0.270 = 38 \text{ lb}$$

$$F_y = 164 \times 1.570 + 78 \times 1.564 + 874 \times 0.270 \\ = 615 \text{ lb}$$

$$M_z = 3980 \times 0.270 = 1074.6 \text{ in-lb} = 90 \text{ ft-lb}$$

BASED ON THE AS BUILT SKETCH OF THE BASE  
 PLATES, THE BASE PLATES ARE QUALIFIED BY  
 USING BASE PLATE CHART C, ZONE II.

\* OBE > SSE

STRING  
 ANALYSIS  
 OUTPUT  
 4/25/96

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COMPUTATION  
SHEET

DECO FERMI 2

PROJECT CCHVAC DUCT &amp; DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-1

CALCULATION SET NO

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5/8/96

DATE

5-22-96

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JO 76230502

DATE

DATE

R:F

BASE PLATE CHECK FOR SUPPORT GM-2850-H-87-B

LOADS AT BASE PLATE

JOINT 1 OR 7

DEAD LOAD

$$F_x = 0$$

$$F_y = 20 \text{ lb}$$

$$F_z = 0$$

$$M_x = 1299 \text{ in-lb}$$

$$M_y = 0$$

$$M_z = 0$$

OBE VERTICAL

$$F_x = 0$$

$$F_y = 64 \text{ lb}$$

$$F_z = 0$$

$$M_x = 4166 \text{ in-lb}$$

$$M_y = 0$$

$$M_z = 0$$

OBE HORIZONTAL (X)

$$F_x = 182 \text{ lb}$$

$$F_y = 0$$

$$F_z = 741 \text{ lb}$$

$$M_x = 0$$

$$M_y = 5584 \text{ in-lb}$$

$$M_z = 0$$

HORIZONTAL (Z)

$$F_x = 0$$

$$F_y = 0$$

$$F_z = 54 \text{ lb}$$

$$M_x = 0$$

$$M_y = 0$$

$$M_z = 0$$

DEAD LOAD OF MEMBER

$$F_y = 56 \text{ lb}$$

$$F_x = F_z = 0$$

$$M_x = 2552 \text{ in-lb} \quad M_y = M_z = 0$$

ATT. B 4  
STADYNE  
OUTPUT FOR  
H-87  
4/22/96

<b>Raytheon</b> Engineers & Constructors DECO FERMI 2 SHEET CCHVAC DUCT & DUCT SUPPORTS PROJECT STRUCTURAL QUALIFICATION SUPPORT EVALUATION FOR SUBJECT SYSTEM 2850-1	CALCULATION SET NO. DC-5766			REV.	COMP. BY	CHK'D BY
	PRELIM.	FINAL	VOID	0	KTW	JFS
		✓			DATE 4/27/96	DATE 5-22-96
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JO 76230502						

BASE PLATE CHECK FOR SUPPORT 6M-2850-H-87-B  
(CONT'D)

BASED ON MODIFIED STRING ANALYSIS OUTPUT:  
RATIOS OF SSE LOADS TO OBE LOADS

$$\text{HORIZONTAL } X = \frac{299}{345} = 0.867$$

$$\text{HORIZONTAL } Z = \frac{63}{73} = 0.863$$

PROPORTIONED BASE PLATE LOADS FOR D+SSE  
ARE:

$$F_x = 182 \times 0.867 = 158 \text{ lb}$$

$F_y$

$$F_z = 741 \times 0.867 + 54 \times 0.863 = 689 \text{ lb}$$

$$M_x = 2552 = 212 \text{ FT-LB}$$

$$M_y = 5584 \times 0.867 = 4841 \text{ IN-LB} = 403 \text{ FT-LB}$$

$$M_z = 0$$

REF

STRING  
ANALYSIS  
OUTPUT  
4/25/96

**Raytheon**  
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 GENERAL  
 COMPUTATION

DECO FERMI 2 SHEET

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATIONSUPPORT EVALUATION FORSUBJECT SYSTEM 2850-1

CALCULATION SET NO.

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DATE

5/23/96

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DATE

5/23/96

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JO 76230502

DATE

DATE

REF

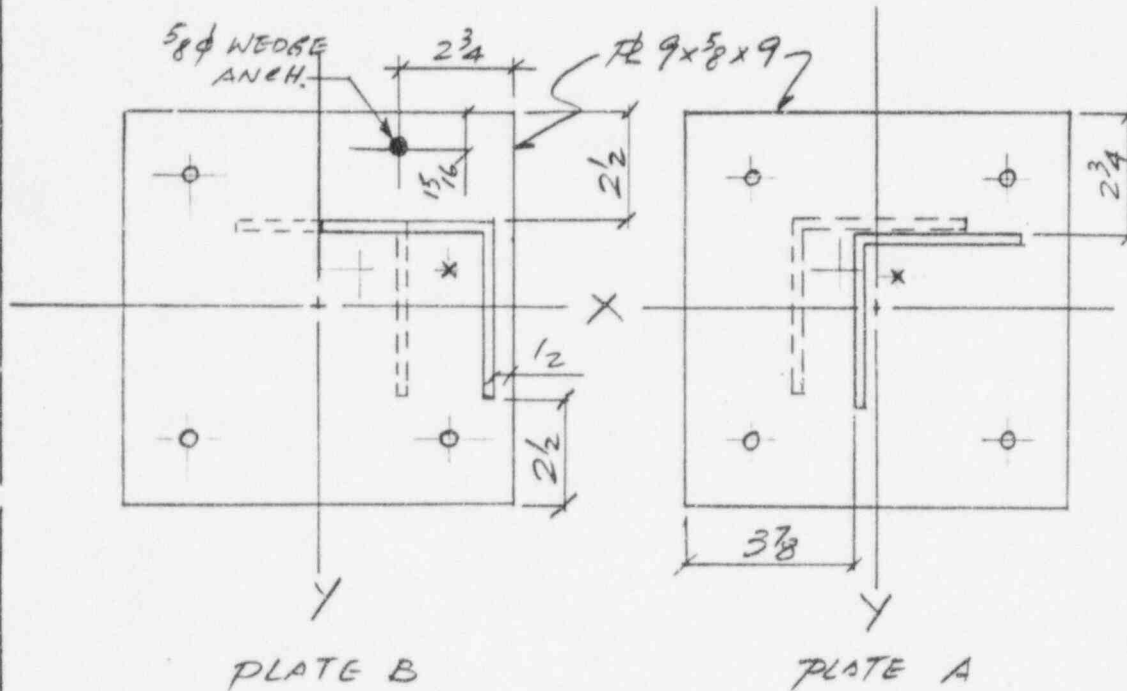
 BASE PLATE CHECK FOR SUPPORT EM-2850-H-87-B  
 (CONT'D)

 DC 2833  
 DCN 10939A  
 E  
 ABM-0507

 PLATE A IS QUALIFIED BY USING BASE PLATE  
 CHART B, ZONE I.

FOR PLATE B, CHART A, ZONE I IS UTILIZED

$$IR = \frac{689}{2090} + \frac{403}{673} = 0.93 < 1.0 \text{ O.K.}$$

SHEAR IS NEGLIGIBLE

$$\text{PLATE STRESS} = \frac{3575 \times (2.5 - 0.9375)}{9 \times \frac{0.825^2}{6}} = 9533 \text{ PSI} < 27,000 \text{ PSI}$$

 SINCE PLATE STRESS IS LOW, AN OPEN HOLE IN  
 BASE PLATE 'B' DOES NOT ALTER THE PLATE  
 STRUCTURAL ADEQUACY, THEREFORE, IT IS ACCEPTABLE.

<b>Raytheon</b> Engineers & Constructors GENERAL COMPUTATION DECO FERMI 2 SHEET CCHVAC DUCT & DUCT SUPPORTS PROJECT STRUCTURAL QUALIFICATION SUBJECT <u>SUPPORT EVALUATION FOR SYSTEM 2850-1</u>	CALCULATION SET NO. DC-5766			REV	COMP. BY	CHK'D BY
	PRELIM	FINAL	VOID	0	ETW. DATE 4/27/96	GRS DATE 5-8-96
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	JO 76230502					

REF

BASE PLATE CHECK FOR SUPPORT 6M-2850-H-86-B

RATIOS OF LOADS FOR SUPPORTS H-86 & H-87

$$\text{DEAD LOAD} = \frac{64}{29} = 2.207$$

$$\text{SSE VERTICAL} = \frac{63}{93} = 0.677$$

$$\text{SSE HORIZONTAL (X)} = \frac{140}{345} = 0.406$$

$$\text{SSE HORIZONTAL (Z)} = \frac{193}{73} = 2.644$$

PROPORTIONED BASE PLATE LOADS FOR D+SSE ARE:

$$F_x = 182 \times 0.406 = 74 \text{ lb}$$

$$F_y = 20 \times 2.207 + 64 \times 0.677 + 56 = 165 \text{ lb}$$

$$F_z = 741 \times 0.406 + 54 \times 2.644 = 444 \text{ lb}$$

$$M_x = 1299 \times 2.207 + 4166 \times 0.677 + 2552 \\ = 8239 \text{ IN-LB} = 686 \text{ FT-LB}$$

$$M_y = 5584 \times 0.406 = 2267 \text{ IN-LB} = 189 \text{ FT-LB}$$

$$M_z = 0$$

STRING  
ANALYSIS  
OUTPUT  
4/4/96

**Raytheon**  
Engineers & Constructors

GENERAL  
COMPUTATION  
SHEET

D/ J FERMI 2

PROJECT CCHVAC DUCT & DUCT SUPPORTS

~~SUPPLEMENTAL QUALIFICATION~~

SUBJECT SYSTEM 2850-1

CALCULATION SET NO.

DC-5766

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4/27/96

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BASE PLATE CHECK FOR SUPPORT 6M-2850-H-86-B  
(CONT'D)

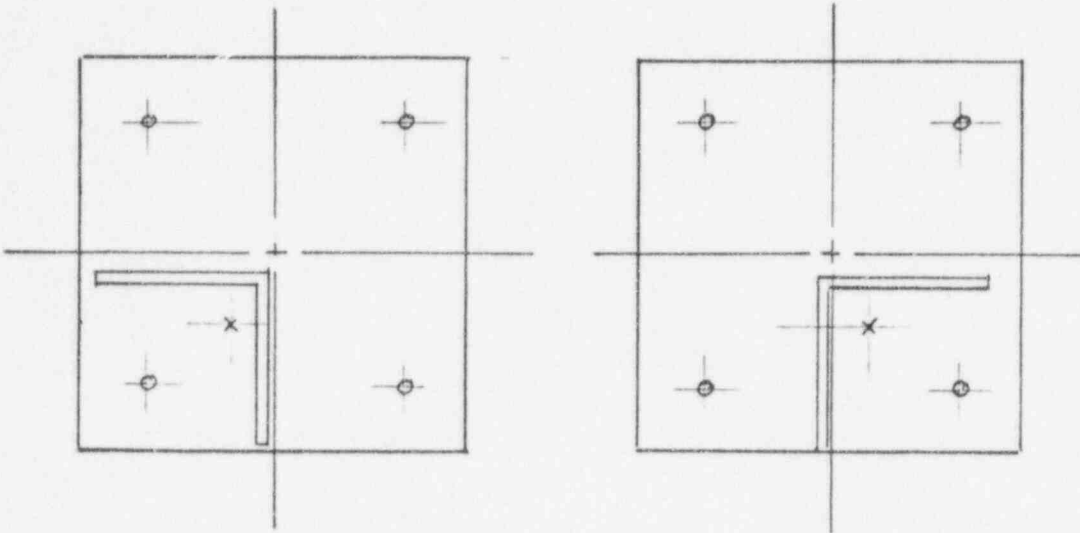


PLATE B

PLATE A

DC 2835  
ASM-0507

BASED ON THE AS BUILT SKETCH FOR PLATES A & B,  
THE BASE PLATES ARE QUALIFIED BY USING BASE PLATE  
CHART C, ZONE I.

**Raytheon**  
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GENERAL  
COMPUTATION

DECO FERMI 2 SHEET

CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUBJECT SUPPORT EVALUATION FOR  
SYSTEM 2850-2

CALCULATION SET NO

DC-5766

REV

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DATE  
5/21/96

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JO 76230.502

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DATE

SUPPORT MARK	JOINT NO.	BASE PL. SIZE	ANCHOR BOLT PHILIPS ANCH UON	SUPPORT MEMBER	* BASE PLATE LOADS			RESULT	REMARKS
					TENSION (LBS)	MOMENT (FT-LBS)	SHEAR (LBS)		
GM-2850 H-49	1	8x8x5/8	4-1/2" φ	L3x3x1/4	1511	991	755	D+SSE	SHARED WITH SYS. 2850-4
	9	↓	4-3/8" φ	L2 1/2 x 2 1/2 x 1/4	969	491	51	↓	
	13	↓	4-1/2" φ	L3x3x1/4	1670	16	26	↓	
GM-2850 H-54	1	8x8x1/2	4-3/8" φ	L2 1/2 x 2 1/2 x 1/4	2054	68	23	D+SSE	BOUNDED BY H-54
	7	↓			647	32	8	↓	
	8	↓			1514	14	865	↓	
GM-2850 H-56				↓					BOUNDED BY H 54
GM-2850 H-58	1	8x8x5/8	4-1/2" φ	L3x3x1/4	1395	272	129	D+SSE	PASS
	10	"	"	"	1395	65	22	"	

\* SEE STRING ANALYSIS OUTPUT 3/29/96

**Ravtheon**  
Engineers & Constructors

DECO FERM 12

GENERAL  
COMPUTATION  
SHEET

CC/IVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUBJECT SUPPORT SYSTEM 2850-2  
EVALUATION FOR

CALCULATION SET NO

DC-5766

PRELIM

FINAL

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CHK'D BY

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5/23/96

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DATE

DATE

SUPPORT MARK	JOINT NO.	BASE PL. SIZE	ANCHOR BOLT PHILLIPS ANCHOR UN	SUPPORT MEMBER	BASE PLATE LOADS				RESULT	REMARKS
					TENSION (LBS)	MOMENT (FT-LBS)	SHEAR (LBS)	GOVERNING LOAD CASE		
GM-2850 H-59	1	L4x4x1/4 x 1'-0" LG	4-3/8" φ	L2x2x1/4	1116	M <sub>x</sub> =287 M <sub>z</sub> =53	140	D+OBE	PASS	
	10	6x6x3/8	4-3/8" φ		1116	81	25	"		
GM-2850 H-60	1								PASS	MODIFIED SUPPORT DCN10891A
	10									
GM-2850 H-61	1		3-3/8" φ 1-1/2" φ W.B.						PASS	MODIFIED SUPPORT DCN10890B
	10		4-3/8" φ							
GM-2850 H-62	1				1051	335	141	D+SSSE	PASS	
	10				1051	81	141			
GM-2850 H-63	1								PASS	MODIFIED SUPPORT DCN10893
	10									
GM-2850 H-64	1								PASS	MODIFIED SUPPORT DCN10895 DCN10883C
	10									
GM-2850 H-65	1	8x8x1/2	4-3/8" φ	L2 1/2 x 2 1/2 x 1/4					PASS	BOUNDED BY H-59
	10	"	"	"						



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GENERAL  
COMPUTATION  
SHEET

DECO FERM 2  
CCHVAC DUCT & DUCT SUPPORTS

CALCULATION SET NO

DC-5766

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PROJECT STRUCTURAL QUALIFICATION

SUBJECT

SUPPORT EVALUATION FOR  
SYSTEM 2850-2

SHEET 184 OF 246

JO 76 230.502

DATE

DATE

SUPPORT MARK	JOINT NO.	BASE PL. SIZE	ANCHOR BOLT PHILLIPS ANCHORS	SUPPORT MEMBER	BASE PLATE LOADS				RESULT	REMARKS
					TENSION (LBS)	MOMENT (FT-LBS)	SHEAR (LBS)	GOVERNING LOAD CASE		
GM-2850 H-66	1	8x8x1/2	4-3/8"	L2 1/2 x 2 1/2 x 1/4	1442	468	197	D+SSE	PASS	BOUNDED BY H-59
	10									
GM-2850 H-67	1	6x6x3/8	4-3/8"	L2 1/2 x 2 1/4	1442	114	197	D+SSE	PASS	BOUNDED BY H-59
	10									
GM-2850 H-68	1	8x8x1/2	4-3/8"	L2 1/2 x 2 1/4	1442	468	197	D+SSE	PASS	BOUNDED BY H-59
	10									
GM-2850 H-69	1	6x6x3/8	4-3/8"	L2 1/2 x 2 1/4	1442	114	197	D+SSE	PASS	BOUNDED BY H-59
	10									
GM-2850 H-70	1	8x8x1/2	4-3/8"	L2 1/2 x 2 1/4	1442	468	197	D+SSE	PASS	BOUNDED BY H-59
	10									
GM-2850 H-71	1	6x6x3/8	4-3/8"	L2 1/2 x 2 1/4	1442	114	197	D+SSE	PASS	BOUNDED BY H-68
	10									
GM-2850 H-72	1	8x8x1/2	4-3/8"	L2 1/2 x 2 1/4	1442	468	197	D+SSE	PASS	SEE ATT. C1
	10									

**Raytheon**  
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COMPUTATION  
SHEET

DECO FERMI 2

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-2

CALCULATION SET NO

DC-5766

PRELIM

FINAL

VOID

✓

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DATE  
5/21/96

DATE

DATE

GM-2850-H-49JOINT 1

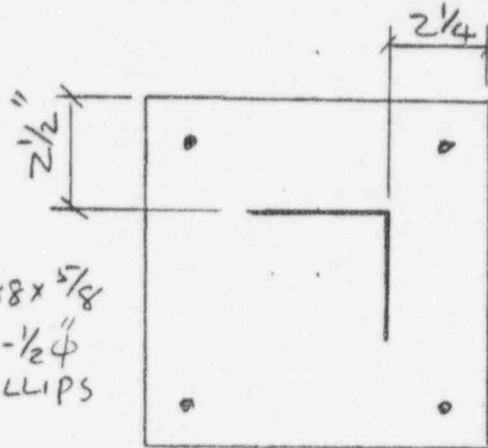
DL+SSE

$$F_x = 755 \#$$

$$F_y = 1511 \#$$

$$M_z = 991 \#$$

B.P.: 8x8x 5/8

A.B.: 4-1/2"  $\phi$   
PHILLIPS

REF

DC 2836  
ATT. 5STAR DYNE  
OUTPUT  
4/5/96  
ATT. B 9

FROM TEN-MOMENT CAPACITY CHART 'B', ZONE II

$$\text{BOLT TEN IR} = \frac{1511}{5560} + \frac{991}{1388} = 0.986$$

$$\text{SHEAR IR} = \frac{755}{4 \times 1370} = 0.138$$

$$\text{IR} = (0.986)^{5/3} + (0.138)^{5/3} \approx 1.0 \quad \text{OK}$$

PLATE STRESS IS WITHIN THE TEN-MOMENT  
CAPACITY CHART IS OK

**Raytheon**  
Engineers & Constructors

GENERAL  
COMPUTATION  
SHEET

DECO FERMI 2

CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-2

CALCULATION SET NO. DC-5766			REV	COMP. BY	CHK'D BY
PRELIM	FINAL	VOID	0	GPS DATE 4-6-96	KTW DATE 5/1/96
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JO. 76230.502					

H-49 : CONT

JOINT 9

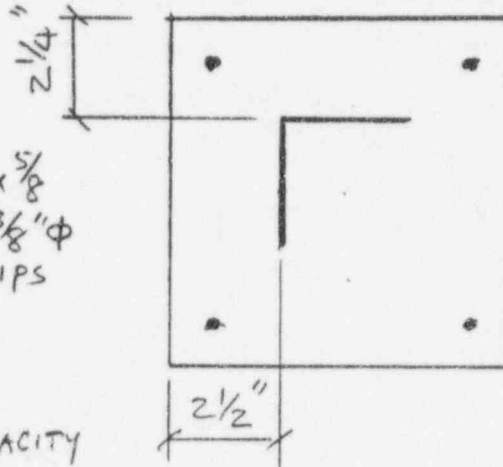
DL ± SSE

$F_x = 5 \#$

$F_y = 969 \#$

$M_z = 49 \#'$

B.P. :  $8 \times 8 \times \frac{5}{8}$   
A.B. :  $4 - \frac{3}{8} \text{ " } \phi$   
PHILLIPS



REF

DC 2836  
ATT. 5

STARDYNE  
OUTPUT  
4/5/96  
ATT. B 9

FROM BASE PL. TEN-MOMENT CAPACITY  
CHART B ZONE III

$$\text{BOLT TEN IR} = \frac{969}{3730} + \frac{49}{955} = 0.31$$

SHEAR IS NEGLIGIBLE

PLATE STRESS IS WITHIN THE T-M CAPACITY CHART  
IS OK

\* UNVELOPED FOR L.C. # 7 & 8

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COMPUTATION  
SHEET

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

SUBJECT SUPPORT EVALUATION FOR  
SYSTEM 2850 - 2

CALCULATION SET NO.

DC-5766

PRELIM FINAL VOID

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0	JRS DATE 4-6-96	KTW DATE 5/21/96
	DATE	DATE

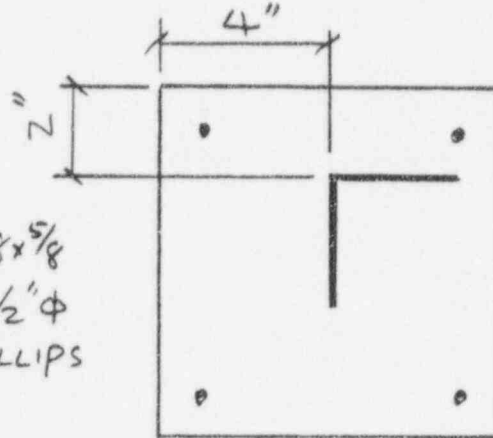
H-49: CONT

JOINT 13

$F_x = 26 \#$   
 $F_y = 1670 \#$   
 $M_z = 16 \#$

\* DLISSE

B.P.:  $8 \times 8 \times \frac{5}{8}$   
A.B.:  $4 - \frac{1}{2} \phi$   
PHILLIPS



REF

DC 2836  
ATT 5

STARDYNE  
OUTPUT  
4/5/96  
ATT 139

FROM BASE PL. TEN-M CAPACITY CHART 'C', ZONE II  
CONSERVATIVE

$$\text{BOLT IR} = \frac{1670}{3150} + \frac{16}{920} = 0.55 < 1.0$$

SHEAR IS NEGLIGIBLE

PLATE STRESS IS WITHIN THE T-M CAPACITY CHART  
IS OK

\* ENVELOPED FOR LC # 7 & 8

**Raytheon**  
 Engineers & Constructors

 GENERAL  
 COMPUTATION  
 SHEET

 DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

 SUBJECT SUPPORT EVALUATION FOR  
SYSTEM 2850-2

CALCULATION SET NO

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 DATE  
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EVALUATION OF BASE PLATE

GM-2850-H-54

DL+OBE (X+Y)

JOINT 1 :

$$F_x = 14 \text{ }^\# \text{ COMB 4}$$

$$F_y = 1345 \text{ }^\# \text{ COMB 4}$$

$$M_z = 495 \text{ }^{\prime\prime\#} \text{ COMB 4}$$

JOINT 7 :

$$F_x = 5 \text{ }^\# \text{ COMB 3}$$

$$F_y = 498 \text{ }^\# \text{ COMB 4}$$

$$M_z = 240 \text{ }^{\prime\prime\#} \text{ COMB 4}$$

JOINT 8 :

$$F_x = 517 \text{ }^\# \text{ COMB 1}$$

$$F_y = 912 \text{ }^\# \text{ COMB 1}$$

$$M_z = 109 \text{ }^{\prime\prime\#} \text{ COMB 4}$$

REF

 STARDYNE  
 OUTPUT  
 = 4/4/96  
 ATT. B10

<b>Raytheon</b> Engineers & Constructors DECO FERMI 2 SHEET CCHVAC DUCT & DUCT SUPPORTS PROJECT STRUCTURAL QUALIFICATION SUBJECT <u>SUPPORT EVALUATION FOR SYSTEM 2850-2</u>			CALCULATION SET NO.			REV.	COMP BY	CHK'D BY
			DC-5766			0	JFS	ETW
PRELIM	FINAL	VOID	SHEET 189 OF 246		DATE	DATE		
JO 76230.502					4-6-96	5/2/96		

H-54 : CONT.

DL + SSE (X+Y)

JOINT 1

$$F_x = 23 \text{ \#} \quad \text{COMB } 8$$

$$F_y = 2054 \text{ \#} \quad \text{COMB } 8$$

$$M_z = 814 \text{ \#} \quad \text{COMB } 8$$

JOINT 7

$$F_x = 8 \text{ \#} \quad \text{COMB } 7$$

$$F_y = 647 \text{ \#} \quad \text{COMB } 8$$

$$M_z = 389 \text{ \#} \quad \text{COMB } 8$$

JOINT 8

$$F_x = 865 \text{ \#} \quad \text{COMB } 7$$

$$F_y = 1514 \text{ \#} \quad \text{COMB } 7$$

$$M_z = 173 \quad \text{COMB } 8$$

REF

ATT. B10  
 STARDYNE  
 OUTPUT  
 4/4/96

**Raytheon**  
 Engineers & Constructors

 GENERAL  
 COMPUTATION  
 SHEET

DECO FERMI 2

CCHVAC DUCT &amp; DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION

 SUBJECT SUPPORT EVALUATION FOR  
 SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

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 DATE  
 4-6-96

 KTW/L  
 DATE  
 5/21/96

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JO 76230.502

DATE

DATE

H-54 : CONT.

BASE PLATE : 8x8x 1/2

A. B : 4 - 3/8" φ PHILLIPS ANCHOR

SUPPORT MEMBER : L 2 1/2 x 2 1/2 x 1/4

JOINT 1

DL + SSE (X+Y)

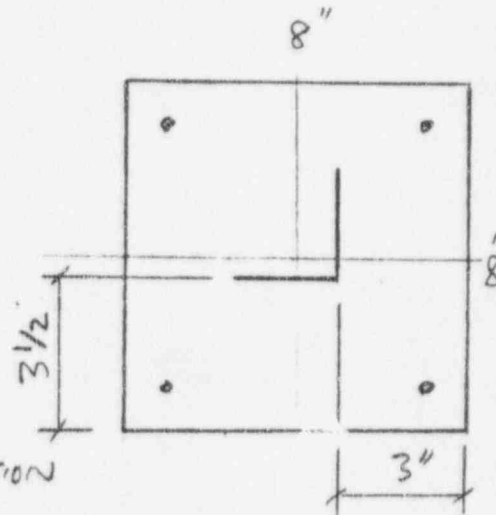
$$F_x = 23 \#$$

$$F_y = 2054 \#$$

$$M_z = 814 \text{ " \#} = 68 \text{ ' \#}$$

 CAPACITY OF BASE PLATE IS  
 TAKEN BY LINEAR INTERPOLATION

OF CHARTS B &amp; C, ZONE III



REF

 DWG. 10-4221  
 000-GM-2850-  
 54-A

 ABM-0510  
 SH. Z  
 DETAIL A-A

$$\text{TENSION, } T = 3730 - (3730 - 2110) \times \frac{(3.5 - 2.75)}{(2.75 - 1.55)} = 2718 \#$$

$$\text{MOMENT, } M = 955 - (955 - 633) \times 0.625 = 754 \text{ ' \#}$$

$$\text{BOLT TENSION I.R.} = \frac{2054}{2718} + \frac{68}{754} = 0.85 < 1.0 \text{ OK}$$

$$\text{BOLT SHEAR I.R.} = \frac{23}{4 \times 690} = 0.008 \text{ NEGLIGIBLE}$$

PLATE STRESS IS WITHIN THE T&amp;M CHART IS OK

THIS BASE PLATE IS ADEQUATE

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PROJECT STRUCTURAL QUALIFICATION

 SUBJECT SUPPORT EVALUATION FOR  
 SYSTEM 2850 - 2

CALCULATION SET NO

DC-5766

REV

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 JFS  
 DATE  
 4-8-96

 KTW  
 DATE  
 5/21/96

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DATE

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H-54 : CONT.

JOINT 7

DL + SSE. (X+Y)

$$F_x = 8 \text{ \#}$$

$$F_y = 647 \text{ \#}$$

$$M_z = 389 \text{ \#} = 32 \text{ \#}$$

T &amp; M CAPACITIES :

USE CHART C, ZONE III

$$T = 2110 \text{ K} ; M = 633 \text{ IK}$$

$$\text{BOLT TENSION IR} = \frac{647}{2110} + \frac{32}{633} = 0.36$$

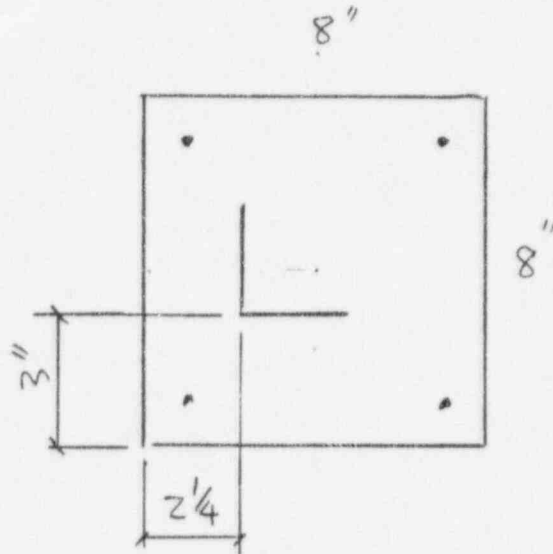
$$\text{BOLT SHEAR IR} = \frac{8}{4 \times 690} = 0$$

$$\text{BOLT IR} = 0.36 < 1.0 \quad \text{OK}$$

PLATE STRESS IS WITHIN THE T-M CHART

IS WITHIN THE ALLOWABLE LIMIT

THIS BASE PLATE IS ADEQUATE



REF.

ABM-0510  
SH 2

DET. B-B



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H-54 : CONT.JOINT 8

DL + SSE (X+Y) :

$$F_x = 865 \#$$

$$F_y = 1514 \#$$

$$M_z = 173 \text{ " \#} = 14 \text{ ' \#}$$

T &amp; M CAPACITIES :

USE CHART C, ZONE III

$$T = 2110 \text{ K}$$

$$M = 633 \text{ K}$$

$$\text{BOLT TENSION IR} = \frac{1514}{2110} + \frac{14}{633} = 0.74$$

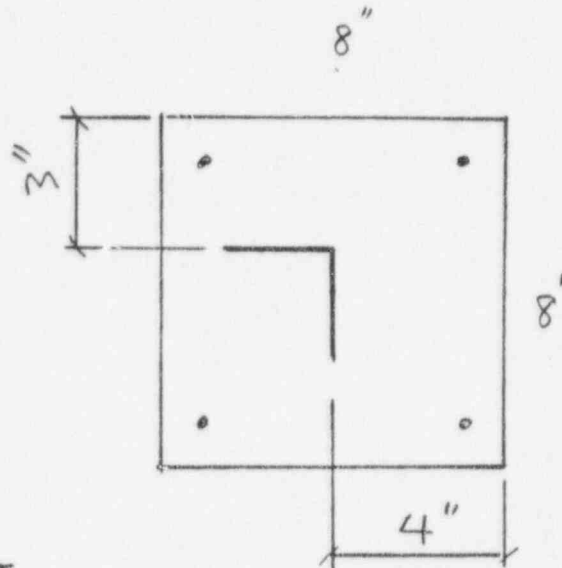
$$\text{BOLT SHEAR IR} = \frac{865}{4 \times 690} = 0.31$$

$$\text{BOLT IR} = (0.74)^{5/3} + (0.31)^{5/3} = 0.75 < 1.0$$

OK

PLATE STRESS IS WITHIN THE T & M CHART  
 THEREFORE IS OK

THIS BASE PLATE IS ADEQUATE



REF.

 ABM-0510  
 SM Z  
 DET C-C

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GM-2850-H-58

REF

DL+SSE : LINEAR ADJUSTMENT OF H-59 FORCE  
AND MOMENTJOINT 1 -

$$F_x = 140 \times \left( \frac{46}{115} \right) + 140 \times \left( \frac{60}{115} \right) = 129 \text{ \#}$$

$$F_y = 164 \times \left( \frac{286+171}{240} \right) + 78 \times \left( \frac{687+45}{156} \right) + 874 \times \left( \frac{60}{115} \right) + 874 \times \left( \frac{46}{115} \right) \left( \frac{102}{136.6} \right)$$

$$= 1395 \text{ \#}$$

$$M_z = 3985 \text{ \#} \times \left( \frac{60}{115} \right) + 3985 \text{ \#} \times \left( \frac{46}{115} \right) \left( \frac{102}{136.6} \right) = 3269 \text{ \#} \text{ in}$$

$$= 272 \text{ \#}$$

BASE PLATE : 8 x 8 x 5/8

A.B. : 4 - 1/2"  $\phi$  PHILLIPS

SUPPORT MEMBER : L3 x 3 x 1/4

PER ABM-0510 SH 5 THIS BASE PLATE IS  
INSTALLED PER DESIGNABM-0510  
SH. 5FROM BASE PL TENSION-MOMENT CAPACITY CHART 'B'  
ZONE II

$$\text{TEN CAP } T = 5560 \text{ \#}$$

$$\text{M CAP } M = 1388 \text{ \#}$$

$$\text{BOLT TEN } I R = \frac{1395}{5563} + \frac{272}{1388} = 0.45 < 1.0 \text{ OK}$$

SHEAR IS NEGLIGIBLE

BASE PLATE STRESS IS WITHIN THE CAPACITY CHART 'B' IS OK

DWG GM-  
2850-58-ASTRING  
ANALYSIS  
OUTPUT  
3/29/96

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H-58 : CONTJOINT 10

DL + SSE

$$F_x = 25^{\#} \times \left( \frac{46+60}{115} \right) = 23^{\#}$$

$$F_y = 1395^{\#}$$

$$M_z = 974 \times \left( \frac{46+60}{115} \right) = 898^{\#\text{in}}$$

$$= 75^{\#\text{in}}$$

BASE PL : 8x8x 5/8

A.B. : 4 - 1/2"  $\phi$  PHILLIPS

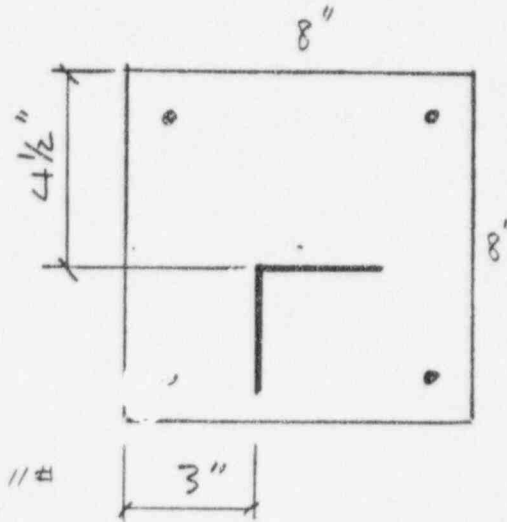
SUPPORT MEMBER : L 3x3x 1/4

FROM BASE PL. TEN-MOMENT CAPACITY CHART C, ZONE II

$$\text{BOLT TEN IR} = \frac{1395}{3150} + \frac{75}{920} = 0.52 < 1.0 \quad \text{OK}$$

SHEAR IS NEGLIGIBLE

PLATE STRESS IS WITHIN THE CAPACITY CHART IS OK



REF.

ABM-0510  
SH. 5

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## EVALUATION OF BASE PLATE

GM-2850-H-59

SUPPORT JOINT 1:

 D.L OF SUPPORT  $\frac{1}{2}$  DUCT PLUS DL OF MEMBER

$$F_x = 0$$

$$F_y = 120 \# + 43.6 = 163.6 \#$$

$$M_z = 0$$

OBE Y

$$F_x = 0$$

$$F_y = 78.0 \#$$

$$M_z = 0$$

OBE X

$$F_x = 140. \#$$

$$F_y = 873.9 \#$$

$$M_z = 3985 \#$$

COMB. LOAD : DL + OBE (X+Y)

$$F_x = 140 \# \quad \text{COMB 3}$$

$$F_y = 163.6 + 78. + 873.9 = 1115.5 \# \quad \text{COMB 3}$$

$$M_z = 3985 \# \quad \text{COMB 3}$$

$$= 3321 \#$$

 OBE  
 GOVERNS

REF

 ATT. B11  
 STARDYNE  
 OUTPUT

# 4/4/96

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SUBJECT SUPPORT EVALUATION FOR  
SYSTEM 2850-2

CALCULATION SET NO. DC-5766			REV 0	COMP. BY JFS	CHK'D BY ETW.
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H-59 : CONT.

SUPPORT JOINT 1 : CONT.

DL + SSE (X+Y)

$$F_x = 140 \times \left( \frac{101}{115} \right) = 123 \text{ \#}$$

$$F_y = 163.6 + 78 \times \left( \frac{247}{156} \right) + 873.9 \times \left( \frac{101}{115} \right) = 1054.6 \text{ \#}$$

$$M_z = 3985 \times \left( \frac{101}{115} \right) = 3500. \text{ \#} = 292 \text{ ' \#}$$

$$F_z = M_x = M_y = 0$$

JOINT 10 :

DL + OBE (X+Y) :

$$F_x = 25 \text{ \#} \quad \text{COMB 4}$$

$$F_y = 120 + 44 + 78 + 874 = 1116 \text{ \#} \quad \text{COMB 4}$$

$$M_z = 974 \text{ \#} = 81 \text{ ' \#} \quad \text{COMB 4}$$

GOVERNS

ATT. B II  
STARDYNE  
OUTPUT  
# 4/4/96

DL + SSE (X+Y)

$$F_x = 25 \left( \frac{101}{115} \right) = 22 \text{ \#}$$

$$F_y = 1055 \text{ \#}$$

$$M_z = 974 \left( \frac{101}{115} \right) = 855 \text{ \#}$$

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H 59 : CONT.

JOINT 1

DL + OBE (X+Y)

@ E OF BOLTS

$$F_{X'} = 140$$

$$F_{Y'} = 1115.5 \#$$

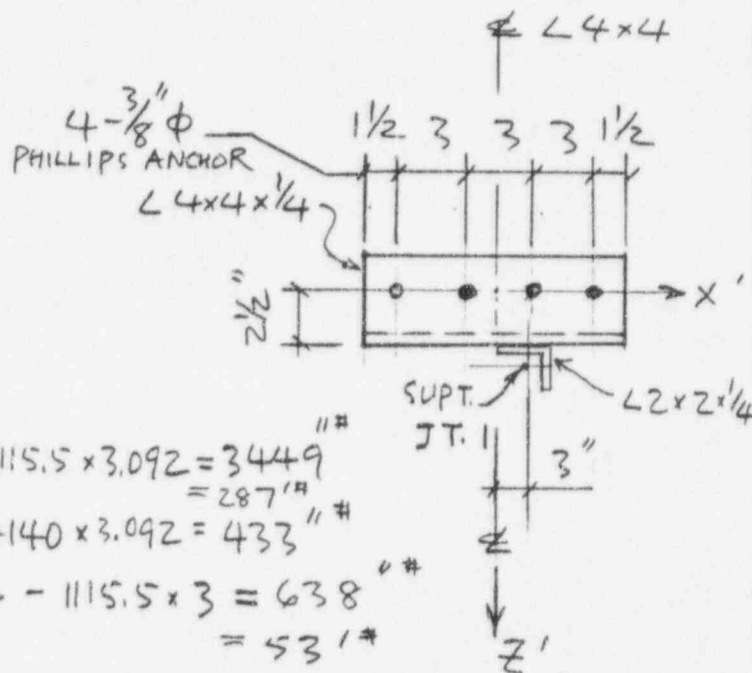
$$F_{Z'} = 0$$

$$M_{X'} = F_{Y'} \times (2.5 + 0.592) = 1115.5 \times 3.092 = 3449$$

$$M_{Y'} = F_{X'} \times (2.5 + 0.592) = 140 \times 3.092 = 433 \#$$

$$M_{Z'} = M_Z - F_{Y'} (3) = 3984 - 1115.5 \times 3 = 638$$

$$= 53 \#$$



BOLT TENSION : T

$$T = \frac{1116}{4} + \frac{638 \times 10.5}{(1.5^2 + 4.5^2 + 7.5^2 + 10.5^2)} + \frac{3449}{4 \times (4 - 2.5)}$$

$$= 279 + 35.4 + 575 = 889 \# < 1240 \text{ ALLOW OK}$$

$$\text{SHEAR : } V = \sqrt{\left(\frac{140}{4}\right)^2 + \left(\frac{433 \times 4.5}{2(1.5^2) + 2(4.5^2)}\right)^2} = 56 \# < 690 \# \text{ ALLOW OK}$$

$$I R = \left(\frac{889}{1240}\right)^{5/3} + \left(\frac{56}{690}\right)^{5/3} = 0.573 + 0.015 = 0.59 < 1.0 \text{ OK}$$

CHECK L4x4x1/4

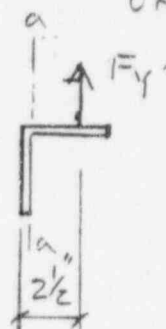
@ SECTION A-A

$$M = F_{Y'} \times (2.5 - 0.25) = 1116 \times 2.25 = 2511$$

$$f_b = \frac{2511}{12 \times 0.25^2 / 6} = 20,088 \text{ PSI} < 21,600 \text{ PSI OK}$$

$$f_v = 1116 \# / (12 \times 0.25) = 372 < 0.4 \times 36000 = 14,400 \text{ PSI OK}$$

THIS BASE PLATE IS ADEQUATE



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SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-2

CALCULATION SET NO

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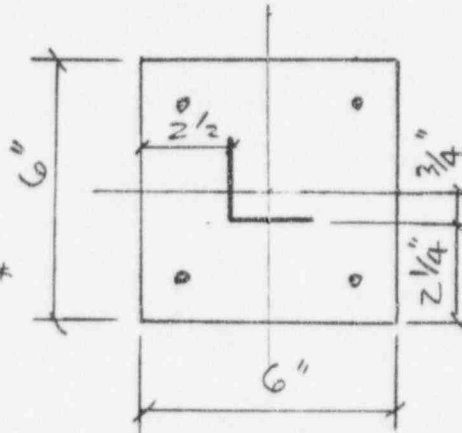
H 59 : CONT.JOINT 10 :

$$F_x = 25 \#$$

$$F_y = 1116 \#$$

$$M_z = 974 \text{ " } \# = 81 \text{ ' } \#$$

$$F_z = M_x = M_y = 0$$



REF

 ABM-0510  
 SH. 6
BASE PL:  $6 \times 6 \times \frac{3}{8}$ A.B.:  $4 - \frac{3}{8} \text{ " } \phi$  PHILLIPS ANCHORSUPPORT MEMBER:  $\angle 2 \times 2 \times \frac{1}{4}$ 
 EVALUATION OF BASE PLATE BY USING  
 BASE PLATE TENSION-MOMENT CHART C  
 ZONE IV,

$$\text{BOLT TEN IR} = \frac{1116}{3660} + \frac{81}{630} = 0.43$$

$$\text{BOLT SHEAR IR} = \frac{25}{4 \times 690} = 0.009$$

$$\text{IR} = 0.44 < 1.0 \text{ OK}$$

 PLATE STRESS IS WITHIN THE TENSION-MOMENT  
 CHART IS OK

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GM-2850-H-60

BASE PLATE LOADS ARE LINEAR ADJUSTMENT OF H-59.

JOINT 1

DL + OBE (X+Y)

$$F_x = 140 \times 2.148 = 301 \text{ \#}$$

$$F_y = 160 \times 83 + 78 \times 0.34 + 874 \times 2.148$$

$$= 2043 \text{ \#}$$

$$M_z = 3985 \times 2.148 = 8560 \text{ \#\#}$$

$$= 713 \text{ \#\#}$$

OBE RATIO OF  $\left(\frac{H-60}{H-59}\right)$ 

$$\text{OBE } X : \frac{247}{115} = 2.148$$

$$\text{OBE } Y : \frac{53}{156} = 0.340$$

$$\text{DL} : \frac{199}{240} = 0.83$$

SINCE THE LATERAL REACTION FOR SDE (205<sup>\#</sup>)  
 IS LESS THAN OBE (247<sup>\#</sup>), THEREFORE THE  
 OBE CASE GOVERNS

JOINT 10

DL + OBE (X+Y)

$$F_x = 25 \times 2.148 = 54 \text{ \#}$$

$$F_y = 2043 \text{ \#}$$

$$M_z = 974 \times 2.148 = 2092 \text{ \#\#}$$

$$= 174 \text{ \#\#}$$

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H-60 : CONTJOINT 1

BASE PLATE 'B'

BASE PLATE :  $6 \times 6 \times \frac{3}{8}$ A.B. :  $4 - \frac{3}{8}'' \phi$  PHILLIPS ANCHORSSUPPORT MEMBER :  $L 2 \times 2 \times \frac{1}{4}$ 

DL + OBE (X+Y)

@ C.G. OF SUPPORT MEMBER

$$F_x = 301 \text{ \#}$$

$$F_y = 2043 \text{ \#}$$

$$M_z = 8560 \text{ \#}$$

REACTIONS @ C.G. OF BASE PLATE :

$$F_x = 301 \text{ \#}$$

$$F_y = 2043 \text{ \#}$$

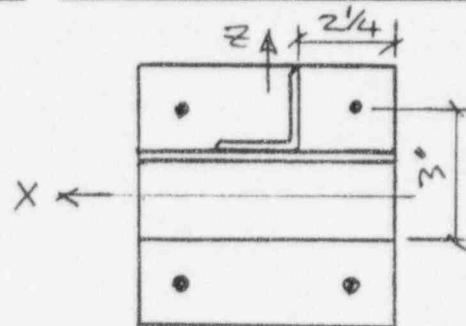
$$M_z = 8560 - 2043 \times (3.0 - 2.25 - 0.592) = 8237 \text{ \#}$$

$$M_y = 301 \times (1 + 0.592) = 479 \text{ \#}$$

$$M_x = 2068 \times 1.592 = 3292 \text{ \#}$$

$$\text{BOLT TENSION, } T = \frac{2043}{4} + \frac{8237}{2 \times 5} + \frac{3292}{2 \times 3} = 1883 > 1240 \text{ \#}$$

NG



RUF

DCN-10891

REV A

SH. 2

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H-60 : CONTJOINT 10

BASE PL 'A'

BASE PL : 6x6x3/8

A.B. : 4 - 3/8" PHILLIPS ANCHORS

SUPP. MEMBER : L2x2x1/4

DL+OBE (X+Y)

REACTIONS @ C.G. OF SUPP. MEMBER

$$F_x = 54 \text{ \#}$$

$$F_y = 2043 \text{ \#}$$

$$M_z = 2092 \text{ \#}$$

REACTIONS @ C.G. OF BASE PLATE :

$$F_x = 54 \text{ \#}$$

$$F_y = 2068 \text{ \#}$$

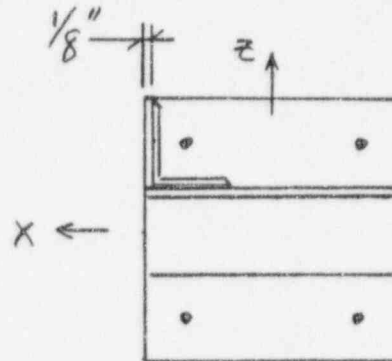
$$M_z = 2092 - 2043 \times (3 - 0.125 - 0.592) = -2572 \text{ \#}$$

$$M_x = 2043 \times (1 + 0.592) = 3252 \text{ \#}$$

$$M_y = 54 \times 1.592 = 86 \text{ \#}$$

$$\text{BOLT TEN. } T = \frac{2043}{4} + \frac{2572}{2 \times 5} + \frac{3252}{2 \times 3} = 1310.71240 \text{ \#}$$

N 07



REF.

 DCN-10891  
 REV A  
 SH 2

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 EVALUATION OF BASE PLATES BASED ON MOD. STRING ANALY.  
 OUTPUTS

GM-2850-H-60 (CONT'D)

SUPPORT LOADS

D.L : VERT (Y) = 199 #

LAT (X) = 0

OBE : VERT (Y) = 52 #

LAT (X) = 89

SSE : VERT (Y) = 53 #

LAT (X) = 99 #

GOVERNS

 STRING  
 ANALYSIS  
 OUTPUT

4/22/96

 BASE PLATE JOINT REACTIONS ARE CALCULATED  
 BASED ON LINEAR ADJUSTMENT OF H-59  
 FORCES & MOMENTS.

$$DL \text{ RATIO} = \frac{199}{240} = 0.83$$

$$SEISMIC \text{ VERT (Y) RATIO} = \frac{53}{156} = 0.340$$

$$LAT (X) \text{ RATIO} = \frac{99}{115} = 0.861$$

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GM-2850-H-60 : CONT.

REF

JOINT #1 CONTROLLING JOINT

DL + SSE (X + Y)

$$F_x = 140 \times 0.861 = 121 \text{ \#}$$

$$F_y = 160 \times 0.83 + 78 \times 0.340 + 874 \times 0.861 = 912 \text{ \#}$$

$$M_z = 3985 \times 0.861 = 3431 \text{ \#} = 286' \text{ \#}$$

CHART 'A' ZONE IV

$$IR = \frac{912}{1300} + \frac{286}{314} = 1.61 > 1.0 \quad NG$$

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 SUBJECT SYSTEM 2850-2

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5/22/96

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H-60 (MODIFIED)

TENSION ON VERTICAL MEMBER

$$\frac{99 \times 130 + 252 \times 37}{32} = 693 \text{ lb}$$

TENSION ON BRACING

$$\frac{99 \times 130 + 252 \times 5}{5 + 32} = 382 \text{ lb}$$

FOR BASE PLATES A &amp; B

SINCE THE TENSION ON BASE PLATE IS SMALLER THAN THE ALLOWABLE FOR A SINGLE  $\frac{3}{8}$ "  $\phi$  PHILLIPS ANCHOR (1240 lb/ANCH). THEREFORE, PLATES A & B ARE ADEQUATE SHEAR IS NEGLIGIBLE.

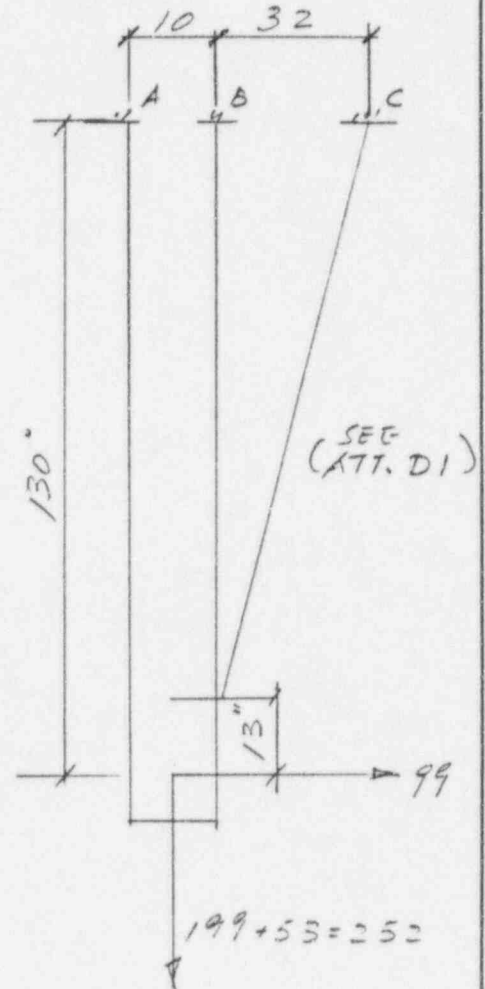
BASE PLATE AT C IS

$10 \times 10 \times \frac{3}{4}$  WITH 4  $\frac{1}{2}$ "  $\phi$  HKB II HILTI BOLTS. THIS PLATE IS ADEQUATE SINCE THE TENSION AT BASE PLATE IS SMALLER THAN THE ALLOWABLE OF A SINGLE HILTI BOLT. (2625 lb/BOLT)

\* FROM DECO SPEC 3071-226  
REV J, APPENDIX C

\* 2 GATEWAY INSERTS AND 2  $\frac{1}{2}$ "  $\phi$  HKB II IS USED IN LIEU OF 4  $\frac{1}{2}$ "  $\phi$  HKB II.

GATEWAY INSERT ALLOWABLE IS 3000 #/LFT  $\therefore$  IS ACCEPTABLE  
LEA, 6/1/96



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CC-IVAC DUCT & DUCT SUPPORTS

PROJECT: STRUCTURAL QUALIFICATION

SUBJECT: SUPPORT EVALUATION FOR  
SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

REV

COMP. BY

CHK'D BY

PRELIM

FINAL

VOID

0

JFS

KTW/c.

DATE

DATE

4-9-96

5/21/96

SHEET 205 OF 246

JO 76230, 502

DATE

DATE

GM-2850-H-61

BASE PLATE LOADS ARE CALCULATED BY  
LINEAR ADJUSTMENT OF H-59 :

REF

STRING  
ANALYSIS  
OUTPUT  
3/29/96

JOINT 1

DL + OBE (X+Y)

OBE RATIO OF  $\left(\frac{H-61}{H-59}\right)$

$$F_x = 140 \# \times 3.452 = 483 \#$$

$$\text{OBE } X : \frac{397}{115} = 3.452$$

$$F_y = 164 \times 0.73 + 78 \times 0.288 + 874 \times 3.452$$

$$= 3159 \#$$

$$\text{OBE } Y : \frac{45}{156} = 0.288$$

$$\text{DL} : \frac{175}{240} = 0.73$$

$$M_z = 3985 \times 3.452 = 13,756 \text{ ''\#}$$

$$= 1146 \text{ ' \#}$$

DL + SSE (X+Y)

SINCE THE LATERAL REACTION FORCE FOR SSE (327)<sup>#</sup>  
IS LESS THAN OBE (397<sup>#</sup>), THEREFORE THE OBE  
CASE GOVERNS.

JOINT 10

DL + OBE (X+Y)

$$F_x = 25 \times 3.452 = 86 \#$$

$$F_y = 3159 \#$$

$$M_z = 974 \times 3.452 = 3362 \text{ ''\#}$$

SINCE THE LATERAL REACTION FOR SSE IS  
LESS THAN OBE THEREFORE THE OBE CASE  
GOVERNS

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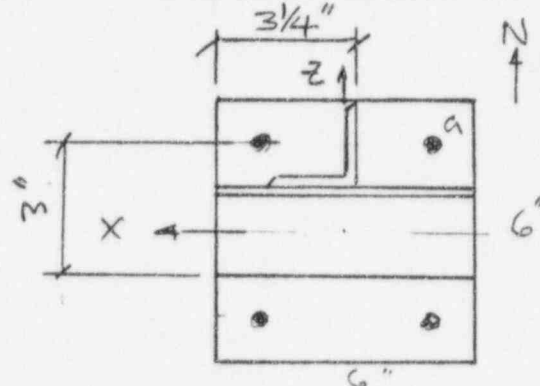
GENERAL  
COMPUTATION  
SHEET

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS

PROJECT STRUCTURAL QUALIFICATION  
SUBJECT SUPPORT EVALUATION FOR SYSTEM 2850-2

CALCULATION SET NO. DC-5766			REV	COMP. BY	CHK'D BY
PRELIM	FINAL	VOID	0	JFS DATE 4-9-96	KT W/L DATE 5/2/96
SHEET 206 OF 246				DATE	DATE
JO 76230.502					

H-61 : CONT.  
JOINT 1  
PLATE 'B'



REF  
DCN-10890  
REV 'B'

BASE PL : 6x6x 3/8"  
A.B. : 1-1/2" φ WEDGE BOLT & 3-3/8" φ PHILLIP ANCHORS  
SUPPORT MEMBER : L 2x2x1/4

DL + ORE (X+Y) :

$F_x = 483 \text{ \#}$	REACTIONS @ C.G OF SUPPORT MEMBER
$F_y = 3159 \text{ \#}$	
$M_z = 13756 \text{ \#}$ $= 1146 \text{ \#}$	

REACTIONS @ E OF BASE PLATE :

$F_x = 483 \text{ \#}$   
 $F_y = 3159 \text{ \#}$   
 $M_z = 3159 \times (0.592 - 0.25) + 13756 = 14836 \text{ \#\#}$   
 $M_x = 3159 \times (0.592 + 1.0) = 5029 \text{ \#\#}$   
 $M_y = 483 \times (0.592 + 1.0) = 769 \text{ \#\#}$

BOLT TENSION  $T_a = \frac{3159}{4} + \frac{5029}{2 \times 3} + \frac{14836}{2 \times 5} = 3111 \text{ \#}$   
 $> 1240 \text{ \#}$  NG

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 SUBJECT SUPPORT EVALUATION FOR  
 SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

PRELIM.

FINAL

VOID

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REV

COMP. BY

CHK'D BY

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 JPS  
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 4-9-96

 KTW.  
 DATE  
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DATE

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H-61 : CONT.

JOINT 10

PLATE 'A' :

 BASE PL :  $6 \times 6 \times \frac{3}{8}$ "

 A.B. : 4 -  $\frac{3}{8}$ "  $\phi$  PHILLIPS ANCHOR

 SUPPORT MEMBER :  $L2 \times 2 \times \frac{1}{4}$ 

DL + OBE (X+Y)

$$F_x = 86 \#$$

$$F_y = 3204 \#$$

$$M_z = 3362 \text{ " \#} = 280' \text{ \#}$$

 REACTIONS @ C.G. OF  
 SUPPORT MEMBER

 REACTION @  $\phi$  OF BASE PLATE :

$$F_x = 86$$

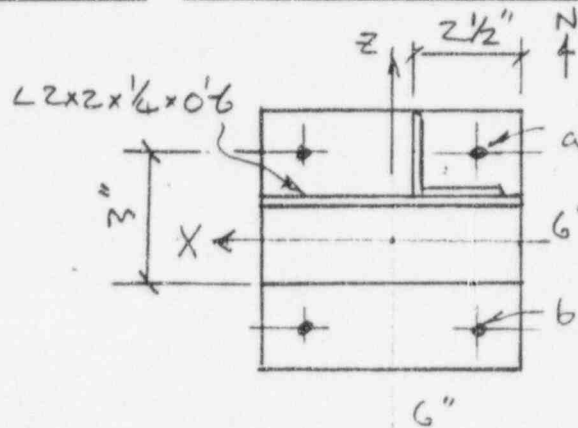
$$F_y = 3204 \#$$

$$M_x = 3204 \times (0.592 + 1) = 5101 \text{ " \#}$$

$$M_z = 3204 \times (0.592 + 0.5) + 3362 = 6861 \text{ " \#}$$

$$M_y = 86 \times 1.592 = 137 \text{ " \#}$$

$$\text{BOLT TENSION, } T_a = \frac{3204}{4} + \frac{5101}{2 \times 3} + \frac{6861}{2 \times 5} = 2337 \text{ \#}$$

 $> 1240 \text{ NG}$ 


REF

 DCN-10890  
 REV "B"



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PROJECT STRUCTURAL QUALIFICATION

SUBJECT SUPPORT EVALUATION FOR  
SYSTEM 2850-2

CALCULATION SET NO. DC-5766			REV	COMP. BY	CHK'D BY
PRELIM	FINAL	VOID	0	YFS DATE 5-4-96	KTW DATE 5/2/96
SHEET 208 OF 246				DATE	DATE
JO. 76230.502					

REF

GM-2850-H-61 : CONT.

BASE PLATE LOADS ARE BASED ON MODIFIED  
STRING ANALYSIS OUTPUT (4/22/96)

JOINT # 1 :

DL + SSE :

DCN-10891  
REV A  
SH. 2

$$F_x = 140 \# \times \left( \frac{97}{115} \right) = 118 \#$$

$$F_y = 160 \times \left( \frac{175}{240} \right) + 78 \times \left( \frac{39}{156} \right) + 874 \times \left( \frac{97}{115} \right) = 873 \#$$

$$M_z = 3985 \times \left( \frac{97}{115} \right) = 3361 \text{ " \#} = 280 \text{ ' \#}$$

STRING ANALYSIS  
OUTPUT  
4/22/96

USE T-M CAPACITY CHART C ZONE IV

$$IR = \frac{873}{2110} + \frac{280}{420} = 1.08 > 1.0 \text{ NG}$$

GM-2850-H-63 & H-64

BASE PLS. FAIL BY COMPARISON

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PROJECT STRUCTURAL QUALIFICATION  
SUBJECT SUPPORT EVALUATION FOR SYSTEM 2850-2

CALCULATION SET NO. DC-5766			REV 0	COMP BY KT wli	CHK'D BY JFS
PRELIM	FINAL ✓	VOID		DATE 5/17/96	DATE 5/22/96
SHEET 209 OF 246				DATE	DATE
JO 76230.502					

H-61 (MODIFIED)

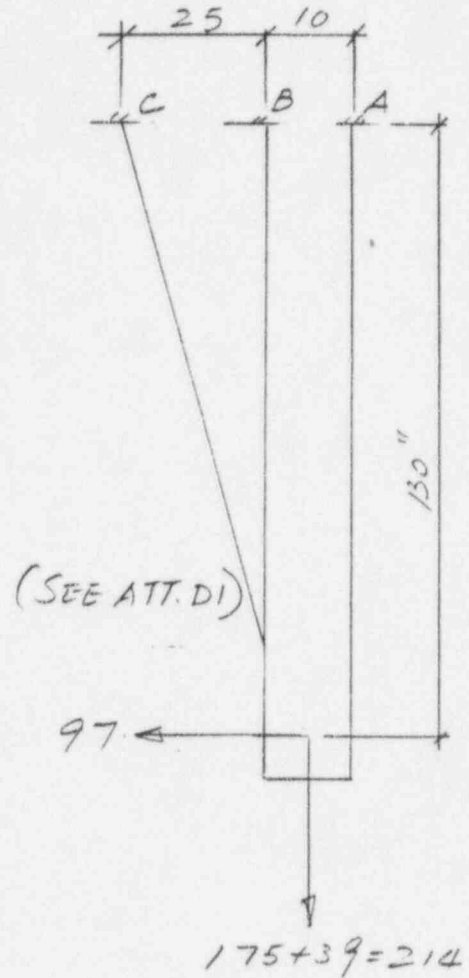
TENSION ON VERTICAL MEMBER

$$\frac{97 \times 130 + 214 \times 30}{25} = 760 \text{ lb}$$

TENSION ON BRACING:

$$\frac{97 \times 130 + 214 \times 5}{30} = 456 \text{ lb}$$

THE BASE PLATES ARE ADEQUATE BY THE SAME JUSTIFICATION FOR H-60



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PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

REV

COMP BY

CHK'D BY

0  
 DATE  
 5/10/96JRS  
 DATE  
 5/10/96PRELIM  
 FINAL  
 VOID

SHEET 210 OF 246

JO 76230.502

DATE

DATE

6M-2850-H-62

DL+SSE (JT. 1. PLATE A AS PER ABM-0510)

$$F_x = 140 \times \frac{116}{115} = 141 \text{ #}$$

$$F_y = 160 \times \left(\frac{215}{240}\right) + 78 \times \left(\frac{53}{156}\right) + 874 \times \left(\frac{116}{115}\right) = 1051 \text{ #}$$

$$M_z = 3985 \times \left(\frac{116}{115}\right) = 4020 \text{ ''-#} = 355 \text{ ' - #}$$

THE AVERAGE VALUES OF BASE PLATE CHART B & C, ZONE IV  
 ARE USED FOR EVALUATION

$$IR = \left(\frac{1051}{\frac{2110+3660}{2}}\right)^{\frac{4}{3}} + \left(\frac{335}{\frac{420+631}{2}}\right)^{\frac{4}{3}} = 0.81 < 1.0 \quad \text{O.K.}$$

SHEAR IS NEGLIGIBLE

JT. 10 PLATE B AS PER ABM-0510

$$F_x = 141 \text{ #}$$

$$F_y = 1051 \text{ #}$$

$$M_z = 974 \times \left(\frac{116}{115}\right) = 982 \text{ ''-#} = 81 \text{ ' - #}$$

PLATE B IS BOUNDED BY PLATE A. THEREFORE BOTH  
 PLATES AND ANCHOR BOLTS ARE ADEQUATE.

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COMPUTATION  
SHEET

DECO FERMI 2

PROJECT CCHVAC: DUCT & DUCT SUPPORTS

STRUCTURAL QUALIFICATION

SUBJECT SYSTEM 2850-2

CALCULATION SET NO <b>DC-5766</b>			REV	COMP BY	CHK'D BY
PRELIM	FINAL	VOID	0	<i>ETU/LC</i>	<i>JPL</i>
	✓		DATE	DATE	DATE
			5/10/96	5/22/96	
SHEET <u>211</u> OF <u>246</u>			DATE	DATE	DATE
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6M-2850-H-63      §      6M-2850-H-64

USE THE BASE PLATE LOADS FROM H-62 SINCE THE SUPPORT LOADS OF H-63 ARE ENVELOPED BY THAT OF H-62  
§ H-64

JT. 1 (PLATE B; H-63, SEE ABM-0510)

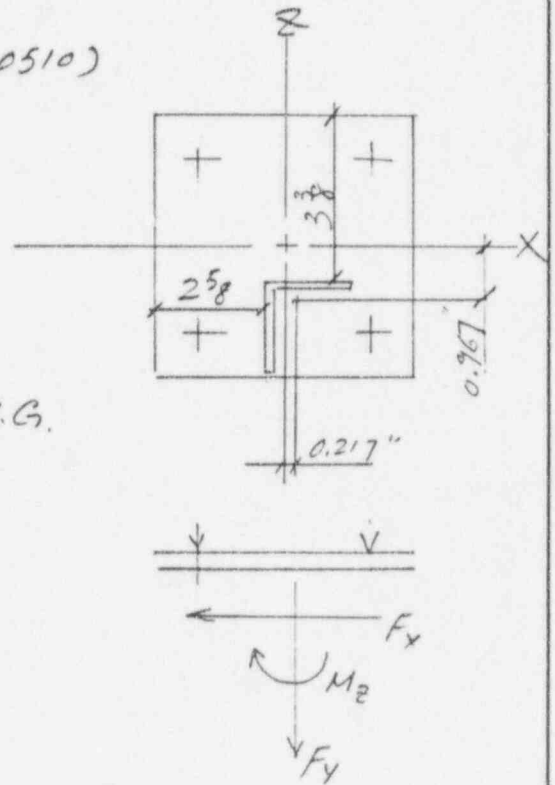
$$F_x = 141 \#$$

$$F_y = 1051 \#$$

$$M_z = 4020 \text{''-}\# = 335 \text{'}-\#$$

USE BASE PLATE CHART C, ZONE IV,

$$IR = \frac{1051}{2110} + \frac{335}{420} = 1.30 > 1.0 \text{ N.G.}$$



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PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-2

CALCULATION SET NO.

DC-5766

REV

COMP BY

CHK'D BY

PRELIM

FINAL

VOID

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E.T.W.

JFK

DATE

DATE

5/17/96

5/22/96

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DATE

H-63 (MODIFIED)

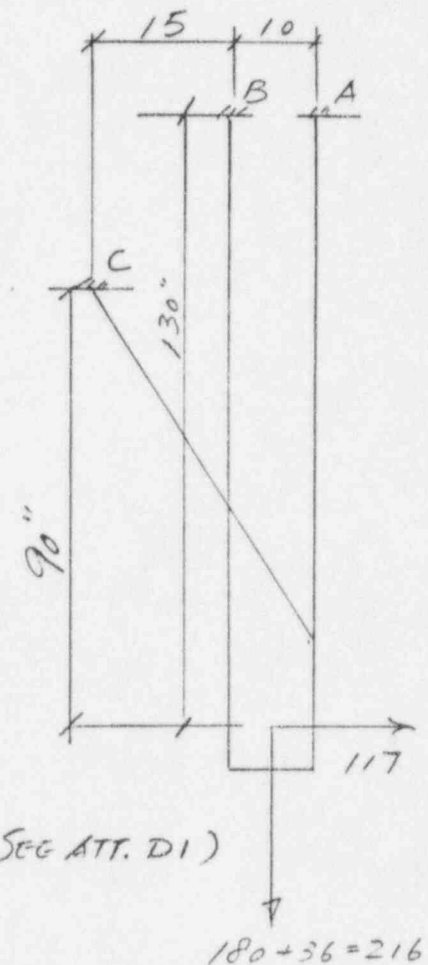
TENSION ON VERTICAL MEMBER

$$\frac{117 \times 90 + 216 \times 20}{3} = 693 \text{ lb}$$

TENSION ON BRACING

$$\frac{117 \times 130 + 216 \times 5}{20} = 815 \text{ lb}$$

SINCE THE TENSION ON BASE PLATES  
 A & B IS SMALLER THAN THE ALLOWABLE  
 FOR A SINGLE  $\frac{3}{8}$ "  $\phi$  PHILLIPS ANCHOR  
 (1240 LB/ANCH). THEREFORE, PLATES  
 A & B ARE ADEQUATE  
 SHEAR IS NEGLIGIBLE.



BASE PLATE AT C IS

8 x 5 x  $\frac{3}{4}$  WITH 2 -  $\frac{1}{2}$ "  $\phi$  HKB II HILTI BOLTS

TOTAL TENSION = 815 lb &lt; 2625 lb/BOLT. O.K.

$$\text{PLATE STRESS} = \frac{815 \times 2.5}{5 \times \frac{0.75^2}{6}} = 4347 \text{ PSI} < 27,000 \text{ PSI}$$

O.K.

SHEAR IS NEGLIGIBLE.

BASE PLATE AT C IS ADEQUATE.

BOLT SPACING VIOLATION IS NOTED FOR BASE PLATE AT C.

$$\text{ACTUAL SPACING} = (3^2 + 2.5^2)^{\frac{1}{2}} = 3.91 < 6" \text{ (REQ'D)}$$

$$\text{REDUCED ALLOW. TEN} = 2625 \times \left(\frac{3.91}{6}\right) = 1711 \text{ \#}$$

$$> 815 \text{ \#}$$

THIS SPACING VIOLATION IS ACCEPTABLE

 DECO SPEC 3071-226  
 REV J PAGE 8

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PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR

SUBJECT SYSTEM 2850-2

CALCULATION SET NO.			REV	COMP. BY	CHK'D BY
DC-5766			0	KTW	QPS
PRELIM	FINAL	VOID		DATE	DATE
	✓			5/17/96	5/22/96
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H-64 (MODIFIED) SEE ATT. D1

H-64 SUPPORT HAS SIMILAR SUPPORT CONFIGURATION TO THAT OF H-63. THE LOADS ON H-64 IS SMALLER THAN THAT ON H-63. THE BASE PLAT FOR THE ADDED EQUIP. (10x5x34) IS SIMILAR TO THAT ON H-63. THEREFORE BASE PLATES FOR H-64 ARE ADEQUATE BY COMPARISON TO H-63

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PROJECT STRUCTURAL QUALIFICATION

SUPPORT EVALUATION FOR  
SYSTEM 2850-2

CALCULATION SET NO

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REV

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CHK'D BY

PRELIM

FINAL

VOID

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JFS  
DATEK.T.W.C.  
DATE

4-9-96

5/21/96

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DATE

GM-2850-H-68

REF

BASE PLATE LOADS ARE LINEAR ADJUSTMENT OF H-59

JOINT 1

DL RATIO =  $\frac{266}{240} = 1.108$

DL + SSE

$$F_x = 140 \times \frac{162}{115} = 197 \text{ \#}$$

$$F_y = 164 \times 1.108 + 78 \times \frac{58}{156} + 874 \times \frac{162}{115} = 1442 \text{ \#}$$

$$M_z = 3985 \times \frac{162}{115} = 5614 \text{ \#} = 468 \text{ \#}$$

STRING  
ANALYSIS  
OUTPUT  
3/29/96

BASE PL : 8 x 8 x 1/2

A.B. : 4 - 3/8"  $\phi$  PHILLIPS ANCHOR.

TEN &amp; MOMENT CAPACITIES

USE CHART B IN ZONE III

$$\text{TEN. IR} = \frac{1442}{3730} + \frac{468}{955} = 0.88$$

$$\text{SHEAR IR} = \frac{197}{4 \times 690} = 0.07$$

$$\text{IR} = (0.87)^{5/3} + (0.07)^{5/3} = 0.80 < 1.0 \text{ OK}$$

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 PROJECT STRUCTURAL QUALIFICATION  
SUPPORT EVALUATION FOR  
 SUBJECT SYSTEM 28SD-2

CALCULATION SET NO.

DC-5766

PRELIM

FINAL

VOID

✓

REV

COMP. BY

CHK'D BY

0

 JFS  
 DATE  
 4-9-96

 ETW  
 DATE  
 5/1/96

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DATE

H-68 : CONT.JOINT 10

DL + SSE

$$F_x = 25 \times \left( \frac{162}{115} \right) = 35 \#$$

$$F_y = 1442 \#$$

$$M_z = 974 \times \left( \frac{162}{115} \right) = 1372 \text{ " #} = 114 \text{ ' #}$$

BASE PL. : 8x8x1/2

A.B. : 4-3/8"  $\phi$  PHILLIPS
 TEN & MOMENT CAPACITIES :  
 USE CHART C, ZONE III

BOLT TENSION :

$$I R = \frac{1442}{2110} + \frac{114}{633} = 0.86 < 1.0 \quad \text{OK}$$

SHEAR IS NEGLIGIBLE

REF

 ABM-OSIO  
 SH. 3



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PROJECT STRUCTURAL QUALIFICATION

SUBJECT SUPPORT EVALUATION FOR  
SYSTEM 2850-2

CALCULATION SET NO. DC-5766			REV 0	COMP. BY JFS	CHK'D BY ETW.
PRELIM	FINAL ✓	VOID	DATE 4-9-96	DATE 5/21/96	
SHEET 216 OF 246			DATE	DATE	
JO 76230.502					

GM-2850-H-72

DL+SSE : LINEAR ADJUSTMENT OF H-59

JOINT 1

$$F_x = 140 \times \left(\frac{147}{115}\right) = 180 \text{ \#}$$

$$F_y = 164 \times \left(\frac{113}{240}\right) + 78 \times \left(\frac{26}{156}\right) + 874 \times \left(\frac{147}{115}\right) \left(\frac{128.6}{136.6}\right) = 1142 \text{ \#}$$

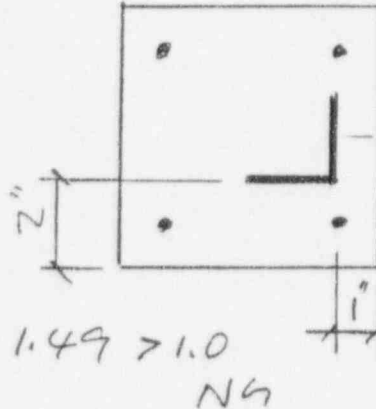
$$M_z = 3985 \times \left(\frac{147}{115}\right) \left(\frac{128.6}{136.6}\right) = 4796 \text{ \#} = 400 \text{ \#}$$

BASE PL. 6x6x 3/8

A.B. : 4-3/8" φ PHILLIPS

SUPPORT MEMBER : L2x2x 1/4

FROM BASE PL TEN-M CAPACITY  
CHART C, ZONE IV



$$\text{BOLT TEN IR} = \frac{1142}{2111} + \frac{400}{420} = 1.49 > 1.0 \quad \text{NG}$$

PER STRUDL RUN # 4/6/96, MAX. BOLT TENSION  
IS 846 # < 1240 # (ALLOWABLE), THEREFORE  
THIS BASE PLATE & ANCHOR BOLTS ARE OK

REF

STRING  
ANALYSIS  
OUTPUT  
3/29/96

ABM-0510  
SH 19

ATT. C1  
STRUDL RUN  
OUTPUT  
# 4/6/96

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CALCULATION SET NO

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REV

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DATE  
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DATE

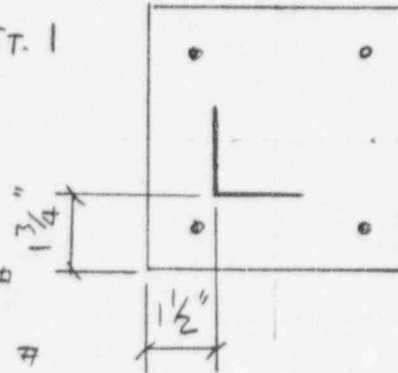
H-72 : CONTJOINT 10 B.P & A.B SAME AS JT. 1

$$F_x = 25 \times \left( \frac{147}{115} \right) = 32 \#$$

$$F_y = 1142 \#$$

$$M_z = 974 \left( \frac{147}{115} \right) \left( \frac{128.6}{136.6} \right) = 1172 \# \text{ in}$$

$$= 98 \# \text{ ft}$$



REF

ABM-USID  
SH. 19STRING  
ANALYSIS  
OUTPUT  
3/29/96

BASE PL TEN-M CAPACITY :

USE CHART C, ZONE IV

$$\text{BOLT TEN IR} = \frac{1142}{2111} + \frac{98}{420} = 0.77 < 1.0$$

SHEAR IS NEGLIGIBLE

PLATE STRESS IS WITHIN THE TEN-M CHART, OK

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SUBJECT SUPPORT EVALUATION FOR

SYSTEM 2850-4

CALCULATION SET NO

DC-5766

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JFJ

DATE

4-12-96

KTW

DATE

4/15/96

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SUPPORT MARK	JOINT NO.	BASE PL. SIZE	ANCHOR BOLT PHILLIPS	SUPPORT MEMBER	BASE PLATE LOADS			RESULT	REMARKS	
					TENSION (LBS)	MOMENT (FT-LBS)	GOVERNING SHEAR (LBS)			GOVERNING LOAD CASE
GM-2850 H-50	1	C6x10.5	4-3/8" φ	L2 1/2 x 2 1/2 x 1/4	1443	210	163	DL + SSE	PASS	COMBINED BASE PLATE
	10	C6x10.5		L2 1/2 x 2 1/2 x 1/4	-1227 (COMP)	76	23	DL + SSE	PASS	COMBINED BASE PLATE BOUNDED BY H-50
GM-2850 H-51	1	C6x10.5	4-3/8" φ	L2 1/2 x 2 1/2 x 1/4						
	10	C6x10.5		L2 1/2 x 2 1/2 x 1/4						

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PROJECT STRUCTURAL QUALIFICATION  
SUPPORT EVALUATION FOR  
SUBJECT SYSTEM 2850-4

CALCULATION SET NO DC-5766			REV 0	COMP. BY AFS	CHK'D BY L.T.W.
PRELIM	FINAL ✓	VOID		DATE 4-15-96	DATE 4/16/96
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REF

DC #2836  
REV. A  
ATTACH 5  
PAGE 10  
DWG 6M-  
2850-50  
R/O

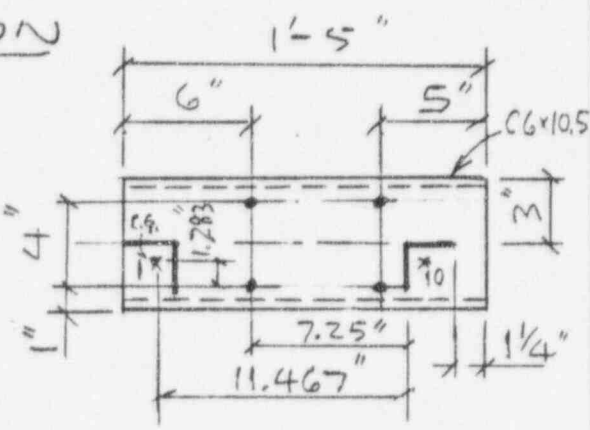
STAR DYNE  
RUN, 4/10/96  
H-50  
(ATT. B12)

STRU DL  
RUN  
BASE PL  
2850-H-50  
BASE PLATE  
DATED 4/15/96  
(ATT. C2)

H-50

BASE PLATE EVALUATION

BASE PL: C 6x10.5  
A.B.: 4 - 3/8" φ PHILLIPS  
SUPPORT MEMBER: 2 - L2 1/2 x 2 1/2 x 1/4  
DL + SSE:  
LOAD COMB #5



JOINT # 1:  
 $F_x = 163 \#$   
 $F_y = 1443 \#$  TENSION  
 $M_z = 2520 \# = 210' \#$

JOINT # 10:  
 $F_x = 23 \#$   
 $F_y = 1227 \#$  COMPRESSION  
 $M_z = 910 \# = 76' \#$

$$\text{BOLT TENSION} = \frac{[1443 \times (11.467) + (2520 + 910)]}{2 \times 7.25} + \frac{1227 \times 0.717}{2 \times 7.25} + \frac{(1443 - 1227) \times (4 - 1.283)}{2 \times 4.0}$$

$$= 1378 + 60 + 73 = 1511 \# > 1240 \# \text{ NG}$$

USE STRU DL BASE PLATE COMPUTER PROGRAM  
BOLT I R = 0.97 < 1.0 OK  
PLATE STRESS = 24.64 KSI < 27.0 KSI (ALLOW) OK

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COMPUTATION  
SHEET

CALCULATION SET NO DC - 5766			REV	COMP. BY	CHK'D BY
PRELIM	FINAL	VOID	0	ETW	E.A
	V			DATE 6/6/96	DATE 6/6/96
SHEET 220 OF 246				DATE	DATE
JO 76230.502					

PROJECT DECO FERMI 2  
SUBJECT CCHVAC' DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM  
2850-4

H-51

COMPARISON OF THE SUPPORT LOADS FOR H50 & H51 IS AS FOLLOWS:

	H50	H51	H51/H50
DL VERT	172	117	0.68
SSE VERT	44	24	0.58
SSE HORIZ.	185	129	0.70

USE RATIO 0.70 TO OBTAIN THE BOLT TENSION & SHEAR FOR H51

	H50	H51
TENSION	1209	846
SHEAR	44	31

MIN. SPACING BETWEEN  $\frac{3}{8}$ " PHILLIPS IS  $3\frac{1}{2}$ ". MIN. SPACING FOR FULL CAPACITY OF THE ANCHOR IS 4". THEREFORE, THE ADJUSTED  $\frac{3}{8}$ " PHILLIP ANCHOR CAPACITIES ARE:

TENSION  $1240 \times \frac{3.5}{4} = 1085 \text{ lb}$

SHEAR  $690 \times \frac{3.5}{4} = 604 \text{ lb}$

IR FOR H51 ARE

TENSION  $\frac{846}{1085} = 0.78$

SHEAR  $\frac{31}{604} = 0.05$

$0.78 + 0.05 = 0.83 < 1.0$  O.K.

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

DETROIT  
EDISON

221  
HY of 246

**COMPUTATION SHEET**

calculation number

DC-5766

QA Category I

change document number

EDP-28147 REV.0

Seismic Cat I

STRUCTURAL ATTACHMENT LOADING SCHEDULE

Support Number(s) GM-2850-H-73-A

Preparer J. SHIRH

Date 5-22-96

Checker K. T. W.

Date 5/23/96

JOINT NO.	SUPPORT REACTIONS						LOAD COND *
	X GLOBAL			LOCAL **			
	Fx lbs	Fy lbs	Fz lbs	Mx ft-lbs	My ft-lbs	Mz ft-lbs	
14	648	1514	0	0	0	5	DL+SSE

\* Load Condition \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

\*\* Local Coordinate System  
 SEE NEXT SHEET FOR GLOBAL COORDINATION.

DWG TO BE REVISED

10-4221-000-GM-2850-73-A

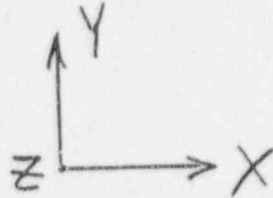
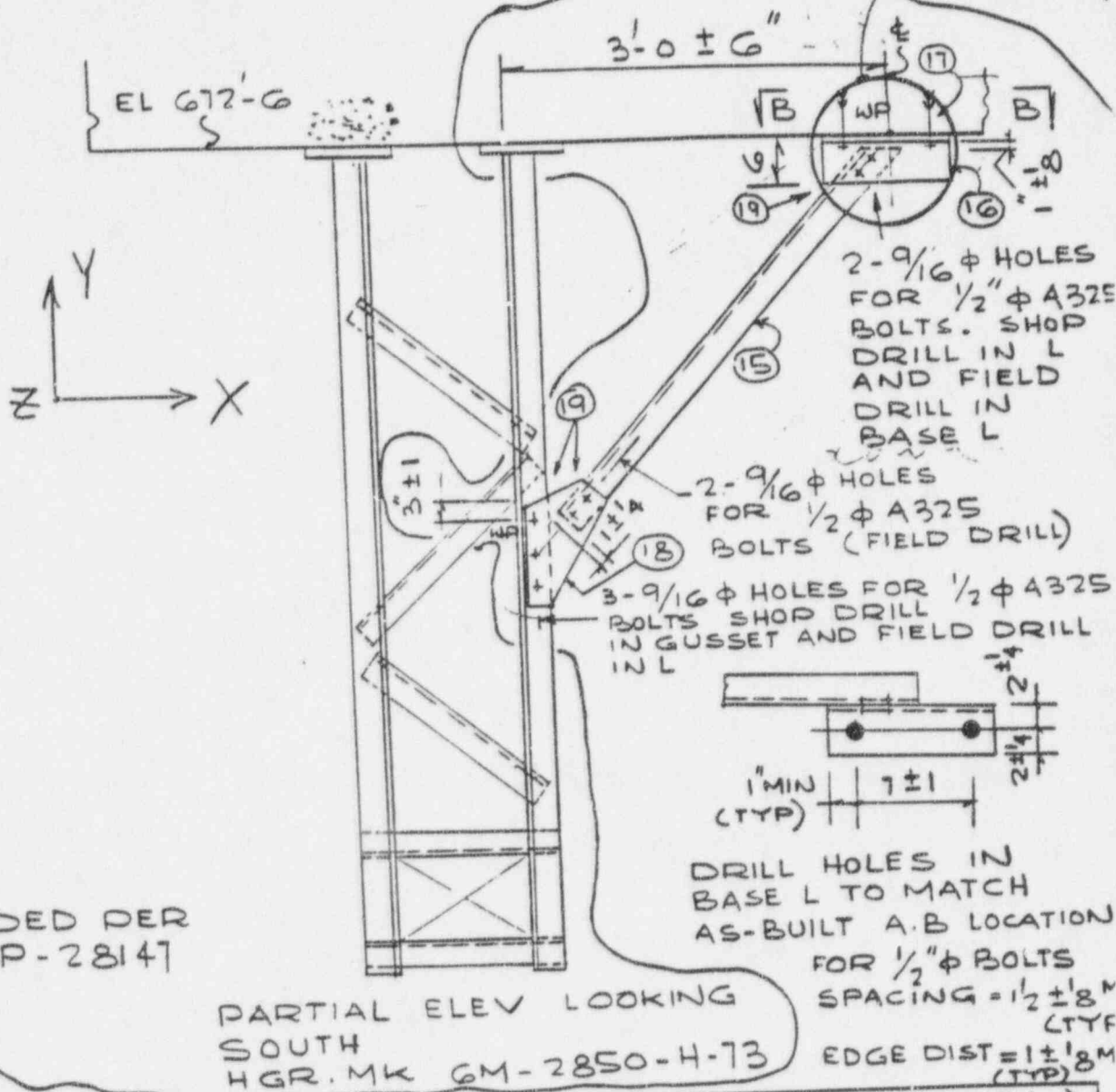
EDP-28147

Rev. 0

Index Item No. 15

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JOINT #14



ADDED PER EDP-28147

PARTIAL ELEV LOOKING SOUTH HGR. MK GM-2850-4-73

DRILL HOLES IN BASE L TO MATCH AS-BUILT A.B LOCATION FOR 1/2" φ BOLTS SPACING = 1 1/2 ± 1/8" (CTYP) EDGE DIST = 1 ± 1/8" (CTYP)

TYPE OF SUPPORT RIGID HANGER & RESTRAINT (X)			
BILL OF MATERIALS	QTY	ITEM	DESCRIPTION
	1	15	L 4x4x1/4 x 5'-0" (CUT TO FIT)
	1	16	L 6x4x1/2 x 0'-10" w/2-9/16 φ HOLES
	2	17	1/2 φ HKB II HILTI BOLTS
	1	18	3/8 GUSSET PL (CUT TO FIT)
	7	19	1/2 φ A-325 HS BOLTS w/NUT & WASHER

SIGNATURES

A) Prepared By Sign	Date	B) Checked By Sign	Date
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DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

PREPARED BY: K.T.W. 5/23/96  
CHECKED BY: E. AYAY 5/24/96

DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION

DETROIT  
 EDISON

pg<sup>223</sup> of 246

COMPUTATION SHEET

calculation number DC-5766

QA Category I

change document number EDP-28147 REV. 0

Seismic Cat I

STRUCTURAL ATTACHMENT LOADING SCHEDULE

Support Number(s) GM-2850H-77

Preparer J. SHIH

Date 5-22-96

Checker K. T. Wu

Date 5/23/96

JOINT NO.	SUPPORT REACTIONS <input checked="" type="checkbox"/> GLOBAL <input type="checkbox"/> LOCAL **						LOAD COND *
	Fx lbs	Fy lbs	Fz lbs	Mx ft-lbs	My ft-lbs	Mz ft-lbs	
15	939	859	0	0	0	31	DL+SSE

\* Load Condition \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

\*\* Local Coordinate System

SEE NEXT SHEET FOR GLOBAL COORDINATION.



DWG TO BE REVISED

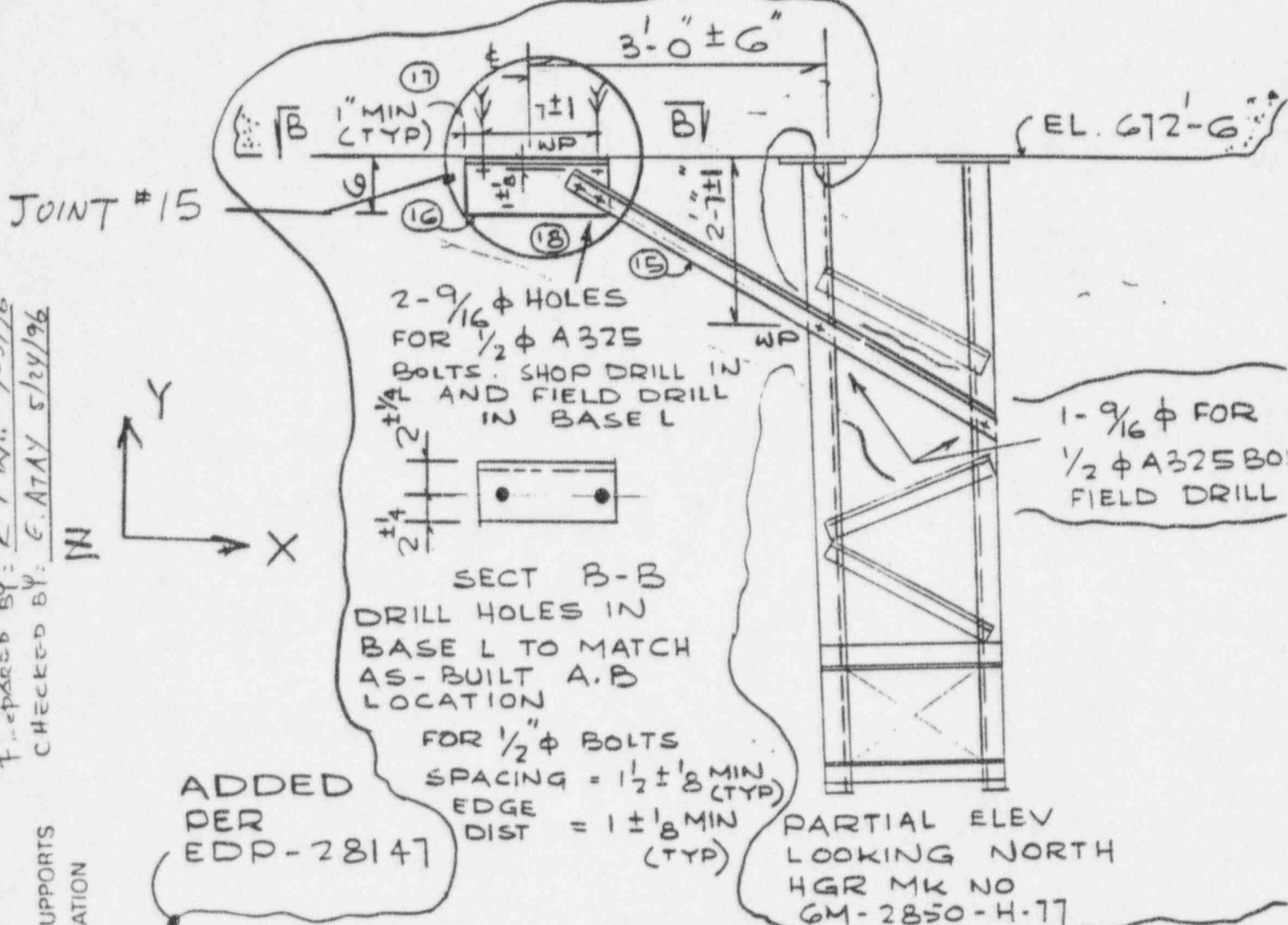
10-4221-000-GM-2850-77

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Prepared BY: K.T.W. 5/23/96  
 Checked BY: E. ATAY 5/24/96

DECO FERM 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION

TYPE OF SUPPORT: RIGID HANGER RESTRAINT (X)

BILL OF MATERIALS	QTY	ITEM	DESCRIPTION
	1	15	L 4x4x1/4 x 7'-8" (CUT TO FIT)
	1	16	L 6x4x1/2 x 10" W/2-9/16 φ HOLES
	2	17	1/2 φ HKB II HILTI BOLTS
	4	18	1/2 φ A325 BOLTS W/NUT & WASHER

SIGNATURES

A) Prepared By Sign	Date	B) Checked By Sign	Date
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DETROIT  
 EDISON

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calculation number

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QA Category I

change document number

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Seismic Cat I

STRUCTURAL ATTACHMENT LOADING SCHEDULE

Support Number(s) GM-2850-H-74-A

Preparer J. SHIEH

Date 5-22-96

Checker KTWu

Date 5/23/96

JOINT NO.	SUPPORT REACTIONS <u>X</u> GLOBAL LOCAL **						LOAD COND *
	Fx lbs	Fy lbs	Fz lbs	Mx ft-lbs	My ft-lbs	Mz ft-lbs	
C	391	0	0	0	0	0	DL+SSE

\* Load Condition

\_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

\*\* Local Coordinate System

SEE NEXT SHEET FOR GLOBAL COORDINATION.

DWG TO BE REVISED

10-4221-000-GM-2850-74 A

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ADDED PER  
EDP-28147

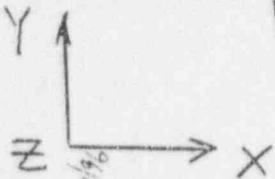
EL 672-6

NOTE: DRILL HOLES IN BASE  $\phi$  TO MATCH AS-BUILT A.B LOCATION

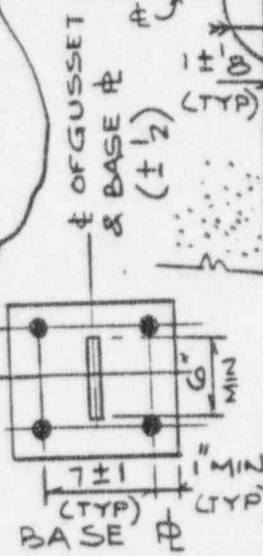
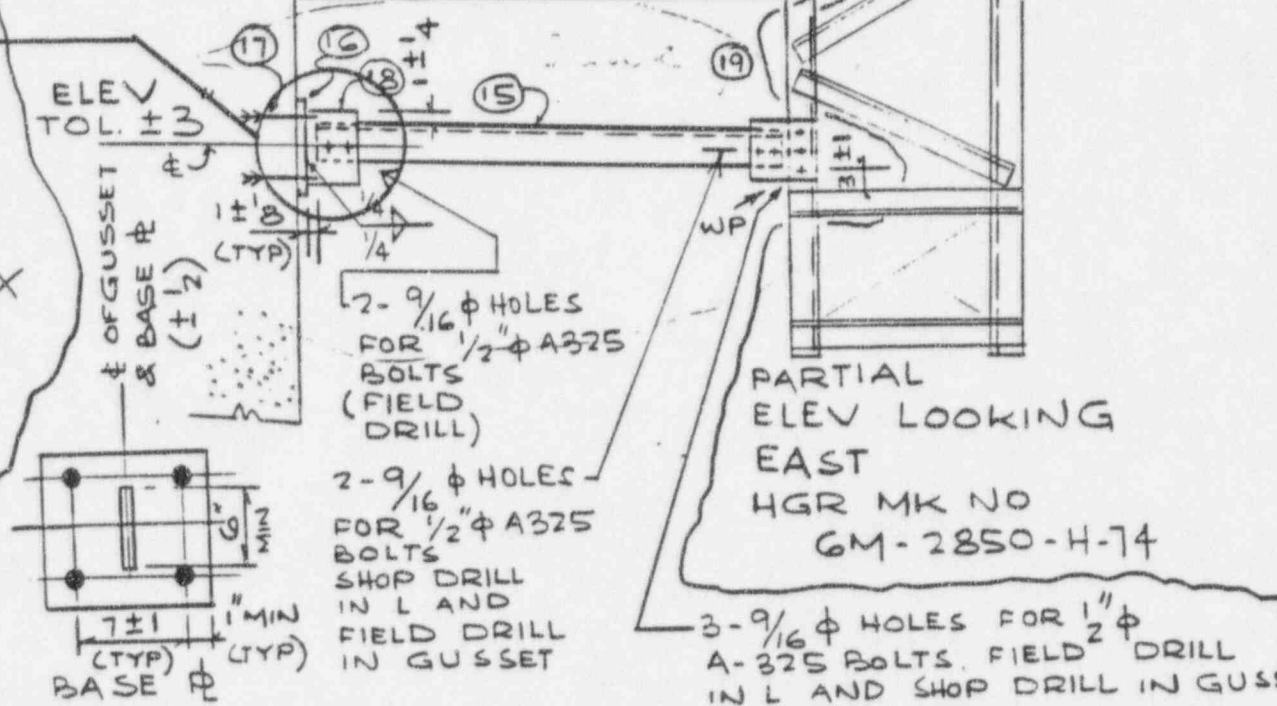
FOR  $\frac{1}{2}$ "  $\phi$  BOLTS  
SPACING  $\cdot \frac{1}{2} \pm \frac{1}{8}$  MIN (TYP)  
EDGE DIST  $\cdot 1 \pm \frac{1}{8}$  MIN (TYP)  
8'-8 $\frac{1}{2}$ " (REF)

JOINT C

ELEV TOL.  $\pm 3$



PREPARED BY: K. W. Fisher  
CHECKED BY: E. ATY / s. h. / 16



2-9/16  $\phi$  HOLES FOR  $\frac{1}{2}$ "  $\phi$  A325 BOLTS (FIELD DRILL)  
2-9/16  $\phi$  HOLES FOR  $\frac{1}{2}$ "  $\phi$  A325 BOLTS SHOP DRILL IN L AND FIELD DRILL IN GUSSET

PARTIAL ELEV LOOKING EAST  
HGR MK NO  
GM-2850-H-74

3-9/16  $\phi$  HOLES FOR 1"  $\phi$  A-325 BOLTS. FIELD DRILL IN L AND SHOP DRILL IN GUSSET

TYPE OF SUPPORT: RIGID HANGER & RESTRAINT (2)

BILL OF MATERIALS	QTY	ITEM	DESCRIPTION
	1	15	L 4x4x1/4 x 8'-9 (CUT TO FIT)
	1	16	3/4 x 10 x 0'-10 $\phi$ W/ 4-9/16 $\phi$ HOLES
	4	17	1/2" $\phi$ HKB II HILTI BOLTS
	2	18	3/8" GUSSET $\phi$ (CUT TO FIT)
	7	19	1/2" $\phi$ A325 HS BOLTS W/NUT & WASHER

DECO FERM 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

SIGNATURES			
A) Prepared By	Sign	Date	B) Checked By
			Sign
			Date

DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION

DETROIT  
 EDISON

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QA Category I

change document number EDP-28147, REV 0

Seismic Cat I

STRUCTURAL ATTACHMENT LOADING SCHEDULE

Support Number(s) 6M-2850-H-76

Preparer J. SHIEH

Date 5-22-96

Checker K. T. W.

Date 5/23/96

JOINT NO.	SUPPORT REACTIONS <u>X</u> GLOBAL <u>  </u> LOCAL **						LOAD COND *
	Fx lbs	Fy lbs	Fz lbs	Mx ft-lbs	My ft-lbs	Mz ft-lbs	
C	409	0	0	0	0	0	DL+SSE

\* Load Condition

\_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

\*\* Local Coordinate System

SEE NEXT SHEET FOR GLOBAL COORDINATION.

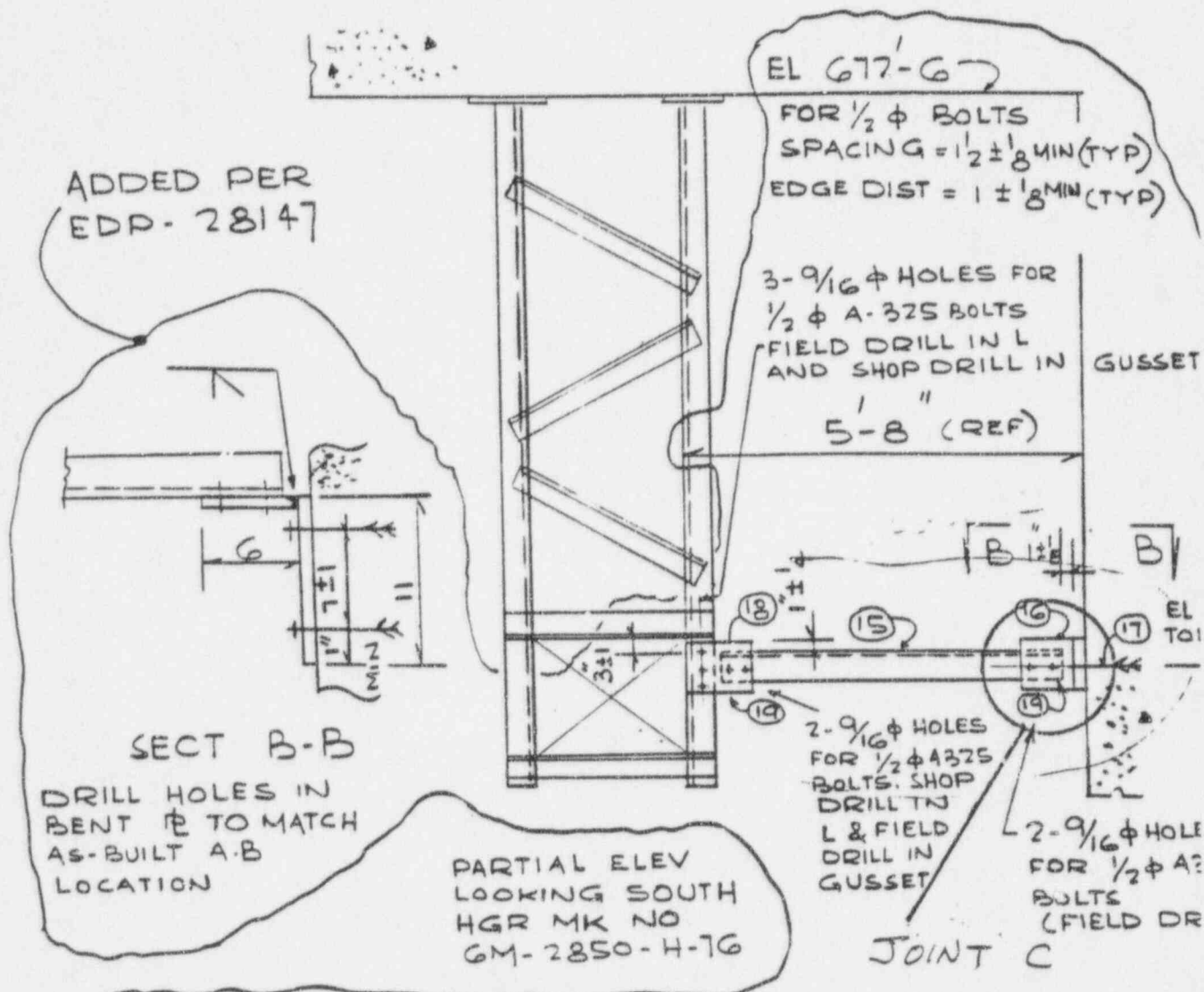
DWG TO BE REVISED

10-4221-000-GM-2850-76

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PREPARED BY: ETW... 23/96  
 CHECKED BY: EATAY SHU...

DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION



ADDED PER EDP-28147

SECT B-B  
 DRILL HOLES IN BENT  $\Phi$  TO MATCH AS-BUILT A-B LOCATION

PARTIAL ELEV LOOKING SOUTH  
 HGR MK NO GM-2850-H-76

JOINT C

TYPE OF SUPPORT: RIGID HANGER & RESTRAINT X

BILL OF MATERIALS	QTY	ITEM	DESCRIPTION
	1	15	L 3x3x1/4 x 5'-10 (CUT TO FIT)
	1	16	BUILT UP 11x6x3/4 x 0'-5 LONG W/ 2- 9/16 phi HOLES ANGLE
	2	17	1/2 phi HKB II HILTI BOLTS
	1	18	3/8 THK GUSSET $\Phi$
	7	19	1/2" phi A325 HS BOLTS W/ NUT & WASHER

SIGNATURES			
A) Prepared By	Sign	Date	
B) Checked By	Sign	Date	

DETROIT  
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COMPUTATION SHEET

calculation number DC-5766

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change document number EDP-28147 REV 0

Seismic Cat I

STRUCTURAL ATTACHMENT LOADING SCHEDULE

Support Number(s) GM-2850-H-60

Preparer J. SHILZH

Date 5-22-96

Checker K.T. v.l.

Date 5/24/96

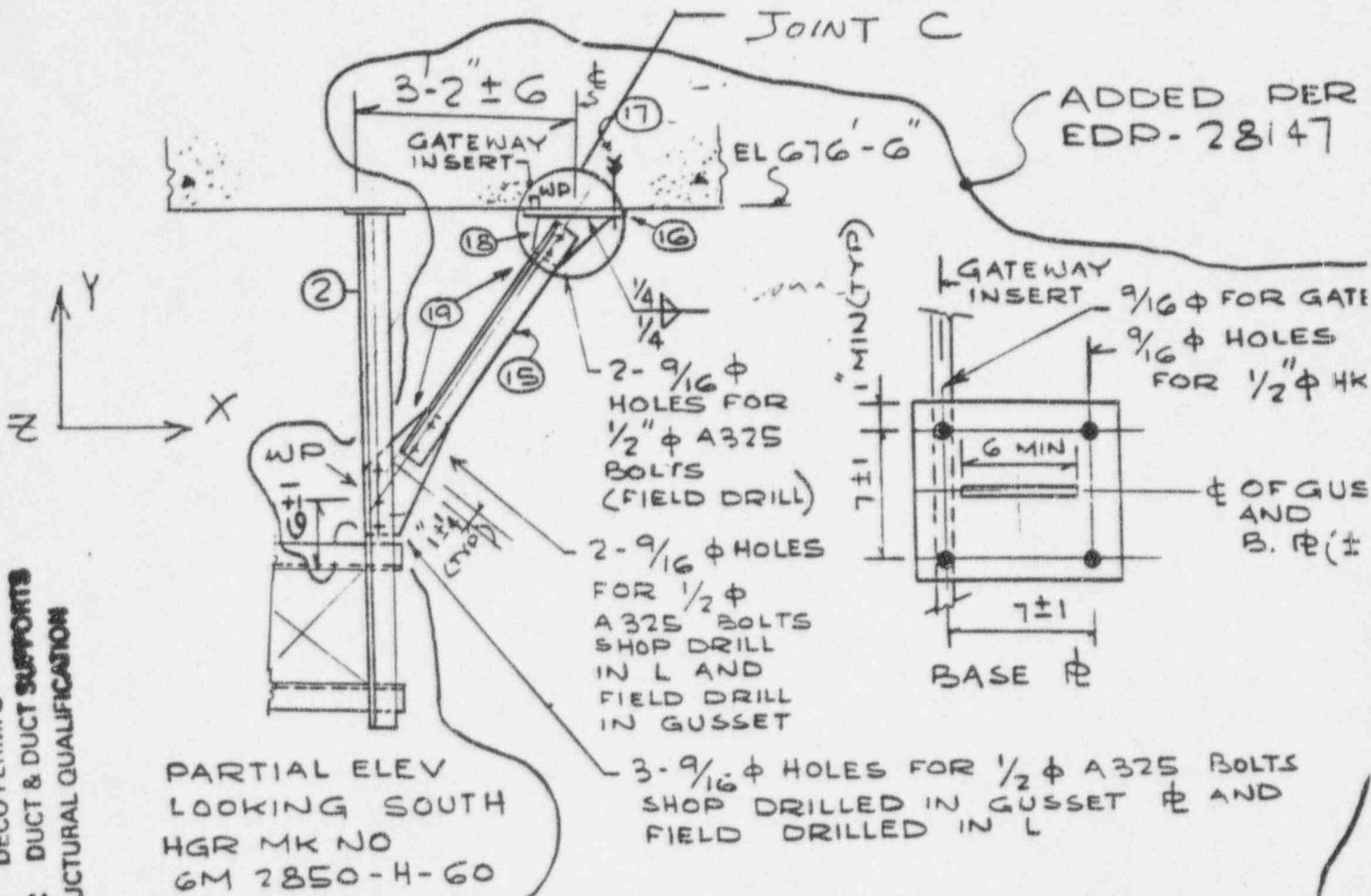
JOINT NO.	SUPPORT REACTIONS <u>X</u> GLOBAL <u>LOCAL</u> **						LOAD COND *
	Fx lbs	Fy lbs	Fz lbs	Mx ft-lbs	My ft-lbs	Mz ft-lbs	
C	99	382	0	0	0	0	DL+SSE

\* Load Condition \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

\*\* Local Coordinate System  
 SEE NEXT SHEET FOR GLOBAL COORDINATION.

DWG TO BE REVISED  
10-4221-000-GM-2850-60

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PARTIAL ELEV  
LOOKING SOUTH  
HGR MK NO  
GM 2850-H-60

NOTE: DRILL HOLES IN BASE PLATE TO MATCH A.B AS-BUILT LOCATION.  
BOLT SPACING FOR 1/2" φ BOLT = 1 1/2" ± 1/8" MIN(TYP)  
EDGE DIST. FOR 1/2" φ BOLT = 1" ± 1/8" MIN(TYP)

TYPE OF SUPPORT X-RESTRAINT & HANGER			
BILL OF MATERIALS	QTY	ITEM	DESCRIPTION
	1	15	L 4x4x1/4 x 10'-4" (CUT TO FIT)
	1	16	3/4" x 10" x 0'-10" PL W/4 - 9/16" φ HOLES
	2	17	1/2" φ HKB II HILTI BOLTS
	2	18	3/8" THK GUSSET PL (CUT TO FIT)
	7	19	1/2" φ A325 HS BOLTS W/NUT & WASHER
	2	20	1/2" φ BOLT ASSEMBLY FOR GATEWAY INSERT

DECO FERMI 2  
DUCT & DUCT SUPPORTS  
CCHVAC  
STRUCTURAL QUALIFICATION

Prepared by: K.T.W. 5/24/96  
Checked by: E. ATAY 5/24/96

SIGNATURES			
A) Prepared By	Date	B) Checked By	Date
Sign		Sign	

DETROIT  
 EDISON

COMPUTATION SHEET

calculation number DC-5766

QA Category I

change document number EDP-28147 REV 0

Seismic Cat I

STRUCTURAL ATTACHMENT LOADING SCHEDULE

Support Number(s) GM-2850-H-61

Preparer J. SHIEN

Date 5-22-96

Checker K T W

Date 5/24/96

JOINT NO.	SUPPORT REACTIONS <u>X</u> GLOBAL <u>  </u> LOCAL **						LOAD COND *
	Fx lbs	Fy lbs	Fz lbs	Mx ft-lbs	My ft-lbs	Mz ft-lbs	
C	97	456	0	0	0	0	DL+SSE

\* Load Condition \_\_\_\_\_  
 \_\_\_\_\_  
 \_\_\_\_\_

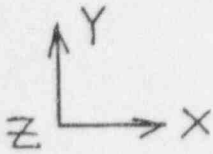
\*\* Local Coordinate System

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DWG TO BE REVISED  
10-4221-000-GM-2850-61

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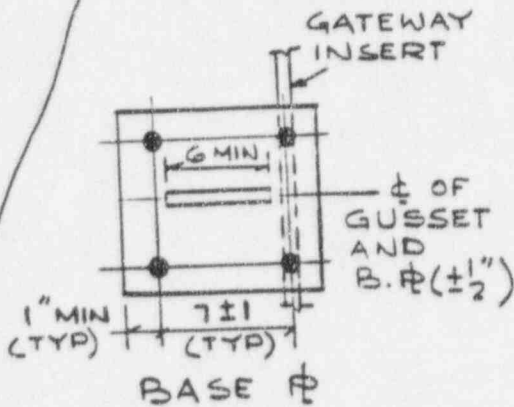
ADDED  
PER  
EDP-28147

JOINT C

2-9/16 φ HOLES  
FOR 1/2 φ A325  
BOLTS  
(FIELD DRILL)

2-9/16 φ HOLES  
FOR 1/2 φ A325 BOLTS  
SHOP DRILL IN L  
AND FIELD DRILL  
IN GUSSET

3-9/16 φ HOLES FOR  
1/2 φ A325 BOLTS  
SHOP DRILLED IN  
GUSSET φ AND  
FIELD DRILLED  
IN L



NOTE: DRILL HOLES IN BASE φ  
TO MATCH A.B AS-BUILT  
LOCATION.  
BOLT SPACING FOR 1/2 φ BOLT = 1 1/2 ± 1/8 MIN (TYP)  
EDGE DIST. FOR 1/2 φ BOLT = 1" ± 1/8 MIN (TYP)

PARTIAL ELEV  
LOOKING SOUTH  
HGR MK NO GM  
2850-H-61

TYPE OF SUPPORT X-RESTRAINT & HANGER

BILL OF MATERIALS	QTY	ITEM	DESCRIPTION
	1	15	L 4x4x14 x 10'-6 (CUT TO FIT)
	1	16	3/4 x 10 x 0'-10" w/4 - 9/16 φ HOLES
	2	17	1/2" φ HKB II HILTI BOLTS
	2	18	3/8 THK GUSSET φ (CUT TO FIT)
	7	19	1/2" φ A-325 HS BOLTS W/NUT & WASHER
	2	20	1/2" φ BOLT ASSEMBLY FOR GATEWAY INSERT

Prepared BY: K.T. Wu 5/30/96  
Checked BY: E. ATAY 5/24/96

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

SIGNATURES

A) Prepared By Sign	Date	B) Checked By Sign	Date
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DETROIT  
EDISON

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION  
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calculation number DC-5766

QA Category I

change document number EDP-28147 REV 0

Seismic Cat I

STRUCTURAL ATTACHMENT LOADING SCHEDULE

Support Number(s) GM-2850-H-63

Preparer J. SHIEH

Date 5-22-96

Checker K T Wu

Date 5/24/96

JOINT NO.	SUPPORT REACTIONS						LOAD COND *
	X GLOBAL			LOCAL **			
	Fx lbs	Fy lbs	Fz lbs	Mx ft-lbs	My ft-lbs	Mz ft-lbs	
C	117	815	0	0	0	0	DL+SSE

\* Load Condition \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

\*\* Local Coordinate System  
SEE NEXT SHEET FOR GLOBAL COORDINATION,

DWG TO BE REVISED

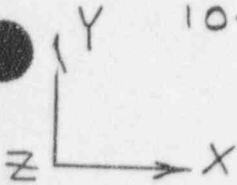
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BOLT SPACING FOR 1/2" φ BOLT = 1 1/2 ± 1/8" MIN (TYP)

EDGE DIST FOR 1/2" φ BOLT = 1 ± 1/8" MIN (TYP)

EXISTG ANCHORS DIM. ACROSS FLATS = 9/16"

φ OF GUSSET & B.φ (± 1/2")

BASE φ

DRILL HOLES IN B.φ TO MATCH A.B AS-BUILT LOCATION

2-9/16" φ HOLES FOR A325 BOLTS SHOP DRILL IN L & FIELD DRILL IN GUSSET

ADDED PER EDP-28147

1-9/16" φ HOLE FOR A 325 BOLT FIELD DRILL (TYP)

PARTIAL ELEV LOOKING SOUTH HGR MK NO GM 2850-H-63

TYPE OF SUPPORT X RESTRAINT & HANGER

BILL OF MATERIALS	QTY	ITEM	DESCRIPTION
	1	15	L 3x3x1/4 x 7'-0 (CUT TO FIT)
	1	16	3/4 x 5 x 0'-8 W/2 - 9/16 φ HOLES
	2	17	1/2" φ HKB II HILTI BOLTS
	1	18	3/8 GUSSET φ
	4	19	1/2" φ A 325 HE BOLTS W/NUT & WASHER

SIGNATURES

A) Prepared By  
Sign

Date

B) Checked By  
Sign

Date

PREPARED BY: K.T. W. Spaul/96  
 CHECKED BY: E. ATAY 5/24/96

DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 STRUCTURAL QUALIFICATION

DETROIT  
EDISON

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION  
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calculation number DC-5766

QA Category I

change document number EDP-28147 REV 0

Seismic Cat I

STRUCTURAL ATTACHMENT LOADING SCHEDULE

Support Number(s) GM-2850-H-64

Preparer J. SHIEH

Date 5-22-96

Checker K T WU

Date 5/24/96

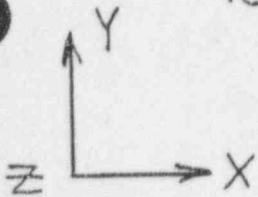
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	Px lbs	Fy lbs	Fz lbs	Mx ft-lbs	My ft-lbs	Mz ft-lbs	
C	117	815	0	0	0	0	DL+SSE

\* Load Condition \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

\*\* Local Coordinate System  
*SEE NEXT SHEET FOR GLOBAL COORDINATION.*

DWG TO BE REVISED  
10-4221-000-GM-2850-64

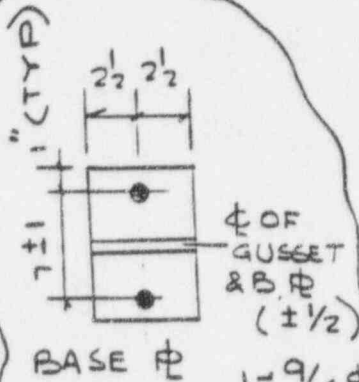
EDP-28147	Rev. 0
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Prepared by: K.T.W. 5/24/96  
Checked by: E. ATAY 5/24/96

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

ADDED PER  
EDP-28147

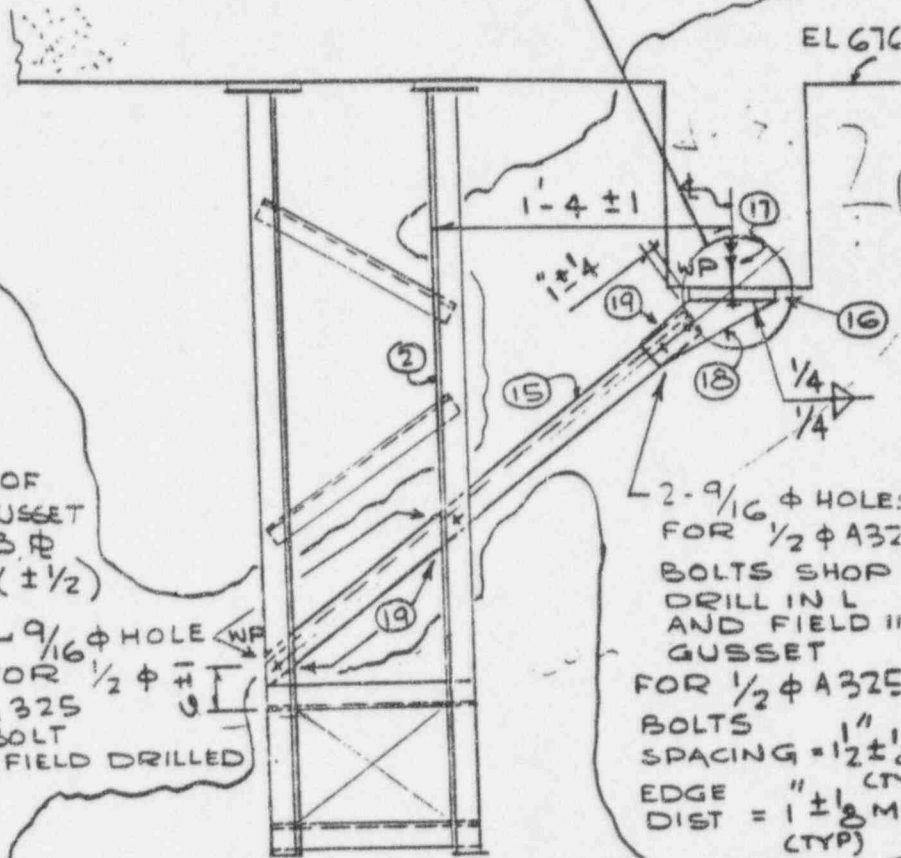


1-9/16" HOLE FOR 1/2" A325 BOLT (FIELD DRILLED)

DRILL HOLES IN B. P. TO MATCH AS-BUILT A.B LOCATION

PARTIAL ELEV  
LOOKING NORTH  
HGR MK NO  
GM 2850-H-64

JOINT C



2-9/16" HOLE FOR 1/2" A325 BOLTS SHOP DRILL IN L AND FIELD II GUSSET FOR 1/2" A325 BOLTS SPACING = 1 1/2" ± 1/8" CT EDGE DIST = 1" ± 1/8" M (TYP)

TYPE OF SUPPORT X-RESTRAINT & HANGER

BILL OF MATERIALS	QTY	ITEM	DESCRIPTION
	1	15	L2x2x3/8 x 7'-0 (CUT TO FIT)
	1	16	3/4 x 5 x 0'-10" PL WITH 2-9/16" HOLES
	2	17	1/2" φ HKB II HILTI BOLTS
	1	18	3/8" THK GUSSET PL (CUT TO FIT)
	4	19	1/2" φ A-325 HS BOLTS W/NUT & WASHER

SIGNATURES

A) Prepared By Sign	Date	B) Checked By Sign	Date
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**Raytheon**  
 Engineers & Constructors

 GENERAL  
 COMPUTATION  
 SHEET

CALCULATION SET NO.			REV	COMP. BY	CHK'D BY
DC-5766			0	R. Law	E. A.
PRELIM	FINAL	VOID		DATE	DATE
	X			6/7/96	6/7/96
SHEET 237 OF 246				DATE	DATE
J.O. 76230.502					

 PROJECT DECO FERMI 2  
 CCHVAC DUCT & DUCT SUPPORTS  
 SUBJECT STRUCTURAL QUALIFICATION

## (D) Modifications

## 1. System 2850-1

The string analysis of system 2850-1 was performed with the result that member overstresses occurred in hangers 6M-2850-H-74, H-76, H-80 and H-81.

To relieve these overstresses, the first proposed modification was one where braces were added to hangers H-74 and H-81. The first system rerun was with the braces at the two hangers. The lateral stiffness at hangers H-74 and H-81 were taken to be approximately 20 percent of the stiffness in the vertical direction. This approximation was proven to be acceptable after performing a stiffness evaluation of hanger 6M-2850-H-74 with a bracing member added to it. The results of the initial system rerun indicated that three hangers, H-74, H-76 & H-81, would require a hanger member modification and four hangers, H-73, H-75, H-76 & H-77 would require anchorage modification.

Inspection in the field indicated that modification of hangers H-73, H-74, H-76, H-77 & H-81 were feasible with the addition of braces to strengthen the supports. (The bracing arrangements are included in EDP-28147). The system was reanalyzed a second time by increasing the stiffnesses at hangers H-74, H-76, H-77 & H-81 to reflect the addition of the bracing members. With the results of the string analysis, evaluation of the initial six problem hangers was performed. All six of the hangers, H-73, H-74, H-75, H-76, H-77 and H-81 were found to be structurally acceptable.

## 2. System 2850-2

The string analysis of system 2850-2 was performed with the result that member overstresses occurred in hangers 6M-2850-H-60, H-61, H-63 and H-64.

To relieve these overstresses, the system was rerun with a proposed modification wherein braces would be added to hangers H-61 and H-64. Therefore the lateral stiffness at hangers H-61 and H-64 was increased to account for the addition of braces to the hangers. The lateral stiffness of the hangers were taken to be approximately 20 percent of the stiffness in the vertical direction. (This approximation was proven to be acceptable with the results of the stiffness evaluation of hanger 6M-2850-H-74, in system 2850-1, when a bracing member was added to it.) The results of the reanalysis of the system indicated that supports H-60 thru H-64 had anchorage overstresses. To relieve these overstresses braces were added to supports H-60, H-61, H-63 and H-64. Inspection in the field indicated that addition of bracing members to four hangers (H-60, H-61, H-63 and H-64) to strengthen the supports were feasible, and the bracing arrangements are included in EDP-28147. The system analysis was not rerun to account for the additional braces at supports H-60 and H-63 because it was expected that it would cause less amplification on the system seismic responses. With the addition of the braces on the four supports, all five supports and their anchorages were reanalyzed and were found to be structurally acceptable.

## 3. System 2850-4

The string analysis of system 2850-4 was performed with the result that the existing duct and supports are acceptable.

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PROJECT DECO FERMI 2

SUBJECT ACHVAC : DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

(E) ANCHOR DESIGN CHECKLISTS

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PROJECT DECO FERMI 2  
SUBJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM

(E) ANCHOR DESIGN CHECKLISTS 2850-4

C.4.1 Checklist for Anchor Design

- Design Discipline:  Mechanical  Controls  
 Electrical  Structural  
CM-2850-
- Anchors used for support No. H-50 Other Structure   
Specify \_\_\_\_\_
- Classification: QA Level  1  Non-Q  
Seismic Cat.:  I  II/I  None
- Anchor Size 3/8" If different sizes, specify all
- Anchor Type:  Hilti Kwik Bolt II  
 Other, Specify RED HEAD SELF DRILLING (PHILLIPS)
- Actual maximum tension 1209 (lb) SEE ATT C2 PP 31  
Allowable tension 1240 (lb)
- Actual maximum shear 44 (lb)  
Allowable shear 690 (lb)
- Shear-tension interaction less than 1.0:  Yes  No  
with exponential power equal to  1.0  5/3
- Actual minimum spacing between anchors 4 (in) is more than  
the minimum spacing required between anchors  Yes  No  
(in the same attachment and between the adjacent attachments)  
If no, state the minimum spacing required: N/A (in), and fill  
out "Anchor spacing Violation, Acceptance Documentation."
- Actual minimum edge distance from the edge of concrete N/A (in)  
is more than the minimum edge distance required to obtain  
maximum loads.  Yes  No If no, state the edge distance  
required and fill out "Edge Distance Violation, Acceptance  
Criteria."

Prepared by E. ATAY

Date 6/6/96

Checked by K. T. W.

Date 6/6/96



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PROJECT DECO FERMI 2  
SUBJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM

(E) Anchor Design Checklist 2850-4  
C.4.1 Checklist for Anchor Design

- Design Discipline:  Mechanical  Controls  
 Electrical  Structural
- Anchors used for support No. 6M-2850 H-51 Other Structure   
Specify \_\_\_\_\_
- Classification: QA Level  1  Non-Q  
Seismic Cat.:  I  II/I  None
- Anchor Size 3/8"  $\phi$  If different sizes, specify all
- Anchor Type:  Hilti Kwik Bolt II  
 Other, Specify RED HEAD SELF DRILLING
- Actual maximum tension 846 (lb)  
Allowable tension 1085 (lb) } SEE CALC. SH.
- Actual maximum shear 31 (lb)  
Allowable shear 604 (lb) }
- Shear-tension interaction less than 1.0:  Yes  No  
with exponential power equal to  1.0  5/3
- Actual minimum spacing between anchors 3 1/2 (in) is more than  
the minimum spacing required between anchors  Yes  No  
(in the same attachment and between the adjacent attachments)  
If no, state the minimum spacing required: 4 (in), and fill  
out "Anchor spacing Violation, Acceptance Documentation."
- Actual minimum edge distance from the edge of concrete NA (in)  
is more than the minimum edge distance required to obtain  
maximum loads.  Yes  No If no, state the edge distance  
required and fill out "Edge Distance Violation, Acceptance  
Criteria."

Prepared by KTW  
Checked by E. ATAY

Date 6/6/96  
Date 6/6/96

DECO FERMI 2  
CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION

SYSTEM  
2850-4

Specification No. 3071-226  
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C.4.2 EXPANSION ANCHOR SPACING AND EDGE DISTANCE VIOLATION  
ACCEPTANCE DOCUMENTATION

Reporting Document: ( ) ABN \_\_\_\_\_ ( ) ECR \_\_\_\_\_  
(X) ABM-0560

Description of Violation:

Bolt Dia. 3/8"  $\phi$   
Min'm. spacing req'd. 4"  
Min'm. edge dist. req'd. 3"  
Actual Spacing 3 1/2"  
Actual Edge Distance N/A

Affected Items and Corresponding Design Documents for the Interaction

( ) C: Controls \_\_\_\_\_ ( ) I ( ) II/I ( ) N DC# \_\_\_\_\_  
( ) E: Electrical \_\_\_\_\_ ( ) I ( ) II/I ( ) N DC# \_\_\_\_\_  
( ) M: Mechanical \_\_\_\_\_ ( ) I ( ) II/I ( ) N DC# \_\_\_\_\_  
(X) S: Structural \_\_\_\_\_ (X) I ( ) II/I ( ) N DC# \_\_\_\_\_

Acceptance (of violation):

( ) This Violation has been evaluated generically "severe case simulation" DC-3200 - Case No. \_\_\_\_\_  
( ) Per attached Computations  
(X) SEE CALC. SHEET

Results (of evaluation):

(X) This violation has no adverse effect on the load carrying capability of the anchors  
( ) \_\_\_\_\_

Comments: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

Prepared by K T Wa \_\_\_\_\_ Date 6/6/96  
Checked by E. ATAY \_\_\_\_\_ Date 6/6/96

**Raytheon**  
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CALCULATION SET NO.

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PROJECT DECO FERMI 2

SUBJECT CCHVAC DUCT & DUCT SUPPORTS  
STRUCTURAL QUALIFICATION SYSTEM

2850-1

C.4.1 Checklist for Anchor Design

- Design Discipline:  Mechanical  Controls  
 Electrical  Structural
- Anchors used for Support No. H-73-A Other Structure   
H-73A ENVELOPS H-77, H-74-A, H-76, H-60, H-61, H-63, H-64  
Specify \_\_\_\_\_
- Classification: QA Level  1  Non-Q  
Seismic Cat.:  I  II/I  None
- Anchor Size  $\frac{1}{2} \phi$  If different sizes, specify all
- Anchor Type:  Hilti Kwik Bolt II  
 Other, Specify \_\_\_\_\_
- Actual maximum tension 765 (lb)  
Allowable tension 2623 (lb)
- Actual maximum shear 324 (lb)  
Allowable shear 1500 (lb)
- Shear-tension interaction less than 1.0:  Yes  No  
with exponential power equal to  1.0  5/3
- Actual minimum spacing between anchors 6 (in) is more than  
the minimum spacing required between anchors  Yes  No  
(in the same attachment and between the adjacent attachments)  
If no, state the minimum spacing required: \_\_\_\_\_ (in), and fill  
out "Anchor spacing Violation, Acceptance Documentation."
- Actual minimum edge distance from the edge of concrete N/A (in)  
is more than the minimum edge distance required to obtain  
maximum loads.  Yes  No If no, state the edge distance  
required and fill out "Edge Distance Violation, Acceptance  
Criteria."

Prepared by KTW

Date 6/7/96

Checked by E. ATAY

Date 6/7/96

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C.4.1 Checklist for Anchor Design

1. Design Discipline:  Mechanical  Controls  
 Electrical  Structural
2. Anchors used for Support No. 6M-2850 - H-57 Other Structure   
PLATES 'A' & 'B' Specify \_\_\_\_\_
3. Classification: QA Level  1  Non-Q  
Seismic Cat.:  I  II/I  None
4. Anchor Size  $\frac{1}{2} \phi$  If different sizes, specify all
5. Anchor Type:  Hilti Kwik Bolt II  
 Other, Specify RED HEAD SELF DRILLING ANCHOR
6. Actual maximum tension \_\_\_\_\_ (lb)  
Allowable tension \_\_\_\_\_ (lb)
7. Actual maximum shear \_\_\_\_\_ (lb)  
Allowable shear \_\_\_\_\_ (lb)
8. Shear-tension interaction less than 1.0:  Yes  No  
with exponential power equal to  1.0  5/3
9. Actual minimum spacing between anchors 2 1/2 (in) is more than  
the minimum spacing required between anchors  Yes  No  
(in the same attachment and between the adjacent attachments)  
If no, state the minimum spacing required: 5 (in), and fill  
out "Anchor spacing Violation, Acceptance Documentation."
10. Actual minimum edge distance from the edge of concrete NA (in)  
is more than the minimum edge distance required to obtain  
maximum loads.  Yes  No If no, state the edge distance  
required and fill out "Edge Distance Violation, Acceptance  
Criteria."

SEE CALC. SHEET 156

Prepared by K T Wu

Date 6/7/96

Checked by E. ATAY

Date 6/7/96

Specification No. 3071-226  
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C.4.2 EXPANSION ANCHOR SPACING AND EDGE DISTANCE VIOLATION  
ACCEPTANCE DOCUMENTATION

Reporting Document: ( ) ABN \_\_\_\_\_ ( ) ECR \_\_\_\_\_  
(X) DCN 10881 REV. A

Description of Violation:

Bolt Dia. 1/2" φ  
Min'm. spacing req'd. 5"  
Min'm. edge dist. req'd. 3 1/2"  
Actual Spacing 2 1/4"  
Actual Edge Distance NA

Affected Items and Corresponding Design Documents for the Interaction

( ) C: Controls \_\_\_\_\_ ( ) I ( ) II/I ( ) N DC# \_\_\_\_\_  
( ) E: Electrical \_\_\_\_\_ ( ) I ( ) II/I ( ) N DC# \_\_\_\_\_  
( ) M: Mechanical \_\_\_\_\_ ( ) I ( ) II/I ( ) N DC# \_\_\_\_\_  
(X) S: Structural \_\_\_\_\_ ( ) I ( ) II/I ( ) N DC# \_\_\_\_\_

Acceptance (of violation):

( ) This Violation has been evaluated generically "severe case simulation" DC-3200 - Case No. \_\_\_\_\_  
( ) Per attached Computations  
(X) SEE CALCULATION SHEET 156

Results (of evaluation):

(X) This violation has no adverse effect on the load carrying capability of the anchors  
( ) \_\_\_\_\_

Comments: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

Prepared by KTW Date 6/7/96  
Checked by E. ATAY Date 6/7/96

Specification No. 3071-226  
Revision J  
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C.4.1 Checklist for Anchor Design

1. Design Discipline:  Mechanical  Controls  
 Electrical  Structural
2. Anchors used for Support No. GM-2850 - H-57 Other Structure   
PLATE 'A' & 'B' Specify \_\_\_\_\_
3. Classification: QA Level  1  Non-Q  
Seismic Cat.:  I  II/I  None
4. Anchor Size 5/8" If different sizes, specify all
5. Anchor Type:  Hilti Kwik Bolt II  
 Other, Specify RED HEAD WEDGE ANCHOR
6. Actual maximum tension 2054 (lb)  
Allowable tension 2858 (lb)
7. Actual maximum shear NEGLECTIBLE (lb)  
Allowable shear 3078 (lb)
8. Shear-tension interaction less than 1.0:  Yes  No  
with exponential power equal to  1.0  5/3
9. Actual minimum spacing between anchors 3 3/4 (in) is more than  
the minimum spacing required between anchors  Yes  No  
(in the same attachment and between the adjacent attachments)  
If no, state the minimum spacing required: 4.69\* (in), and fill  
out "Anchor spacing Violation, Acceptance Documentation."
10. Actual minimum edge distance from the edge of concrete NA (in)  
is more than the minimum edge distance required to obtain  
maximum loads.  Yes  No If no, state the edge distance  
required and fill out "Edge Distance Violation, Acceptance  
Criteria."

Prepared by K T W

Date 6/7/96

Checked by E. ATAY

Date 6/7/96

\* SEE CALCULATION SHEET 156

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C.4.2 EXPANSION ANCHOR SPACING AND EDGE DISTANCE VIOLATION  
ACCEPTANCE DOCUMENTATION

Reporting Document: ( ) ABN ( ) ECR  
(X) DCN 10881 REV. A

Description of Violation:

Bolt Dia. 5/8"  $\phi$  WEDGE ANCHOR  
Min'm. spacing req'd. 4.69" (SEE CALC. SHEET 156)  
Min'm. edge dist. req'd. 3"  
Actual Spacing 3 3/4"  
Actual Edge Distance NA

Affected Items and Corresponding Design Documents for the Interaction

( ) C: Controls ( ) I ( ) II/I ( ) N DC#  
( ) E: Electrical ( ) I ( ) II/I ( ) N DC#  
( ) M: Mechanical ( ) I ( ) II/I ( ) N DC#  
(X) S: Structural ( ) I ( ) II/I ( ) N DC#

Acceptance (of violation):

( ) This Violation has been evaluated generically "severe case simulation" DC-3200 - Case No. \_\_\_\_\_  
( ) Per attached Computations  
(X) SEE CALC. SHEET 156

Results (of evaluation):

(X) This violation has no adverse effect on the load carrying capability of the anchors  
( ) \_\_\_\_\_

Comments: \_\_\_\_\_  
\_\_\_\_\_  
\_\_\_\_\_

Prepared by KTW Date 6/7/96  
Checked by E. ATAY Date 6/7/96